

**PROJECT NOTES**

**GENERAL**

- THESE NOTES SUMMARIZE PROJECT INFORMATION. PLANS, DETAILS AND SPECIFICATIONS SHALL ALSO BE REFERENCED FOR COMPLETE REQUIREMENTS.
- REFERENCE ARCHITECTURAL PLANS FOR DIMENSIONAL CONTROL.
- REQUIREMENTS GIVEN FOR ONE LOCATION SHALL ALSO APPLY AT OTHER LOCATIONS WITH SIMILAR CONDITIONS, UNLESS NOTED OTHERWISE.
- CONTRACTOR SHALL COORDINATE WORK OF OTHER TRADES & UTILITIES WITH STRUCTURAL WORK. SHOP DRAWINGS SHALL BE SUBMITTED WITH ALL INTERFERENCES AND CONFLICTS, NOT RESOLVED BETWEEN DISCIPLINES, NOTED FOR INSTRUCTIONS. ANY CONFLICTS THAT ARISE FROM WORK COMPLETED WITHOUT COORDINATED SHOP DRAWINGS SHALL BE CORRECTED AT THE CONTRACTOR'S EXPENSE.
- DESIGN LOADS
  - A. BUILDING CODE: INTERNATIONAL BUILDING CODE, 2018 EDITION
  - B. DEAD LOAD  
ACTUAL WEIGHT OF MATERIALS USED
  - C. LIVE LOAD  
ROOF = 20 PSF  
GROUND SNOW LOAD, Pg, = 5 PSF  
  
BUILDING CATEGORY: II
  - D. WIND LOAD  
BASIC WIND SPEED = 115 MPH  
WIND EXPOSURE CATEGORY = B  
INTERNAL PRESSURE COEFFICIENT, GCpi = 0.18
  - E. EARTHQUAKE LOAD (EQUIVALENT LATERAL FORCE ANALYSIS)  
IMPORTANCE FACTOR, Ie, = 1.0  
USE GROUP = II  
Sds = 0.273      Sd1 = 0.150  
SITE CLASS D - ASSUMED  
SEISMIC DESIGN CATEGORY C  
BASIC SEISMIC-FORCE-RESISTING SYSTEM:  
INTERMEDIATE REINFORCED MASONRY SHEAR WALLS  
SEISMIC RESPONSE COEFFICIENTS, Cs, = 0.078  
RESPONSE MODIFICATION FACTOR, R, = 3 1/2  
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

**EARTHWORK**

- EARTHWORK SHALL BE IN ACCORDANCE WITH DIVISION 2 OF THE SPECIFICATIONS AND THE GEOTECHNICAL REPORT, BY CSRA TESTING.
- STRIP AND STOCK PILE ALL ORGANIC TOPSOIL PRIOR TO GRADING OPERATIONS OR CONSTRUCTION. TOPSOIL SHALL BE STOCKPILED FOR LATER USE AS INDICATED BY PROJECT SPECIFICATIONS. PROJECT GEOTECHNICAL ENGINEER SHALL VERIFY REMOVAL OF TOPSOIL. REMOVE ALL EXISTING FOUNDATIONS, PAVING AND UTILITIES FROM THE PROPOSED CONSTRUCTION AREA AND BACKFILL IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.
- PROOF ROLL BUILDING AREA AND 10 FEET BEYOND PRIOR TO FILLING OR CONSTRUCTION. AREAS THAT EXHIBIT PUMPING SHALL BE CORRECTED AS INDICATED IN THE PROJECT SPECIFICATIONS.
- EXTREME CARE SHALL BE EXERCISED WHEN EXCAVATING OR GRADING ADJACENT TO EXISTING STRUCTURES OR IMPROVEMENTS SO AS NOT TO DAMAGE OR UNDERMINE FOUNDATIONS, WALLS, SLABS, UTILITIES ETC.
- STRUCTURAL FILL SHALL BE PLACED AND COMPACTED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND THE GEOTECHNICAL REPORT.
- PROJECT GEOTECHNICAL ENGINEER SHALL INSPECT AND TEST EXCAVATIONS TO CONFIRM BEARING CAPACITY PER THE FOLLOWING:
  - A. ONE TEST FOR EACH SPREAD FOOTING
  - B. ONE TEST FOR EACH 100 LINEAR FEET OF CONTINUOUS FOOTING
  - C. ONE TEST FOR EACH 1,000 SQ. FEET OF SLAB
- DESIGN SOIL PRESSURE = 2,000 PSF ASSUMED TO BE VERIFIED BY GEOTECHNICAL ENGINEER PRIOR TO FOUNDATION PLACEMENT.

**MASONRY WALL REINFORCING**

- MASONRY WALL CONSTRUCTION AND REINFORCING SHALL BE IN ACCORDANCE WITH DIVISION 4 OF THE SPECIFICATIONS AND TMS 402/602-16 & DETAIL 8/S3.0.
- ALL CONCRETE MASONRY UNITS SHALL HAVE A NET COMPRESSIVE STRENGTH OF 2,000 psi AND SHALL CONFORM TO ASTM C90, GRADE N.
- FOR ALL CONCRETE MASONRY UNITS ABOVE AND BELOW GRADE, MORTAR TO BE TYPE S AND CONFORM TO ASTM C220.
- THE QUALITY OF THE CONSTRUCTION OF LOAD BEARING MASONRY WALLS SHALL BE MONITORED PER THE LEVEL 2 QUALITY ASSURANCE PROGRAM AS DICTATED IN SECTION 1.6 TABLE 3 & 4, TMS 602.  
- ENGINEER TO BE NOTIFIED PRIOR TO FILLING CELLS
- REINFORCING SHALL CONFORM TO ASTM A615, GRADE 60.
- VERTICAL REINFORCING - PROVIDE (1) REBAR (TO MATCH REINFORCING SIZE CALLED FOR ON PLANS) IN CONCRETE FILLED CELL, CONTINUOUS FROM FOOTING TO BOND BEAM AT TOP OF WALL. REBAR SHALL EXTEND 4" INTO BOND BEAM. BREAK-OUT BOTTOM OF BOND BEAM AND POUR TOP 4 COURSES OF CELL WITH BOND BEAM. PROVIDE AT THE FOLLOWING LOCATIONS:
  - AT WALL CORNERS.
  - AT ENDS OF ALL WALLS AND EACH SIDE OF EXPANSION JOINTS.
  - AT ALL DOOR AND WINDOW JAMBS.
  - AT SPACING INDICATED ON THE PLANS.
- REINFORCING SHALL BE TIED ON CENTERLINE OF CELLS. REINFORCING IS TO BE PLACED AND SECURED IN CELLS PRIOR TO PLACING GROUT.
- VERTICAL SPLICES SHALL BE PER THE FOLLOWING:
  - #4'S = 24"
  - #5'S = 30"
- CONCRETE FILLED CELLS SHALL BE FILLED IN MAXIMUM 4 FOOT LIFTS, WITH 2500 psi MASONRY GROUT CONFORMING TO ASTM C476.
- ALL CELLS AND CAVITIES BELOW GRADE SHALL BE FILLED WITH GROUT.
- HORIZONTAL REINFORCEMENT-PROVIDE 9 GA. LADDER TYPE MASONRY REINFORCING AT 16" O.C. ALONG ENTIRE LENGTH OF WALL. DISCONTINUE REINFORCING AT CONTROL JOINTS.
- PROVIDE CONCRETE FILLED BOND BEAM WITH (2)-#5 REBAR WHERE WALLS ARE STRUCTURALLY CONNECTED TO ROOF AND FLOORS AND AT TOPS OF ALL WALLS. BOND BEAM SHALL BE CONTINUOUS.
- PROVIDE BOND BEAM WITH 2-#5'S AT DOOR AND WINDOW HEADS AND WINDOW SILLS. EXTEND 2'-0" BEYOND OPENING.
- CONTROL JOINTS SHALL BE PROVIDED AT THE FOLLOWING LOCATIONS:
  - AT A MAXIMUM OF 3 TIMES THE WALL HEIGHT, WITH A MAXIMUM SPACING OF 30 FEET O.C.
  - AT A MAXIMUM DISTANCE OF 3 TIMES THE WALL HEIGHT OR 30 FEET FROM BONDED INTERSECTIONS OR CORNERS.
  - AT CHANGES IN WALL HEIGHT AND/OR THICKNESS.
  - ALONG JOINTS IN FOUNDATIONS, FLOORS AND ROOF WHICH BEAR ON WALLS.
- MASONRY WALLS SHALL BE BRACED UNTIL ROOF AND/OR FLOOR DIAPHRAGMS ARE TIED IN.
- ALL INTERIOR NON-LOAD BEARING MASONRY WALLS SHALL BE PERMANENTLY BRACED AT 8'-0" O.C. AN INTERSECTING MASONRY WALL IS CONSIDERED A BRACE POINT. SEE WALL BRACING DETAILS 17S-5 & 17S-5
- NO CONDUITS, PIPES, OR SLEEVES SHALL BE PLACED IN REINFORCED AND GROUTED CELLS WITHOUT APPROVAL OF ENGINEER.
- INTERSECTING WALLS SHALL BE CONNECTED BY EITHER OVERLAPPING OF CMU UNITS, 9 GA. TRUSSED WIRE REINFORCING AT 16" O.C. EXTENDING A MIN. OF 30" INTO EACH WALL ELEMENT, OR METAL STRAPS PER ACI 308 AT 4'-0" O.C. MIN. (TYPICAL UNLESS A CONTRADICTION IS REQUIRED ADJACENT TO WALL INTERSECTION).
- CONTRACTOR TO SUBMIT CERTIFICATES OF MATERIALS FOR ALL MATERIALS USED IN THE MASONRY CONSTRUCTION. INCLUDE VERIFICATION OF THE COMPRESSIVE STRENGTH OF THE CONCRETE MASONRY UNITS.

**STRUCTURAL STEEL**

- STRUCTURAL STEEL CONSTRUCTION SHALL BE IN ACCORDANCE WITH DIVISION 5 OF THE SPECIFICATIONS AND AISC 360-16.
- STRUCTURAL STEEL BEAMS AND COLUMNS SHALL CONFORM TO ASTM-A992. MISCELLANEOUS STEEL SHAPES, PLATES AND ANGLES SHALL CONFORM TO ASTM-A36. STRUCTURAL PIPE AND TUBES SHALL CONFORM TO ASTM-A500 GRADE B.
- BOLTED CONNECTIONS SHALL CONFORM TO ASTM -A325N UNLESS OTHERWISE INDICATED. ANCHOR BOLTS SHALL CONFORM TO ASTM-A307 OR A36.
- MINIMUM WELD SIZE SHALL BE 1/4" FILLET WITH E70XX ELECTRODES.
- MINIMUM MATERIAL THICKNESS FOR MISCELLANEOUS PLATES SHALL BE 3/8" INCH.
- ALL BEAMS SHALL BE ON CONCRETE FILLED MASONRY. MINIMUM END BEARING SHALL BE 8". PROVIDE SLOT CONNECTIONS HOLES AT BUILDING EXPANSION JOINTS.
- PLUMB COLUMNS WITH STEEL WEDGES AT EDGES OF BASE PLATE. DO NOT USE LEVELING NUTS. GROUT FOR BASE PLATES SHALL BE NON-SHRINK, NON METALLIC. REMOVE WEDGES AND FILL VOIDS AFTER INITIAL GROUTING HAS REACHED SIGNIFICANT STRENGTH.
- OPEN WEB STEEL JOIST CONSTRUCTION SHALL BE IN ACCORDANCE WITH DIVISION 5 OF THE SPECIFICATIONS AND STEEL JOIST INSTITUTE "STANDARD SPECIFICATION FOR OPEN WEB STEEL JOISTS, K AND LH-SERIES, 1994"
- JOIST MANUFACTURER SHALL DESIGN AND FABRICATE JOIST FOR POINT LOADS AND UNIFORM LOADS INDICATED ON THE DRAWINGS IF STANDARD LOADS ARE NOT GIVEN. JOIST DEPTH SHALL BE AS INDICATED ON PLANS.
- JOIST MANUFACTURER SHALL ADD 10 PSF NET UPLIFT TO ALL JOISTS UNLESS NOTED OTHERWISE.
- BRIDGING SHALL BE ATTACHED TO WALLS AT END OF ALL BRACING LINES.
- PROVIDE SLOPED BEARING SEATS FOR ALL JOISTS ON A SLOPE GREATER THAN 1/4"/FOOT.
- STEEL DECK CONSTRUCTION SHALL BE IN ACCORDANCE WITH DIVISION 5 OF THE SPECIFICATIONS AND STEEL DECK INSTITUTE. ALL DECKING SHALL BE GALVANIZED UNLESS OTHERWISE INDICATED.
- LOCATION AND SIZE OF ROOF/FLOOR PENETRATIONS SHALL BE COORDINATED WITH MECHANICAL DRAWINGS. OPENINGS GREATER THAN 12" WIDE AND ALL MECHANICAL EQUIPMENT SHALL HAVE SUPPORT FRAMING IN ACCORDANCE WITH PROJECT DRAWINGS. SUBMIT FRAMING SHOP DRAWINGS FOR REVIEW.
- SHOP DRAWINGS FOR STRUCTURAL STEEL, OPEN WEB JOIST AND STEEL DECK SHALL BE SUBMITTED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. SHOP DRAWINGS SHALL BE SUBMITTED CONTRACTOR APPROVED.
- CONTRACTOR SHALL NOTIFY ARCHITECT 24 HOURS PRIOR TO STEEL AND DECK ERECTION.

**CONCRETE**

- CONCRETE WORK SHALL BE IN ACCORDANCE WITH DIVISION 3 OF THE SPECIFICATIONS AND ACI 318-14.
- CONCRETE SHALL BE AS FOLLOWS:
 

LOCATION	28 DAY STRENGTH	MIN CEMENT CONTENT	SLUMP	MAX AGG. SIZE
FOOTINGS	3000 PSI	5.5 BAG/YD	4" ±1"	1 1/2"
SLAB ON GRADE	4000 PSI	6.3 BAG/YD	3" ±1"	1/2"
ELEVATED SLABS	4000 PSI	6.3 BAG/YD	3" ±1"	1/2"

FLY ASH PER ASTM C618, TYPE C OR F SHALL BE PERMITTED WITHIN THE FOLLOWING LIMITS:  
-RATE OF REPLACEMENT SHALL BE 1.25 TO 15 LBS OF FLY ASH TO 100 LBS OF CEMENT. QUANTITY OF CEMENT REPLACED SHALL BE MORE THAN 15%.
- GROUT FOR MASONRY WALLS SHALL COMPLY WITH ASTM C-476. GROUT FOR REINFORCED MASONRY. AGGREGATES SHALL COMPLY WITH ASTM-C404. GROUT SHALL BE COURSE GROUT WITH A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI.
- REINFORCING SHALL COMPLY WITH ASTM-A615, GRADE 60. WELDED WIRE FABRIC (W/F) SHALL BE PER ASTM-A25. W/F LAPS SHALL BE A MINIMUM OF 8". ALL REINFORCING (STEEL, DOUGLASS, ANCHOR BOLTS, INSERTS, ET. SHALL BE SECURELY TIED IN PLACE PRIOR TO POURING CONCRETE. CONCRETE BLOCKS SHALL BE USED TO SUPPORT REINFORCING (METAL STAKES OR RODS WILL NOT BE PERMITTED). SLAB REINFORCING SHALL BE ADEQUATELY SUPPORTED BY APPROVED CHAIRS TO MINIMIZE SAG.
- FIELD WELDING OR BENDING OF REINFORCING IS NOT PERMITTED EXCEPT AS APPROVED BY THE STRUCTURAL ENGINEER.
- POLYPROPYLENE FIBERS SHALL BE PER ASTM-C1116.
- CHAMFER ALL EXPOSED CONCRETE EDGES 3/4" UNLESS NOTED OTHERWISE.
- SHOP DRAWINGS FOR CONCRETE, REINFORCING AND EMBEDDED ITEMS SHALL BE SUBMITTED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. SHOP DRAWINGS SHALL BE SUBMITTED CONTRACTOR APPROVED.
- CONTRACTOR SHALL NOTIFY ARCHITECT 24 HOURS TO BEGINNING FILL/BACKFILL OPERATIONS AND CONCRETE PLACEMENT. NOTIFICATIONS SHALL BE FOR OBSERVATION OF FORMWORK, REINFORCING AND EMBED ITEMS.
- CONTRACTOR SHALL COORDINATE DEPTH OF FOOTINGS WITH PLUMBING PIPING. FOOTINGS SHALL BE STEPPED TO ALLOWING PIPE TO PASS OVER FOOTING UNLESS PIPING IS A MINIMUM OF 1'-4" BELOW BOTTOM OF FOOTING ELEVATION. FOOTINGS STEPS SHALL BE SHOWN ON SHOP DRAWINGS. TUNNELING UNDER A PREVIOUSLY PLACED FOOTING SHALL NOT BE PERMITTED.

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STRUCTURAL GENERAL NOTES

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