GENERAL CONDITIONS:

1. THE FOLLOWING NOTES SHALL APPLY TO ALL STRUCTURAL DRAWINGS.
2. ALL DESIGN AND CONSTRUCTION SHALL BE BASED ON AND IN ACCORDANCE WITH THE 2017 FLORIDA BUILDING CODE, 6TH EDITION.
3. THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS AND NOTIFY THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS, SEE THE ARCHITECTURAL DRAWINGS.
4. IF MATERIAL QUANTIITIES, STRENGTHS OR SIZES INDICATED BY THE DRAWINGS OR SPECIFICATIONS ARE NOT IN AGREEMENT WITH THESE NOTES, THE BETTER QUALITY AND/OR QUANTITY, STRENGTH OR NOTED SHALL BE PROVIDED.
5. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO ENSURE THE SAFETY OF THE STRUCTURE AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF TEMPORARY BRACING, GUYS, TIE-DOWNS OR SHORING MAY BE NECESSARY. SUCH MATERIAL SHALL BE REMOVED AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER THE COMPLETION OF THE PROJECT.
6. IT SHALL BE THE SOLE RESPONSIBILY OF THE CONTRACTOR FOR INITIATING, MAINTAINING AND SUPERVISING ALL SAFETY PROCEDURES . THE STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PROCEDURES.
7. THE STRUCTURAL DRAWINGS ARE ONE DISCIPLINE OF THE CONTRACT DOCUMENTS AND DO NOT BY THEMSELVES CONTAIN ALL THE INFORMATION REQUIRED TO PROPERLY COMPLETE THE PROJECT STRUCTURE. THE GENERAL CONTRACTOR SHALL REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS AND COORDINATE THE INFORMATION INDICATED IN THESE CONSTRUCTION DOCUMENTS WITH THE STRUCTURAL DRAWINGS TO PROPERLY CONSTRUCT THE PROJECT.
8. ALL DETAILS, SECTIONS AND NOTES INDICATED ON THE CONSTRUCTION DOCUMENTS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.
9. SPECIALTY ENGINEERED PRODUCTS

A. THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SHOP DRAWINGS FOR SPECIALTY ENGINEERED PRODUCTS WHICH SHALL BE SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. INTERIOR SPECIALTY PRODUCTS SHALL BE DESIGNED FOR LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING:

1. MISCELLANEOUS STEEL
2. LADDERS
3. HANDRAILS
4. CLADDING SYSTEMS

10. DESIGN LOADS:
    - A. DESIGN ROOF DEAD LOAD = 25 PSF
    - B. DESIGN ROOF LIVE LOAD = 20 PSF
    - C. DESIGN LIVE LOAD FOR FLOOR: 100 PSF
    - D. DESIGN WIND LOAD:
      1. ULTIMATE DESIGN WIND SPEED (3 SECOND GUST), V_{ULT} = 139 MPH.
NOMINAL DESIGN WIND SPEED (3 SECOND GUST), V_{ASD} = 108 MPH.
      2. RISK CATEGORY: II
      3. WIND EXPOSURE CATEGORY: C
      4. COMPONENTS AND CLADDING WIND PRESSURE:
SEE WIND LOAD SCHEDULE THIS SHT.
      5. INTERNAL PRESSURE COEFFICIENTS +/- 0.18
      6. WIND-BORNE DEBRIS REGION (WITHIN HURRICANE-PRONE REGIONS)
A. WITHIN 1 MILE OF THE COASTAL MEAN HIGH WATER LINE WHERE V_{ULT} IS 130 MPH OR GREATER; OR
B. IN AREAS WHERE V_{ULT} IS 140 MPH OR GREATER.
11. SHOP DRAWINGS:
    - A. THE CONTRACTOR SHALL SUBMIT, AS REQUIRED, PRINTS OF SHOP DRAWINGS FOR ALL FABRICATED MATERIALS TO ARCHITECT FOR REVIEW. REPRODUCTION OF CONTRACT DRAWINGS FOR SHOP DRAWINGS WILL NOT BE PERMITTED.
    - B. REVIEW OF SHOP DRAWINGS BY THE ARCHITECT/ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF THOSE SHOP DRAWINGS.
    - C. SHOP DRAWINGS REQUIRING A SPECIAL ENGINEERING DESIGN BY THE FABRICATOR SHALL BE STAMPED BY A PROFESSIONAL ENGINEER OF RECORD IN THE STATE OF THE PROJECT LOCATION BEFORE SUBMITTING FOR REVIEW BY THE ARCHITECT/ENGINEER. THESE DRAWINGS SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF OBSERVATIONS.
12. THE FOLLOWING SHOP DRAWINGS SHALL BE SUBMITTED:
    - A. CONCRETE MIX DESIGNS
    - B. CONCRETE REINFORCING STEEL AND WELDED WIRE FABRIC
    - C. STRUCTURAL STEEL
    - D. CONCRETE MASONRY UNIT SUBMITTALS AND OTHER MASONRY ACCESSORIES
    - E. STEEL OPEN WEB JOISTS AND METAL DECK
    - F. PRE-FABRICATED CANOPIES
    - G. COLD FORMED METAL FRAMING

MASONRY WALL CONSTRUCTION:

1. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 2000 PSI (fm = 1500 PSI).
2. MORTAR SHALL BE TYPE M OR S, CONFORMING TO ASTM C270.
3. COARSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAX. AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI W/ SLUMP OF 8-11 INCHES.
4. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS. FILL ALL CELLS CONTAINING REINFORCING WITH COARSE GROUT.
5. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 8'-0". REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL (TYPICAL) UNLESS OTHERWISE NOTED. SEE TYPICAL GROUTING DETAILS FOR ADDITIONAL INFORMATION.
6. REINFORCING STEEL SHALL BE LAPPED MINIMUM 48 BAR DIAMETERS.
7. HORIZONTAL WALL REINFORCING SHALL BE STANDARD LADDER TYPE DUR-O-WAL (9 GA.) HOT DIPPED GALVANIZED AT 16" O.C. (VERTICALLY) UNLESS SHOWN OTHERWISE ON THE DRAWINGS.
8. SPLICED WIRE REINFORCEMENT SHALL BE LAPPED AT LEAST 6" TO CONTAIN AT LEAST ONE CROSS WIRE OF EACH PIECE OF REINFORCEMENT WITHIN THE 6" LAP. USE STANDARD 'T' AND 'L' SHAPED PIECES AT INTERSECTIONS AND CORNERS.
9. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICALS. DOWELS SHALL BE GROUTED INTO A CORE IN VERTICAL ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCEMENT.

FOUNDATION:

1. FOUNDATION DESIGN IS BASED ON AN ALLOWABLE SOIL BEARING PRESSURE OF 2500 PSF FOR A CONVENTIONALLY REINFORCED SHALLOW FOUNDATION AS RECOMMENDED IN THE GEOTECHNICAL REPORT PREPARED BY ANTICUS ENGINEERING, LLC DATED OCTOBER 26, 2020, PROJECT NO.: 01.4335.20.
2. THE SITE SHALL BE PREPARED UNIFORM IN ACCORDANCE WITH CIVIL DRAWINGS, SPECIFICATIONS, SOILS REPORT AND THE ALLOWABLE BEARING PRESSURE.
3. ALL EXCAVATIONS AND BUILDING PADS SHALL BE INSPECTED BY A QUALIFIED GEOTECHNICAL ENGINEER TO VERIFY THE DESIGN ASSUMPTIONS AND REPORT ADVERSE CONDITIONS.
4. WHERE FILL IS REQUIRED, IT SHALL BE PLACED IN ACCORDANCE WITH INSTRUCTIONS OF A QUALIFIED GEOTECHNICAL ENGINEER TO MAINTAIN DESIGN BEARING PRESSURE.
5. FOOTING ELEVATIONS GIVEN ARE FOR THE PURPOSE OF DESIGN. SOIL BELOW FOOTING NOT MEETING DESIGN BEARING PRESSURE SHALL BE EXCAVATED TO A DEPTH OF VERIFIABLE DESIGN PRESSURE AND BACKFILLED PER SOIL REPORT RECOMMENDATIONS TO LEVEL OF FOUNDATION BEARING. THIS SHALL BE APPROVED BY A QUALIFIED GEOTECHNICAL ENGINEER.
6. ALL EXCAVATION SHALL BE KEPT DRY. EXCAVATE TO DEPTHS AND DIMENSIONS INDICATED. TAKE EVERY PRECAUTION TO GUARD AGAINST ANY MOVEMENT OR SETTLEMENT OF ADJACENT STRUCTURES, UTILITIES, PIPING, ETC.
7. PROVIDE ANY BRACING OR SHORING NECESSARY TO AVOID SETTLEMENT OR DISPLACEMENT OF EXISTING FOUNDATION OR STRUCTURES.
8. BACKFILL AGAINST WALLS SHALL BE PLACED EVENLY EACH SIDE UNLESS SHORING IS PROVIDED BY THE CONTRACTOR. SHORING SHALL BE THE SOLE RESPONSIBILTY OF THE CONTRACTOR AND SHALL REMAIN IN PLACE UNTIL STRUCTURAL ELEMENT BRACING THE WALL ARE IN PLACE AND HAVE REACHED FULL DESIGN STRENGTH.

CONCRETE:

1. CONCRETE MEMBERS TO HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTHS AT 28 DAYS:
   - A. FOUNDATIONS, SLAB ON GRADE 3000 PSI
   - B. BEAMS 4000 PSI
2. ALL CONCRETE SHALL BE READY MIX AND MEET THE FOLLOWING REQUIREMENTS:
   - A. CONCRETE SLUMP SHALL BE 4 INCHES PLUS OR MINUS 1".
   - B. CONCRETE SHALL HAVE 2 TO 4 PERCENT AIR ENTRAINMENT.
   - C. ALL CONCRETE TO HAVE MAXIMUM WATER/CEMENT RATIO OF 0.54.
3. CONCRETE MIX SHALL BE IN ACCORDANCE WITH THE 2010 EDITION OF ACI 301 CHAPTER 3, METHOD 1 OR METHOD 3. CONTRACTOR SHALL SUBMIT BACKUP DATA PER CHAPTER 5 SECTION 5.3 OF ACI 318 LATEST EDITION.
4. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-615 GRADE 60. SUBMIT ALL REINFORCING STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO ANY FABRICATION.
5. CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS.
6. WELDED WIRE FABRIC SHALL CONFORM WITH ASTM A 1064. WELDED WIRE FABRIC SHALL BE LAPPED AT LEAST 12 INCHES UNLESS NOTED OTHERWISE.
7. LAP SPLICE FOR BARS, U.N.O.: 31" FOR #4 BARS, 39" FOR #5 BARS, 47" FOR #6 BARS.
8. PROVIDE ACI STANDARD HOOKS UNLESS NOTED OTHERWISE ON DRAWINGS.
9. ALL CONCRETE WORK SHALL CONFORM TO ACI 318-14 "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE STRUCTURES", AND ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS."
10. ALL CONCRETE DETAILS SHALL CONFORM TO ACI 315-05 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" UNLESS NOTED OTHERWISE ON THE DRAWINGS.
11. CONTRACTOR SHALL REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS AND SIZES OF SLEEVES, OPENINGS, EMBEDDED ITEMS, SLAB RECESSES, SLOPES, ECT. THESE ITEMS SHALL BE COORDINATED WITH OTHER TRADES AND INSTALLED PRIOR TO CONCRETE PLACEMENT.
12. CONTRACTOR SHALL VERIFY ANCHOR BOLT SIZES AND LOCATIONS PRIOR TO CONCRETE PLACEMENT.
13. BAR LENGTHS PROVIDED ON DRAWINGS DO NOT INCLUDE HOOK LENGTH. HOOKS SHALL BE PROVIDED AT TOP BARS AT BEAM ENDS AND SLAB EDGES.
14. CONTRACTOR SHALL PROVIDE CHAIRS, BOLSTERS, SPACERS, ETC. AS REQUIRED TO SECURELY SUPPORT REINFORCEMENT. SUPPORT ITEMS ON EXPOSED CONCRETE SHALL BE PLASTIC SUPPORT REINFORCEMENT. TIPPED OR STAINLESS STEEL. IN HIGHLY CORROSIVE ENVIRONMENTS, SUPPORT ITEM SHALL BE PLASTIC.
15. THE CONTRACTOR SHALL SEAL ALL HAIRLINE CRACKING IN CONCRETE SLAB ON GRADE WITH A CRACK SUPPRESSION KIT SUCH AS LATICRETE OR APPROVED EQUAL.
16. A COPY OF THE "FIELD REFERENCE MANUAL" ACI SP-15 LATEST EDITION SHALL BE KEPT BY THE CONTRACTOR ON SITE.
17. ONE COPY OF ALL THE CONCRETE TEST RESULTS SHALL BE SUBMITTED BY THE TESTING AGENCY DIRECTLY TO THE ENGINEER OF RECORD.

METAL DECK:

1. STEEL ROOF DECK SHALL BE:
   - A. FINISHED AND GAGE AS SHOWN ON ROOF PLAN AS MANUFACTURED BY VULCRAFT/NUCOR OR APPROVED EQUAL.
   - B. MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE.
   - C. ROOF DECK MUST COMPLY WITH STEEL DECK INSTITUTE STANDARDS. ALL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS.
2. ALL ROOF DECK TO BE DESIGNED, MANUFACTURED, AND INSTALLED IN ACCORDANCE WITH LATEST FACTORY MUTUAL STANDARDS.

STRUCTURAL STEEL:

1. CONFORM TO AISC MANUAL OF STEEL CONSTRUCTION, 14TH EDITION.
2. MATERIALS:
ASTM A 992, FY=50 KSI IN GENERAL.
ASTM A 500, GRADE B, FY=46 KSI FOR STRUCTURAL TUBING.
ASTM A 501, FY=36 KSI FOR PIPES.
ASTM A 325, TYPE I, FOR HIGH STRENGTH BOLTS.
ASTM A 307, GRADE A, ANCHOR BOLTS.
ASTM A 36, FY=36 KSI FOR PLATES, BARS, RODS, AND ANGLES.
ASTM A 563, HEAVY HEX NUTS.
ASTM A 436, HARDENED STEEL WASHERS.
3. THE DESIGN OF CONNECTIONS FOR ANY PORTION OF THE STRUCTURE NOT INDICATED ON THE DRAWINGS SHALL BE DESIGNED BY THE FABRICATOR AS FOLLOWS:
   - A. STANDARD CONNECTIONS SHALL BE USED WHERE POSSIBLE.
   - B. ALL SHOP CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED. FIELD CONNECTIONS SHALL BE HIGH STRENGTH BOLTED WHERE POSSIBLE.
   - C. UNLESS NOTED, BOLTS SHALL BE 3/4" DIAMETER ASTM A-325 TYPE N BEARING CONNECTIONS. BOLTS SHALL BE "SNUG TIGHT."
4. MEMBERS SUPPORTING DECK AT THE PERIMETER OF THE BUILDING SHALL BE CONTINUOUS EXCEPT AT EXPANSION JOINT. BUTT WELD CONTINUOUS MEMBERS PLACED END TO END.
5. STRUCTURAL STEEL SHALL RECEIVE ONE SHOP COAT OF RUST INHIBITIVE PRIMER UNLESS THE STEEL IS TO RECEIVE SPRAY ON CEMENTITUOUS FIREPROOFING. SEE ARCHITECTURAL DRAWINGS. ALL EXPOSED STEEL SHALL BE HOT DIPPED GALVANIZED OR COATED WITH CORROSION INHIBITED PAINT.
6. STEEL COLUMNS AND BASE PLATES SHALL HAVE MINIMUM 3" CONCRETE COVER PROTECTION.

OPEN WEB STEEL JOISTS:

1. ALL JOISTS SHALL BE FABRICATED WITH STANDARD CAMBER AND ERECTED IN ACCORDANCE WITH STEEL JOIST INSTITUTE, LATEST REVISION.
2. K-SERIES STEEL JOISTS SHALL BE CONNECTED TO STEEL BY 1/8" WELD, 1-1/2" EACH SIDE or (2) 1/2" DIAMETER BOLTS.
3. EXTEND LOWER JOIST CHORD AT ALL COLUMNS PER OSHA REQUIREMENTS. EXTEND AFTER DEAD LOADS ARE APPLIED.
4. HORIZONTAL BRIDGING SHALL BE AN ANGLE AT TOP AND BOTTOM, DESIGNED FOR L/r=300 OR LESS (U.N.).
5. CROSS BRIDGING SHALL BE AN ANGLE DESIGNED FOR L/r=200 OR LESS UNLESS NOTED.
6. BOTTOM CHORD OF ROOF JOIST SHALL BE DESIGNED FOR NET UPLIFT OF 14 PSF.
7. UNLESS NOTED K-SERIES STEEL JOIST SHALL HAVE 2 1/2" DEEP BEARING. WHERE STEEL JOIST OR GIRDER SLOPE EXCEEDS 1/4" PER FT., PROVIDE SLOPED BEARING.
8. ALL JOIST SHALL RECEIVE A COAT OF RUST INHIBITIVE PRIMER. FINISH JOIST PER ARCHITECT.
9. STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED TO THE REQUIREMENTS OF THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE FOR SERIES K JOISTS.
10. MANUFACTURER SHALL BE A MEMBER OF THE STEEL JOIST INSTITUTE. PROVIDE BRIDGING IN ACCORDANCE WITH SJI STANDARDS UNLESS NOTED OTHERWISE ON THE DRAWINGS.

COLD FORMED METAL FRAMING (METAL STUDS AND JOISTS):

1. DESIGN, FABRICATIONS AND ERECTION SHALL CONFORM TO AISI "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS", LATEST EDITION. ALL METAL STUDS SHALL BE GALVANIZED.
2. ALL 43 MIL (18 GAUGE) AND 33 MIL (20 GAUGE) STUDS, JOISTS, TRACK, BRIDGING ENDCLOSURES AND ACCESSORIES SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF AISI "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" WITH A MINIMUM YIELD OF 33,000 PSI (U.N.O.).
3. ALL MATERIAL AND ACCESSORIES SHALL BE FORMED FROM STEEL HAVING A G-60 GALVANIZED COATING, MEETING ASTM A 653.
4. UNLESS NOTED, ALL SCREWS OR PINS SHALL BE NON CORROSIVE NO. 8-18 (D=.125") OR LARGER (DO NOT USE STAINLESS STEEL OR COPPER COATED FASTENERS).
5. UNLESS NOTED, TRACKS SHALL BE SAME DEPTH AS STUDS OR JOISTS AND EQUAL OR THICKER GAUGE THAN STUDS OR JOISTS. TRACKS SHALL BE CONNECTED TO SUPPORTS AT 16" MAX. STUDS OR JOISTS SHALL BE CONNECTED TO TRACKS AT EACH SIDE.
6. THE QUANTITY OF STUDS AND JOISTS DISPLACED OR CUT FOR OPENING SHALL BE PLACED HALF ON EACH SIDE OF OPENING PER METAL STUD HEADER SCHEDULE ON TYPICAL DETAIL SHEET.
7. SEE FRAMING NOTES THIS SHEET.

COMPONENT AND CLADDING ROOF WIND PRESSURES (PSF)

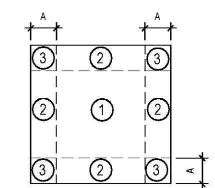
ROOF ZONES \ EFFECTIVE WIND AREA	V_{ASD} = 108 MPH, EXP. C RISK CATEGORY II, GCpi = +/- 0.18			
	10	20	50	100
1	+11.1/-27.1	+10.3/-26.4	+9.5/-25.5	+8.7/-24.7
2	+11.1/-45.4	+10.3/-40.5	+9.5/-34.2	+8.7/-29.3
3	+11.1/-68.4	+10.3/-56.6	+9.5/-41.2	+8.7/-29.3

1. EDGE DISTANCE A = 5.0 FEET

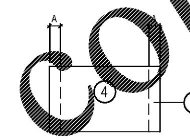
COMPONENT AND CLADDING WALL WIND PRESSURES (PSF)

WALL ZONES \ EFFECTIVE WIND AREA	V_{ASD} = 108 MPH, EXP. C RISK CATEGORY II, GCpi = +/- 0.18			
	10	20	50	100
4	+27.1/-29.3	+25.9/-28.1	+24.2/-26.5	+23.0/-25.4
5	+27.1/-36.2	+25.9/-33.8	+24.2/-30.6	+23.0/-28.1

1. EDGE DISTANCE A = 5.0 FEET



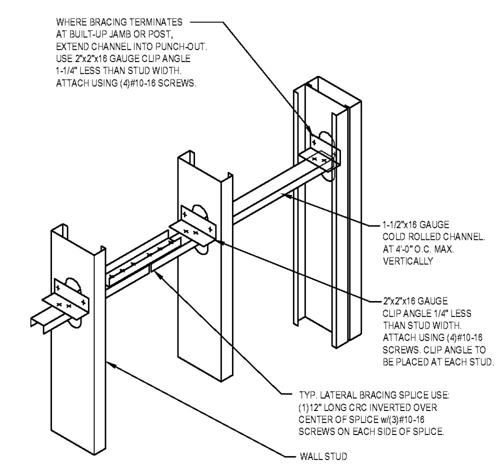
PLAN



ELEVATION

METAL STUD FRAMING NOTES:

1. EXTERIOR METAL STUD FRAMING:
   - A. METAL STUDS SHALL BE: 6" X 1-5/8 X 18GA. METAL STUDS AT 16" O.C.
   - B. SEE ARCH FOR LOCATION AND EXTENTS OF NEW METAL STUD FRAMING.
2. CONNECTIONS:
   - A. WHERE SHOWN, AND U.N.O. PROVIDE CONT. 18 GA. METAL TRACKS. ATTACH TO STUD FLANGE EA. SIDE W/ #10 TEK SCREW, TYP.
   - B. U.N.O. STUD WEB TO STUD WEB TO WEB CONNECTIONS SHALL BE WITH (4) #10 TEK SCREWS.
   - C. U.N.O. STUD CLIP CONNECTION TO STRUCTURAL STEEL SHALL BE WITH 3X3X3" LONG X 16GA CLIP FOR 3 5/8" STUDS AND 4X4X4" LONG X 16 GA. FOR LARGER SIZE STUDS, W/ (4) O.145 PAF TO STRUCTURAL STEEL, AND (4) #10 TEK SCREWS TO STUD, TYP.
   - D. U.N.O. STUD CLIP CONNECTION TO C.M.U. OR CONCRETE SHALL BE WITH 3X3X3" LONG X 16GA CLIP FOR 3 5/8" STUDS AND 4X4X4" LONG X 16GA CLIP FOR LARGER SIZE STUDS, W/ (4) 1/4"Ø X 1 3/4" EMBED. TAPCONS TO C.M.U. OR CONCRETE, AND (4) #10 TEK SCREWS TO STUD, TYP.
   - E. U.N.O. STUD CLIP CONNECTION TO STUD SHALL BE WITH 3X3X3" LONG X 16GA CLIP FOR 3 5/8" STUDS AND 4X4X4" LONG X 16GA. CLIP FOR LARGER SIZE STUDS, W/ AND (4) #10 TEK SCREWS TO STUD EA. LEG, TYP.
   - F. SEE BRIDGING AND BRACING DETAILS THIS SHEET.
4. SHEATHING:
   - A. SEE ARCH DRAWINGS FOR LOCATIONS AND EXTENTS
   - B. ATTACH 5/8" CDX PLYWOOD TO STUDS W/ #10 X 1 1/2" TEK SCREWS AT 6" O.C AT PANEL EDGES, AND AT 12" O.C. IN FIELD.
   - C. ATTACH 5/8"" DENSGLASS TO METAL STUDS W/#8 X 1-1/4" BUGLE HEAD SCREWS AT 8" O.C. AT PANEL EDGES, AND AT 8" O.C. IN FIELD.



TYPICAL LT. GAUGE LATERAL BRACING

NOTE: DETAIL IS NOT REQUIRED IF BOTH FLANGES OF METAL STUDS ARE SHEATHED

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PROJECT NAME
7-ELEVEN #41795 - OCALA, FL
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OCALA, FL 34470

SHEET TITLE
STRUCTURAL NOTES

This item has been electronically signed and sealed by Juan B Cano, P.E. on 12/16/2020 using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

JUAN B. CANO, P.E.
FLORIDA LICENSE NO. 69377

RELEASE

PROJECT NO.	DATE	DRAWN	CHECKED
2200355	12/17/2020	REE	JBC

S001