

GENERAL NOTES:

(THESE SPECIFICATIONS ARE IN ADDITION TO AND DO NOT EXCLUDE ANY FOUND IN THE GENERAL SPECIFICATIONS FOR THE PROJECT)

1. THE CONTRACT STRUCTURAL DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION...

2. SHOP DRAWINGS FOR STRUCTURAL STEEL, DECKING, AND WOOD TRUSSES SUBMITTALS SHALL COMPLY WITH THE FOLLOWING:

A. CONTRACTOR SHALL FURNISH COMPLETE AND DETAILED SHOP DRAWINGS PREPARED UNDER SUPERVISION OF A REGISTERED STRUCTURAL ENGINEER...

B. INDICATE THE DATE OF THE STRUCTURAL DRAWINGS USED FOR SHOP DRAWING PREPARATION.

C. INDICATE WELDS BY STANDARD AWS SYMBOLS AND SHOW SIZE LENGTH AND TYPE OF EACH WELD.

D. PROVIDE SETTING DRAWINGS, TEMPLATES AND DIRECTIONS FOR INSTALLATION OF ANCHOR BOLTS AND OTHER ANCHORAGES TO BE INSTALLED BY OTHERS.

E. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS PRIOR TO SUBMITTAL FOR ENGINEERING REVIEW.

F. CONTRACTOR SHALL HAVE AN APPROVED SET OF STRUCTURAL STEEL SHOP DRAWINGS AND PROOF OF WELDER CERTIFICATION AT THE JOBSITE AT ALL TIMES.

G. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

H. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR BUILDING LOCATION AND ORIENTATION. COORDINATE ALL DIMENSIONS WITH ARCH. DRAWINGS. DO NOT SCALE DRAWING.

I. SECTIONS CUTS INDICATED ON THE DRAWINGS APPLY TO ALL LIKE AND SIMILAR CONDITIONS EVEN THOUGH NOT SPECIFICALLY MARKED ON THE PLANS. COORDINATE SIMILAR CONDITIONS WITH ARCHITECTURAL, MECHANICAL, AND CIVIL DRAWINGS.

3. DESIGN LOADS: THE BUILDING STRUCTURE DESCRIBED IN THESE PLANS SHALL BE CONSTRUCTED IN COMPLIANCE WITH THE 2018 GEORGIA STATE BUILDING CODE w/ ALL AMENDMENTS. USE ASCE 7-16 CHAPTER 2 FOR ALL LOAD COMBINATIONS AND LOADS NOT INDICATED HEREIN.

A. GRAVITY LOADS

DEAD LOADS:

ROOF: 18 PSF

LIVE LOADS:

ROOF: 20 PSF (REDUCED PER CODE)

B. SNOW LOADS:

GROUND SNOW LOAD (Pg): 5 PSF

BALANCED ROOF SNOW LOAD (Ph+RAIN): 9 PSF

SNOW EXPOSURE FACTOR (Ce): 0.9

SNOW IMPORTANCE FACTOR (Is): 1.0

THERMAL FACTOR (Ct): 1.0

C. WIND LOADS:

BASIC WIND SPEED(3 SEC. GUST): 108 MPH

WIND IMPORTANCE FACTOR (Iw): 1.0

RISK CATEGORY: II

EXPOSURE CATEGORY: B

INTERNAL PRESSURE (GCpf): +/-0.18

REFER TO ASCE-7-16 FOR COMPONENT & CLADDING LOADS

D. SEISMIC DESIGN CRITERIA:

SEISMIC IMPORTANCE FACTOR (Ie): 1.25

RISK CATEGORY: II

MAPPED SPECTRAL RESPONSE ACCELERATIONS:

Ss: 0.291

S1: 0.097

SITE CLASS: D (ASSUMED)

SDS1: 0.304g

SD1: 0.154g

SITE COEFFICIENT

Fa = 1.587

Fv = 2.4

SEISMIC DESIGN CATEGORY: C

BASIC SEISMIC FORCE RESISTING SYSTEM:

LIGHT FRAME (WOOD) WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE AND STRUCTURAL STEEL NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

SEISMIC RESPONSE COEFFICIENT (Cs): 0.0821

RESPONSE MODIFICATION FACTORS (R): 3.00

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

FOUNDATION NOTES:

1. SEE PROJECT SPECIFICATION SECTION 03300 FOR CONCRETE STRENGTH REQUIREMENTS. EXTERIOR CONCRETE SHALL BE AIR ENTRAINED TO 6% +/- 1.12%

2. SEE ARCHITECTURAL DRAWINGS FOR SIDE WALK EXTENTS, PLANTER, AND PAVEMENT LOCATIONS, CONCRETE PADS STAIRS, SEE ARCH. FOR DIMENSIONS AT INTERIOR MASONRY PARTITIONS, AND DETAILS.

3. COORDINATE FINISHED FLOOR ELEVATIONS (F.F.E.) WITH ARCH. AND CIVIL DRAWINGS.

4. REFERENCE FFE =790.00' (SEE CIVIL).

5. FOUNDATION DESIGN IS BASED UPON THE REPORT OF GEOTECHNICAL EXPLORATION, DATED DECEMBER 14, 2019, BY AEI ENGINEERING INC. JOB #01-195030. CONTRACTOR SHALL FOLLOW ALL RECOMMENDATIONS MADE IN THE GEOTECHNICAL REPORT.

6. ALL FOUNDATION EXCAVATIONS SHALL BE EVALUATED BY THE GEOTECHNICAL ENGINEER OR TESTING AGENCY PRIOR TO PLACING ANY FOUNDATION CONCRETE. CONTACT STRUCTURAL ENGINEER IF THE ALLOWABLE SOIL BEARING PRESSURE IS LESS THAN 2000 PSF.

7. ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 1'-4" BELOW F.F.E. AND A MINIMUM OF 1'-4" BELOW ADJACENT EXTERIOR FINISH GRADE, UNLESS NOTED OTHERWISE

8. TOP OF INTERIOR FOOTINGS SHALL BE A MINIMUM OF 0'-8" BELOW F.F.E. COORDINATE TOP OF FOOTING ELEV. W/ PLUMBING AND LOWER FOOTING IF NECESSARY

9. PRIOR TO COMMENCING ANY FOUNDATION WORK, COORDINATE WORK WITH ANY EXISTING OR NEW UTILITIES. LOWER FOUNDATION AS REQUIRED TO AVOID INTERFERENCE WITH UTILITIES, EXCEPT WHERE ZERO LOADING FOOTINGS ARE LOCATED PARALLEL TO ADJACENT BUILDINGS. REFER TO THE FOUNDATION FOUNDATION PLAN FOR BOTH STEPS AT ADJACENT BUILDINGS - CONTACT ARCH/STRUCTURAL ENGINEER IF A CONFLICT OCCURS.

10. REFER TO THE GEOTECHNICAL REPORT AND SPECIFICATION FOR GENERAL RECOMMENDATIONS OF EARTHWORK, OVEREXCAVATION, SUBGRADE PREPARATION, FILL AND COMPACTION, WATERPROOFING, AND CONTINGENT REQUIREMENTS AND INFORMATION. IF THERE IS A CONFLICT BETWEEN GEOTECHNICAL REPORT AND STRUCTURAL PLANS THEN THE MORE STRINGENT CRITERIA SHALL APPLY UNLESS OTHERWISE DIRECTED BY THE ARCHITECT.

12. PROTECT PIPES AND CONDUITS RUNNING THROUGH WALLS AND SLABS WITH PROTECTIVE EXPANSION MATERIAL. LOWER CONTINUOUS FOOTINGS AND GRADE BEAMS PERPENDICULAR TO THE PLUMBING TO ALLOW PIPES TO PASS ABOVE THE FOOTINGS OR THROUGH THE GRADE BEAMS. ALTERNATIVELY, PROVIDE A CONCRETE JACKET IF PIPES ARE LOW ENOUGH TO BE PLACED BELOW THE FOOTINGS AND GRADE BEAMS PARALLEL TO PIPE RUNS TO AVOID SURCHARGE ONTO ADJACENT TRENCH EXCAVATIONS.

13. ARRANGE FOR OWNER'S INDEPENDENT TESTING AGENCY TO MONITOR CUT AND FILL OPERATIONS AND PERFORM FIELD DENSITY AND MOISTURE CONTENT TESTS TO VERIFY CONSTRUCTION AND APPROVE FOOTING SUBGRADES PRIOR TO PLACING CONCRETE.

CONCRETE SLAB NOTES:

1. SEE PROJECT SPECIFICATION SECTIONS 03300 FOR CONCRETE STRENGTH REQUIREMENTS.

2. WHERE SPECIFIC CONTROL JOINT LOCATIONS ARE NOT INDICATED, PROVIDE CONTROL CONSTRUCTION JOINTS SUCH THAT NO AREA EXCEEDS 144 SQUARE FEET NOR SHALL THE LENGTH OF ANY PANEL EXCEED 1.5 TIMES THE WIDTH. SEE DETAILS ON AND ARCHITECTURAL CONCRETE PLACEMENT PLAN. JOINT SPACING SHALL NOT EXCEED 12' ON CENTER.

3. CONDUITS AND PIPES EMBEDDED IN SLABS SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE-THIRD THE OVERALL THICKNESS OF THE SLAB. SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS OR WIDTHS ON CENTER. A MINIMUM SLAB THICKNESS OF 2 1/2" MUST BE MAINTAINED OVER THE EMBEDDED CONDUITS OR PIPES.

4. COORDINATE THE EXACT LOCATION AND EXTENTS OF ALL FLOOR SLOPES, RECESSED AREAS AND DRAIN LOCATIONS WITH ARCHITECTURAL AND PLUMBING DRAWINGS.

STEEL NOTES:

1. STRUCTURAL STEEL: A. SHALL CONFORM TO THE LATEST STANDARDS OF ASTM: WIDE FLANGE BEAMS: A992 OTHER STRUCTURAL STEEL SHAPES, PLATES AND BARS: A36 HOLLOW STRUCTURAL STEEL SECTIONS (ROUND AND RECTANGULAR): A500 GRADE B STRUCTURAL STEEL PIPE: A53 GRADE B

B. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE STANDARDS AND SPECIFICATIONS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC 360-16) USING ALLOWABLE STRESS DESIGN.

C. PROVIDE 1" (MINIMUM) NON-SHRINK 5000 PSI GROUT UNDER ALL BASE PLATES.

D. SHOP OR FIELD SPLICES BETWEEN SUPPORTS THAT ARE NOT REQUIRED BY DESIGN WILL NOT BE ALLOWED. ANY MEMBERS CONTAINING SUCH SPLICES FOUND IN THE FIELD SHALL BE REMOVED AND REPLACED WITH UNSPLICED MEMBERS AT THE FABRICATOR'S EXPENSE.

2. STEEL CONNECTIONS:

A. WHERE BEAM REACTIONS OR DETAILS ARE NOT SHOWN IN THE CONSTRUCTION DOCUMENTS, CONNECTIONS SHALL BE DESIGNED FOR ONE-HALF THE MAXIMUM (SIMPLE SPAN) UNIFORM LOAD WHICH THE MEMBER WILL SUPPORT FOR THE SPAN SHOWN ON THE DRAWINGS.

B. UNLESS LARGER REACTION IS SHOWN ON DRAWINGS, PROVIDE MINIMUM DESIGN FORCES AS FOLLOWS:

1. NON-COMPOSITE BEAMS: BEAM-TO-BEAM OR BEAM-TO-COLUMN CONNECTION TO DEVELOP THE REACTION OF CONNECTED BEAM. OBTAIN END REACTION FROM ALLOWABLE UNIFORM LOAD TABLES IN PART 2 OF THE AISC MANUAL OF STEEL CONSTRUCTION.

C. ADD TO REACTIONS LISTED ABOVE, LOADS OR REACTIONS OF MEMBERS SUPPORTED BY BEAM WITHIN 3 FEET OF BEAM END AND VERTICAL COMPONENTS OF FORCES IN BRACE MEMBERS FRAMING INTO BEAM.

D. BRACE CONNECTIONS SHALL BE IN ACCORDANCE WITH THE DETAILS SHOWN ON THE DRAWINGS. ANGLE SIZES, PLATE SIZES, AND SIZE AND LENGTHS OF WELDS SHALL BE DESIGNED IN ACCORDANCE WITH THE FOLLOWING:

- (1) DESIGN CONNECTIONS OF DIAGONAL MEMBERS TO DEVELOP THE LOADS SHOWN ON THE BRACE DETAILS. (2) WHERE FORCES ARE NOT INDICATED ON THE DETAILS, DESIGN CONNECTIONS OF DIAGONAL MEMBERS TO DEVELOP THE FULL TENSILE CAPACITY OF THE DIAGONAL MEMBER. (3) SIZE GUSSET PLATES AND ALL WELDS TO RESIST THE FORCE OF THE DIAGONAL MEMBERS, PLATES AND WELDS SHALL BE SIZED FOR TENSIONS, SHEARS, AND MOMENTS CAUSED BY CONCENTRIC AND ECCENTRIC FORCES. (4) ALL BRACE CONNECTIONS SHALL USE WELDS OR FULLY TENSIONED A325 CLASS A SLIP CRITICAL BOLTS. (5) BOLTS SHALL BE HIGH STRENGTH A-325 BOLTS OF SAME SIZE AND NUMBER AS SHOWN ON DRAWINGS. CONNECTIONS SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS. CONNECTIONS ARE BEARING TYPE. (6) BOLTS SHALL BE TIGHTENED TO THE SNUG TIGHT CONDITION UNLESS OTHERWISE NOTED ON THE DRAWINGS.

3. WELDS:

A. PROVIDE MINIMUM SIZE OF FILLET WELDS AS SPECIFIED IN TABLE J2.4 OF THE AISC MANUAL.

B. ALL WELDING SHALL CONFORM TO THE LATEST 'STRUCTURAL WELDING CODE' BY THE AMERICAN WELDING SOCIETY. ALL WORK SHALL BE PERFORMED BY CERTIFIED WELDERS EXPERIENCED IN THE TYPE OF CONSTRUCTION INVOLVED. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE.

C. DEVELOP THE FULL TENSILE STRENGTH OF THE MEMBER ELEMENT JOINED, ON ALL SHOP AND FIELD WELDS, UNLESS NOTED OTHERWISE ON THE DRAWINGS.

D. ALL WELDS ARE CONTINUOUS FOR THE FULL LENGTH OF THE CONNECTION UNLESS NOTED OTHERWISE ON DRAWINGS.

E. HEADED STUDS (SHEAR AND ANCHOR) AND DEFORMED ANCHORS:

PROVIDE HEADED STUDS (SHEAR AND ANCHOR) MADE OF MATERIAL CONFORMING TO ASTM A 108.

(1) PROVIDE DEFORMED ANCHORS MADE OF MATERIAL CONFORMING TO ASTM A 496.

(2) WELD STUDS ACCORDING TO MANUFACTURER'S RECOMMENDATIONS. MANUAL ARC (STICK) WELDING OF HEADED STUDS AND/OR DEFORMED ANCHORS IS NOT ALLOWED.

(3) DO NOT WELD ANYTHING TO HEADED STUDS

SPECIAL INSPECTION NOTES:

A. THE SPECIAL INSPECTOR SHALL BE ENGAGED BY THE OWNER. SPECIAL INSPECTOR SHALL BE FULLY QUALIFIED, APPROVED BY THE BUILDING OFFICIAL, REGISTERED BY APPLICABLE REGISTRATION BOARD IF REQUIRED BY THE LOCAL BUILDING OFFICIAL AND SHALL BE ACCEPTABLE TO THE ARCHITECT.

B. THE SPECIAL INSPECTOR SHALL PROVIDE VERIFICATION OF CONSTRUCTION QUALITY CONTROL INSPECTIONS AND TESTS. THE SPECIAL INSPECTOR SHALL CERTIFY THAT ALL WORK REQUIRING INSPECTION IS PERFORMED IN COMPLIANCE WITH ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS, BUILDING CODE REQUIREMENTS AND LOCAL BUILDING DEPARTMENT REQUIREMENTS.

C. SPECIAL INSPECTIONS ARE REQUIRED FOR THE ITEMS NOTED IN THE STATEMENT OF SPECIAL INSPECTIONS AND THE 2018 IBC CHAPTER 17. THE CONTRACTOR SHALL OBTAIN A COPY OF THE STATEMENT OF SPECIAL INSPECTIONS AND NOTIFY THE SPECIAL INSPECTOR WHEN WORK IS READY TO BE INSPECTED.

D. FAILURE TO NOTIFY THE SPECIAL INSPECTOR PRIOR TO OBTAINING AN ITEM REQUIRING INSPECTION MAY RESULT IN THE CONTRACTOR REMOVING OTHER WORK TO ALLOW INSPECTION. THIS WORK WILL BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE. FAILURE TO HAVE REQUIRED ITEMS INSPECTED IS REASON FOR REJECTION OF THE WORK.

E. PREMATURE NOTIFICATION FOR INSPECTIONS WILL RESULT IN ADDITIONAL INSPECTIONS WITH ALL EXPENSES AND FEES PAID FOR BY THE CONTRACTOR.

F. SEE SHEET SS-1 FOR SPECIAL INSPECTION CHECKLIST.

Table with 5 columns: ZONE, AREA (SF), MAX (+) (PSF), MAX (-) (PSF), REMARKS. Data for COMPONENT AND CLADDING (MAIN ROOF) showing zones 1, 2, and 3.

Table with 5 columns: ZONE, AREA (SF), MAX (+) (PSF), MAX (-) (PSF), REMARKS. Data for COMPONENT AND CLADDING (WALLS) showing zones 4 and 5.

WIND TABLE NOTES: 1. LOADS BASED ON ASCE 7-16 ULTIMATE LOADS 2. a = 13.4 FT

2 COMPONENTS & CLADDING WIND LOAD SCHEDULES S0.1 1/8" = 1'-0"

WWW.LDILine.com

ROBERTSON LOIA ROOF ARCHITECTS & ENGINEERS 3460 Preston Ridge Road, Suite 275, Alpharetta, GA, 30005 770.674.2600 / www.rlrc.com GA CCA # PEF00582 EXPIRES: 06/30/2022



CHIPOTLE AT CARTERSVILLE E. MAIN STREET CARTERSVILLE, GA 30121

Table with 2 columns: REVISIONS, Description of revisions.

STRUCTURAL GENERAL NOTES

DATE: 09/16/20 PROJECT NUMBER: 19303 SHEET NUMBER: S0.1