GENERAL

1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, DESIGN PROFESSIONAL, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS. NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE DESIGN PROFESSIONAL OF RECORD OR ANY OF THE DESIGN PROFESSIONAL OF RECORD'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.

2. CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.

3. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.

4. CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.

5. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.

6. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SEE THE ARCHITECTURAL DRAWINGS.

7. CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. DESIGN PROFESSIONAL SHALL BE NOTIFIED OF ANY DISCREPANCY.

8. CONTRACTOR SHALL VERIFY THE STRUCTURALLY SUPPORTED MECHANICAL EQUIPMENT WEIGHTS, OPENING SIZES AND LOCATIONS IDENTIFIED ON THE STRUCTURAL DRAWINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

9. CONTRACTOR SHALL VERIFY THAT MISCELLANEOUS FRAMING SHOWN ON THE STRUCTURAL DRAWINGS FOR MECHANICAL EQUIPMENT, OWNER-FURNISHED ITEMS, PARTITIONS, ETC. IS CONSISTENT WITH THE REQUIREMENTS OF SUCH ITEMS.

10. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.

11. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR.

12. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.

13. ELECTRONIC DRAWING FILES WILL NOT BE PROVIDED TO THE CONTRACTOR. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.

14. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE DESIGN PROFESSIONAL DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE DESIGN PROFESSIONAL. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.

15. DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THE TYPICAL DETAILS UNLESS THOSE LOCATIONS ARE SPECIFICALLY DETAILED OTHERWISE.

16. STRUCTURAL DESIGN PROFESSIONAL IS NOT RESPONSIBLE FOR THE DESIGN OF PULLDOWN STAIRS, HANDRAILS OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. SUCH SYSTEMS SHALL BE DESIGNED, FURNISHED, AND INSTALLED AS REQUIRED BY OTHER PORTIONS OF THE CONTRACT DOCUMENTS.

CODE/DESIGN CRITERIA

1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE FOLLOWING:
   - INTERNATIONAL BUILDING CODE, 2018 EDITION, WITH GEORGIA AMENDMENTS

2. GRAVITY LOADS

   A. UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):
   - GENERAL AREAS: 100 PSF
   - CLASSROOMS: 100 PSF
   - OFFICES : 100 PSF
   - CORRIDORS: 100 PSF
   - STORAGE: 100 PSF
   - CATWALK: 40 PSF

   B. UNIFORM ROOF LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):
   - ROOF: 20 PSF
   - GROUND SNOW LOAD: 5 PSF
   - SNOW EXPOSURE COEFFICIENT = 0.9
   - SNOW LOAD IMPORTANCE FACTOR = 1.00
   - THERMAL FACTOR = 1.00

   PONDING AND DRIFT EFFECTS HAVE BEEN INCLUDED IN THE DESIGN.

   C. DEAD LOADS (IN ADDITION TO STRUCTURE SELF-WEIGHT):

   FLOOR:
   - MISCELLANEOUS: 5 PSF
   - CEILING/MEP: 5 PSF

   ROOF:
   - ROOFING: 5 PS
   - INSULATION: 5 PS
   - MISCELLANEOUS: 5 PSF
   - CEILING/MEP: 5 PSF

3. WIND LOADS:
   - 107 MPH BASIC WIND SPEED
   - IMPORTANCE FACTOR = 1.00
   - EXPOSURE B
   - INTERNAL PRESSURE COEFFICIENT = ±0.18

   SEE COMPONENT AND CLADDING DESIGN WIND PRESSURE DIAGRAM

4. EARTHQUAKE LOADS:
   - SEISMIC IMPORTANCE FACTOR: I = 1.00
   - OCCUPANCY CATEGORY: E
   - SHORT PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, S_S = 0.2144
   - 1 SECOND PERIOD MAPPED SPECTRAL RESPONSE COEFFICIENT, S1 = 0.0956
   - SITE CLASS D
   - SHORT PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, SD_S = 0.229
   - 1 SECOND PERIOD DESIGN SPECTRAL RESPONSE COEFFICIENT, SD1 = 0.153
   - SEISMIC DESIGN CATEGORY: C
   - BASIC SEISMIC-FORCE RESISTING SYSTEM: LIGHT FRAME WALLS WITH SHEAR PANELS – ALL OTHER MATERIALS
   - DESIGN BASE SHEAR: 15.1 KIPS
   - OVER STRENGTH FACTOR, OMEGA = 2.0
   - SEISMIC RESPONSE COEFFICIENT, C_S = 0.1143
   - RESPONSE MODIFICATION FACTOR, R = 2.0
   - ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

5. UNLESS NOTED OTHERWISE CALCULATED INDIVIDUAL MEMBER DEFLECTIONS (IN INCHES) DO NOT EXCEED THE FOLLOWING:

   DEAD LOAD
   ROOF MEMBERS: L/360
   FLOOR MEMBERS: L/360

   LIVE LOAD
   ROOF MEMBERS: L/360
   FLOOR MEMBERS: L/360

   DEAD + LIVE LOAD
   ROOF MEMBERS: L/240
   FLOOR MEMBERS: L/240

   - WHERE, L = SPAN LENGTH (IN INCHES) BETWEEN SUPPORTS. (FOR CANTILEVERS, L IS TWICE THE LENGTH OF THE CANTILEVER.) NOTE THAT THE TOTAL MAXIMUM CALCULATED FLOOR SYSTEM DEFECTION WILL BE THE SUM OF THE DEFLECTIONS OF THE SUPPORTED ELEMENTS IN A BAY.

6. SPECIAL INSPECTIONS:

   A. THE STRUCTURAL TESTING/INSPECTION AGENCY, SEE SPECIFICATION SECTION 01410, WILL PERFORM SPECIAL INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE. SEE SPECIAL INSPECTIONS SCHEDULE FOR A COMPLETE LIST OF WORK REQUIRING SPECIAL INSPECTIONS.

   B. SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE ARE REQUIRED FOR STRUCTURAL COMPONENTS AND ASSEMBLIES WHICH ARE NOT FABRICATED AT THE CONSTRUCTION JOB SITE INCLUDING BUT NOT LIMITED TO FLOOR AND ROOF TRUSSES AND JOISTS OF WOOD AND STEEL MATERIALS, STRUCTURAL STEEL FRAMING, AND PRECAST CONCRETE, JOISTS, BEAMS, COLUMNS, SLABS, WALLS AND CLADDING.

   C. SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE MAY BE WAIVED FOR ITEMS WHICH ARE PRODUCED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED UPON REVIEW OF THE FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND BY PERIODIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE WHICH STATES THAT THE FABRICATION WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

   D. THE PROJECT OWNER WILL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PERFORM INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE DURING CONSTRUCTION OF THE PROJECT. DOCUMENTATION THAT SUMMARIZES THE QUALIFICATION AND CREDENTIALS OF EACH SPECIAL INSPECTOR AND DEMONSTRATES COMPETENCE FOR INSPECTION OF EACH PARTICULAR TYPE OF CONSTRUCTION REQUIRING SPECIAL INSPECTION SHALL BE SUBMITTED TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

   E. APPROVED SPECIAL INSPECTORS SHALL FURNISH INSPECTION REPORTS TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE AND TO THE DESIGN PROFESSIONAL WHICH INDICATE THAT THE WORK INSPECTED WAS DONE IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. A FINAL REPORT WHICH DOCUMENTS THE RESULTS OF THE SPECIAL INSPECTIONS PERFORMED INCLUDING CORRECTION OF ANY DISCREPANCIES IDENTIFIED DURING INSPECTION SHALL BE SUBMITTED PERIODICALLY AT A FREQUENCY APPROVED BY CHIEF COMMERCIAL BUILDING INSPECTOR PRIOR TO CONSTRUCTION.

7. NO PROVISIONS HAVE BEEN MADE FOR FUTURE HORIZONTAL OR VERTICAL EXPANSION.

FOUNDATION

1. ALL FOUNDATIONS SHALL BE INSTALLED UNDER THE GUIDANCE OF A REGISTERED PROFESSIONAL GEOTECHNICAL ENGINEER IN THE PROJECT STATE. THE GEOTECHNICAL ENGINEER SHALL GIVE CONSIDERATION TO THE TYPE OF BUILDING AND FOUNDATION LOADS INVOLVED AS WELL AS THE REQUIREMENTS OF THESE DOCUMENTS. DESIGN PROFESSIONAL IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT TO THOSE ASSUMED FOR DESIGN.

2. STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM.

3. INDIVIDUAL SPREAD FOOTINGS AND CONTINUOUS FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 2000 PSF AND 2000 PSF, RESPECTIVELY.

   A. NO FOOTINGS SHALL BEAR ON ROCK. UNDERCUT ROCK A MINIMUM OF 2 FEET BELOW BOTTOM OF FOOTING AND REPLACE WITH STRUCTURAL FILL.

4. FOUNDATION WALLS ARE DESIGNED FOR LATERAL PRESSURES DUE TO THE FOLLOWING EQUIVALENT FLUID DENSITIES:
   - WALLS FREE TO DISPLACE AT TOP (ACTIVE CONDITION): 45 PCF

5. PROOF ROLL BUILDING AREAS WITH TWO COMPLETE COVERAGES OF A LOADED DUMP-TRUCK OR SCRAPER. REPLACE SOFT AREAS WITH COMPACTED STRUCTURAL FILL AS REQUIRED BY THE SPECIFICATIONS.

6. DENSIFY BUILDING AREAS AND A MINIMUM OF 15'-0" OUTSIDE THE BUILDING PERIMETER USING A VIBRATORY ROLLER (SEE SPECIFICATIONS).

7. UNDERCUT THE ENTIRE BUILDING AREA TO THE EXTENT SHOWN ON THE STRUCTURAL DOCUMENTS AND REPLACE WITH COMPACTED STRUCTURAL FILL AS REQUIRED BY THE SPECIFICATIONS.

8. STRUCTURAL FILL SHALL CONTAIN NO ORGANIC MATERIAL AND BE APPROVED BY A GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT. STRUCTURAL FILL UNDER SLABS AND WITHIN 10'-0" OF THE BUILDING FOOTPRINT SHALL BE PLACED IN LIFTS OF THICKNESS DETERMINED BY THE INDEPENDENT TESTING AGENCY AND COMPACTED TO AT LEAST 95% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698. THE TOP 12" SUB-BASE UNDER SLABS ON GRADE SHALL BE COMPACTED TO AT LEAST 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY. ALL BACKFILL, COMPACTION AND PROOF ROLLING OPERATIONS SHALL BE OBSERVED BY AN INDEPENDENT TESTING LABORATORY.

9. SLABS-ON-GRADE SHALL BE PLACED ON A 4" GRANULAR BASE, COMPACTED TO 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698, AND COVERED WITH A 10 MIL CONTINUOUSLY SEALED VAPOR BARRIER. THE BASE FOR SLABS-ON-GRADE SHALL BE INSPECTED BY A GEOTECHNICAL ENGINEER PRIOR TO EACH PLACEMENT OF CONCRETE.

10. BACKFILL SHALL NOT BE PLACED AGAINST EXTERIOR OR RETAINING WALLS UNTIL THE WALLS HAVE ACHIEVED THEIR DESIGN STRENGTH AND THEIR LATERAL SUPPORT ELEMENTS ARE INSTALLED. PROVIDE ADEQUATE DRAINAGE AT BASEMENT AND RETAINING WALLS (SEE ARCHITECTURAL).

11. FOOTINGS SHALL BE CENTERED ABOUT COLUMN LINES UNLESS NOTED OTHERWISE.

12. ALL FOOTINGS AND TURN DOWN SLAB EDGES SHALL PENETRATE TO A MINIMUM DEPTH OF 12" BELOW FINISHED GRADE.

REINFORCEMENT

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.

2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 AND HAVE MINIMUM SIDE AND END LAPS OF 8".

3. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS AND DETAILS IS NOT ACCEPTABLE.

4. SPLICES SHALL BE CLASS B IN ACCORDANCE WITH ACI 318 UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE DESIGN PROFESSIONAL.

5. REINFORCING STEEL DESIGNATED CONTINUOUS SHALL BE LAPPED AS FOLLOWS:

   CONCRETE REINFORCEMENT – CLASS B TENSION LAP

6. PROVIDE DOWELS FROM FOUNDATIONS THE SAME SIZE AND NUMBER AS THE VERTICAL WALL OR COLUMN REINFORCING UNLESS NOTED OTHERWISE.

7. PLACE REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:

   A. CONCRETE REINFORCEMENT COVER

   EXPOSED TO EARTH OR WEATHER:
   - UNFORMED CAST AGAINST EARTH: 3" CLEAR
   - FORMED #6 AND LARGER: 2" CLEAR
   - FORMED #5 AND SMALLER: 1-1/2" CLEAR

   NOT EXPOSED TO EARTH OR WEATHER:
   - WALLS: 1" CLEAR
   - COLUMNS (TIES): 1-1/2" CLEAR
   - BEAMS/GIRDERS (STIRRUPS): 1-1/2" CLEAR
   - PT BEAMS/GIRDERS (STIRRUPS): 1-1/2" CLEAR
   - SLABS: 3/4" CLEAR

   IN AGGRESSIVE ENVIRONMENTS (SEE SPECIFICATIONS FOR DEFINITION):
   - WALLS: 1-1/2" CLEAR
   - COLUMNS: 2" CLEAR
   - BEAMS/GIRDERS: 2" CLEAR
   - SLABS TOP: 1-1/2" CLEAR / BOTTOM: 3/4" CLEAR

8. ADHESIVE FOR REINFORCING DOWELS IN EXISTING CONCRETE SHALL CONFORM TO ASTM C881-02, TYPE IV, GRADE 3, CLASS A, B, & C EXCEPT GEL TIMES AND EPOXY CONTENT. ADHESIVE SHALL CONSIST OF A TWO COMPONENT ADHESIVE SYSTEM CONTAINED IN SIDE BY SIDE PACKAGING CONNECTEED TO A MIXING NOZZLE WHICH THOROUGHLY MIXES THE COMPONENTS AS IT IS INJECTED INTO THE HOLE. ADHESIVE SHALL HAVE PASSED ICC EVALUATION SERVICES, INC (ICC-ES) ACCEPTANCE CRITERIA 308 FOR LONG TERM CREEP. REINFORCING INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 308 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT LENGTH SHALL BE 12 BAR DIAMETERS, UNLESS NOTED OTHERWISE.

9. ALL DOWELS AND TERMINATING BARS SHALL HAVE A STANDARD 90 DEGREE HOOK.

10. ALL HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL AND/OR CONSTRUCTION JOINTS AND AROUND CORNERS.

CAST-IN-PLACE CONCRETE

1. CONCRETE WORK SHALL CONFORM TO ACI 318 AND CRSI STANDARDS.

2. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH:

   A. NORMAL WEIGHT STRUCTURAL CONCRETE:
   - FOOTINGS, PEDESTALS: 3000 PSI
   - FOUNDATION WALLS: 3000 PSI
   - RETAINING WALLS: 3000 PSI
   - SLABS-ON-GRADE: 4000 PSI

3. PIPES OR DUCTS SHALL NOT EXCEED ONE-THIRD THE SLAB OR WALL THICKNESS INCLUDING CROSSING UNLESS SPECIFICALLY DETAILED IN THE STRUCTURAL DOCUMENTS. ALL PIPES AND DUCTS SHALL BE PLACED IN THE MIDDLE THIRD OF THE SLAB OR WALL THICKNESS UNLESS SPECIFICALLY DETAILED OTHERWISE IN THE STRUCTURAL DOCUMENTS. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.

4. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, CLIPS OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND FOR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.

5. CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE DESIGN PROFESSIONAL. NO HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL DRAWINGS.

6. DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.016 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE DESIGN PROFESSIONAL.

*** USE THE REMAINING CONCRETE NOTES FOR SMALL PROJECTS WHERE SPECIFICATIONS ARE NOT ISSUED ***

7. CONCRETE MIX DESIGN FOR 3000 PSI CONCRETE SHALL BE BASED ON A MAXIMUM AGGREGATE SIZE OF 1 IN. MAXIMUM WATER/CEMENT RATIO OF .58 FOR NON-AIR-ENTRAINED CONCRETE AND .46 FOR AIR-ENTRAINED CONCRETE AND A MAXIMUM SLUMP OF 4 IN. AIR ENTRAINED CONCRETE SHALL BE USED FOR EXTERIOR EXPOSED CONCRETE WITH AN AIR CONTENT BETWEEN 5.5 AND 7.5 PERCENT.

8. CONCRETE SLABS ON GRADE SHALL NOT BE LOADED UNTIL A MINIMUM CONCRETE STRENGTH OF 1800 PSI HAS BEEN ATTAINED AND THE CONCRETE IS AT LEAST THREE DAYS OLD. ALL OTHER CONCRETE MEMBERS SHALL NOT BE LOADED UNTIL THE SPECIFIED CONCRETE DESIGN STRENGTH HAS BEEN ATTAINED.

9. CONCRETE SHALL BE TESTED IN ACCORDANCE WITH ACI 301 AND THE SPECIFICATIONS FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS. AT A MINIMUM CONCRETE SPECIMENS SHALL BE TAKEN FOR EVERY 100 YARDS OR PORTION THEREOF FOR EACH MIX DESIGN PLACED IN A DAY. CONCRETE TEST REPORTS SHALL BE AVAILABLE ON SITE FOR INSPECTION.

10. C.J. ON THE SLAB AND FOUNDATION PLAN INDICATES A KEY-FORMED CONSTRUCTION JOINT OR SAW-CUT CONTROL JOINT IN THE CONCRETE SLAB ON GRADE. SAW-CUT CONTROL JOINTS SHALL BE INSTALLED WITHIN 8 HOURS OF SLAB PLACEMENT. CONTINUE REINFORCEMENT THROUGH JOINTS. CONSTRUCTION AND/OR CONTROL JOINTS SHALL BE SPACED NO FARTHER APART THAN 48 TIMES THE SLAB THICKNESS OC IN EACH DIRECTION CREATING PANELS WITH AN ASPECT RATIO NOT GREATER THAN 2:1.

11. UNLESS NOTED OTHERWISE, ALL REINFORCING SHALL BE CONTACT LAP SPLICED WITH A CLASS B SPLICE IN ACCORDANCE WITH ACI 318-05. FOR BARS WITH MINIMUM COVER AND SPACING GREATER THAN AND 3d, RESPECTIVELY, THE MINIMUM SPLICE LENGTH OF NOT LESS THAN 48db (db=BAR DIAMETER) SHALL BE USED. SPLICE LENGTHS SHALL BE INCREASED BY A FACTOR OF 1.3 FOR TOP REINFORCEMENT. LAP WELDED WIRE FABRIC (WWF) ONE SPACE PLUS 2 IN. ON ALL SIDES AT SPLICES.

12. ALL EXPOSED CORNERS OF CONCRETE SHALL HAVE A CHAMFER OR RADIUS OF 3/4" UNLESS NOTED OTHERWISE.

13. CONCRETE SHALL RECEIVE THE FOLLOWING FINISHES:
   - INTERIOR EXPOSED SLABS (UNO OR REQUESTED BY THE OWNER): STEEL TROWEL FINISH
   - FLOOR QUALITY CLASSIFICATION:
     "CONVENTIONAL" FF/FL = 25/20 (CARPETED OFFICE)
     "FLAT" FF/FL = 36/20 (THIN-SET CTD OR VCT)
   - EXTERIOR SLABS: BROOM FINISH, DIRECTION OF SLOPE
   - ALL OTHER CONCRETE: STEEL TROWEL FINISH

14. MAINTAIN CONCRETE AFTER PLACEMENT WITH MINIMAL MOISTURE LOSS AT RELATIVELY CONSTANT TEMPERATURE FOR THE PERIOD NECESSARY FOR HYDRATION OF CEMENT AND HARDENING OF CONCRETE (NOT LESS THAN 7 DAYS). COMPLY WITH THE REQUIREMENTS OF ACI 308- STANDARD PRACTICE FOR CURING CONCRETE; AMERICAN CONCRETE INSTITUTE'S COMBINATION CURING AND SEALING COMPOUND SHALL BE APPLIED AFTER THE CONCRETE HAS BEEN FINISHED OR THE FORMS REMOVED. COMPOUND SHALL MEET THE REQUIREMENTS OF ASTM C1315.

WOOD

1. WOOD FRAMING SHALL BE SOUTHERN PINE, NO. 2 K.D. (15% MAX. MOISTURE CONTENT) OR EQUIVALENT. MINIMUM ALLOWABLE BENDING STRESS SHALL BE:
   - 2X6 – 1100 PSI
   - 2X8 – 1000 PSI
   - 2X10 – 925 PSI
   - 2X12 – 800 PSI
   - 2X14 – 750 PSI

2. STRUCTURAL LAMINATED VENEER LUMBER (LVL) SHALL BE PRODUCED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC), AMD SHALL HAVE THE FOLLOWING PROPERTIES:
   - E = 1,800,000 PSI
   - Fb = 2,250 PSI
   - Fc PERP = 750 PSI
   - Fc PARALLEL = 1,600 PSI

3. WOOD TRUSSES SHALL BE CAPABLE OF SUPPORTING THE SUPERIMPOSED LOADS AS GIVEN IN THE CONTRACT DOCUMENTS.

4. CUTTING OR ALTERING OF WOOD TRUSSES IS NOT PERMITTED.

5. DESIGN OF WOOD TRUSSES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS, DESIGN LOAD DATA, AND SUPPORT REACTIONS SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS WITH REGARD TO TRUSS CONFIGURATION, AND THE CONTRACTOR'S INTERPRETATION OF DESIGN LOADS AND DETAILS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN OF THE TRUSSES OR TRUSS CONNECTIONS NOT SPECIFICALLY DETAILED IN THE CONTRACT DOCUMENTS.

6. ERECTION AND TEMPORARY BRACING OF PREFABRICATED WOOD TRUSSES SHALL BE IN CONFORMANCE WITH THE RECOMMENDATIONS OF THE TRUSS MANUFACTURER AND THE TRUSS PLATE INSTITUTE'S "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS".

7. CONNECTIONS FOR STRUCTURAL TIMBER SHALL BE GALVANIZED STRONG-TIE CONNECTORS BY THE SIMPSON COMPANY OR APPROVED EQUAL.

8. WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE FOUNDATION GRADE PRESSURE- TREATED DOUGLAS FIR. USE GALVANIZED NAILS IN PRESSURE-TREATED WOOD.

9. PLYWOOD SHALL BE ORIENTED AND NAILED TO SUPPORTING MEMBERS AS NOTED IN THE STRUCTURAL DOCUMENTS.

JOB NUMBER: 19221
CHECKED BY: JMW

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HAPPY FACES ACADEMY
LEARN . PLAY . GROW .

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GENERAL NOTES

S0.1

DATE 01/27/21