

GENERAL NOTES:

(THESE SPECIFICATIONS ARE IN ADDITION TO AND DO NOT EXCLUDE ANY FOUND IN THE GENERAL SPECIFICATIONS FOR THE PROJECT)

1. THE CONTRACT STRUCTURAL DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION...

2. SHOP DRAWINGS FOR STRUCTURAL STEEL, DECKING, AND WOOD TRUSSES SUBMITTALS SHALL COMPLY WITH THE FOLLOWING:

A. CONTRACTOR SHALL FURNISH COMPLETE AND DETAILED SHOP DRAWINGS PREPARED UNDER SUPERVISION OF A REGISTERED STRUCTURAL ENGINEER...

B. INDICATE THE DATE OF THE STRUCTURAL DRAWINGS USED FOR SHOP DRAWING PREPARATION

C. INDICATE WELDS BY STANDARD AWS SYMBOLS AND SHOW SIZE LENGTH AND TYPE OF EACH WELD.

D. PROVIDE SETTING DRAWINGS, TEMPLATES AND DIRECTIONS FOR INSTALLATION OF ANCHOR BOLTS AND OTHER ANCHORAGES TO BE INSTALLED BY OTHERS.

E. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS PRIOR TO SUBMITTAL FOR ENGINEERING REVIEW.

F. CONTRACTOR SHALL HAVE AN APPROVED SET OF STRUCTURAL STEEL SHOP DRAWINGS AND PROOF OF WELDER CERTIFICATION AT THE JOBSITE AT ALL TIMES.

G. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

H. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR BUILDING LOCATION AND ORIENTATION. COORDINATE ALL DIMENSIONS WITH ARCH. DRAWINGS. DO NOT SCALE DRAWING.

I. SECTIONS CUTS INDICATED ON THE DRAWINGS APPLY TO ALL LIKE AND SIMILAR CONDITIONS EVEN THOUGH NOT SPECIFICALLY MARKED ON THE PLANS. COORDINATE SIMILAR CONDITIONS WITH ARCHITECTURAL, MECHANICAL, AND CIVIL DRAWINGS.

3. DESIGN LOADS: THE BUILDING STRUCTURE DESCRIBED IN THESE PLANS SHALL BE CONSTRUCTED IN COMPLIANCE WITH THE 2018 GEORGIA STATE BUILDING CODE WITH ALL AMENDMENTS...

A. GRAVITY LOADS

DEAD LOADS:

ROOF: 20 PSF

LIVE LOADS:

ROOF: 20 PSF (REDUCED PER CODE)

B. SNOW LOADS

GROUND SNOW LOAD (Pg): 5 PSF

BALANCED ROOF SNOW LOAD (P+r): 9 PSF

SNOW EXPOSURE FACTOR (Ce): 0.9

SNOW IMPORTANCE FACTOR (Is): 1.0

THERMAL FACTOR (Ct): 1.0

C. WIND LOADS

BASIC WIND SPEED (3 SEC. GUST): 106 MPH

WIND IMPORTANCE FACTOR (Iw): 1.0

RISK CATEGORY: II

EXPOSURE CATEGORY: B

INTERNAL PRESSURE (Gcpi): +/- 0.18

REFER TO ASCE-7-16 FOR COMPONENT & CLADDING LOADS

D. SEISMIC DESIGN CRITERIA

SEISMIC IMPORTANCE FACTOR (Ie): 1.25

RISK CATEGORY: II

MAPPED SPECTRAL RESPONSE ACCELERATIONS:

Ss: 0.169

S1: 0.083

SITE CLASS: D (ASSUMED)

S(D1): 0.183g

S(D2): 0.133g

SITE COEFFICIENT

Fa = 1.6

Fv = 2.4

SEISMIC DESIGN CATEGORY: C

BASIC SEISMIC FORCE RESISTING SYSTEM:

LIGHT-FRAMED WALLS WITH SHEAR PANELS

SEISMIC RESPONSE COEFFICIENT (Cs): 0.0258

RESPONSE MODIFICATION FACTORS (R): 7.00

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE

FOUNDATION NOTES:

1. SEE PROJECT SPECIFICATION SECTION 03300 FOR CONCRETE STRENGTH REQUIREMENTS. EXTERIOR CONCRETE SHALL BE AIR ENTRAINED TO 6% +/- 1 1/2%

2. SEE ARCHITECTURAL DRAWINGS FOR SIDE WALK EXTENTS, PLANTER, AND PAVER LOCATIONS, CONCRETE PADS STAIRS, SEE ARCH. FOR DIMENSIONS AT INTERIOR MASONRY PARTITIONS, AND DETAILS.

3. COORDINATE FINISHED FLOOR ELEVATIONS (F.F.E.) WITH ARCH. AND CIVIL DRAWINGS.

4. REFERENCE FFE = 0.0'

5. FOUNDATION DESIGN IS BASED UPON ASSUMED ALLOWABLE BEARING PRESSURE OF 2600 PSF.

6. ALL FOUNDATION EXCAVATIONS SHALL BE EVALUATED BY A GEOTECHNICAL ENGINEER OR TESTING AGENCY PRIOR TO PLACING ANY FOUNDATION CONCRETE. CONTACT STRUCTURAL ENGINEER IF THE ALLOWABLE SOIL BEARING PRESSURE IS LESS THAN 2600 PSF.

7. TOP OF INTERIOR FOOTINGS SHALL BE A MINIMUM OF 0-8" BELOW F.F.E. COORDINATE TOP OF FOOTING ELEV. W/ PLUMBING AND LOWER FOOTING IF NECESSARY.

8. PRIOR TO COMMENCING ANY FOUNDATION WORK, COORDINATE WORK WITH ANY EXISTING OR NEW UTILITIES. LOWER FOUNDATION AS REQUIRED TO AVOID INTERFERENCE WITH UTILITIES. EXCEPT WHERE ZERO LOT LINE FOOTINGS ARE LOCATED PARALLEL TO ADJACENT BUILDINGS...

9. PROTECT PIPES AND CONDUITS RUNNING THROUGH WALLS AND SLABS WITH 1/2" INCH DIMENSION MATERIAL. LOWER CONTINUOUS FOOTING AND GRADE BEAMS PERPENDICULAR TO PIPE RUNS TO ALLOW PIPES TO PASS ABOVE THE FOOTINGS OR THROUGH THE GRADE BEAMS...

10. ARRANGE FOR OWNER'S INDEPENDENT TESTING AGENCY TO MONITOR CUT AND FILL OPERATIONS AND PERFORM FIELD DENSITY AND MOISTURE CONTENT TESTS TO VERIFY COMPACTION AND PROVE FOOTING SURFACES PRIOR TO PLACING CONCRETE.

11. ALL LONGITUDINAL REBARS IN THE WALL FOOTINGS SHALL BE CONTINUOUS AND LAPLACED AS SPECIFIED. CONTINUE ALL HORIZONTAL REBARS AT BENTS AND CORNERS BY BENDING THE REBARS 90 DEGREES AROUND THE CORNERS OR ADDING MATCHING CORNER BARS...

CONCRETE SLAB NOTES:

1. SEE PROJECT SPECIFICATION SECTIONS 03300 FOR CONCRETE STRENGTH REQUIREMENTS.

2. WHERE SPECIFIC CONTROL JOINT LOCATIONS ARE NOT INDICATED, PROVIDE CONTROL/CONSTRUCTION JOINTS SUCH THAT NO AREA EXCEEDS 144 SQUARE FEET NOR SHALL THE LENGTH OF ANY PANEL EXCEED 15 TIMES THE WIDTH. SEE DETAILS ON AND ARCHITECTURAL CONCRETE PLACEMENT PLAN. JOINT SPACING SHALL NOT EXCEED 12' ON CENTER.

3. CONDUITS AND PIPES EMBEDDED IN SLABS SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE-THIRD THE OVERALL THICKNESS OF THE SLAB. SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS OR WIDTHS ON CENTER. A MINIMUM SLAB THICKNESS OF 2 1/2" MUST BE MAINTAINED OVER THE EMBEDDED CONDUITS OR PIPES.

4. COORDINATE THE EXACT LOCATION AND EXTENTS OF ALL FLOOR SLOPES, RECESSED AREAS AND DRAIN LOCATIONS WITH ARCHITECTURAL AND PLUMBING DRAWINGS.

STEEL NOTES:

1. MATERIALS:

A. STRUCTURAL STEEL

WIDE FLANGE STEEL (COLUMNS/BEAMS): ASTM A992
OTHER STRUCTURAL STEEL SHAPES, PLATES AND BARS: ASTM A36
HOLLOW STRUCTURAL STEEL SECTIONS (ROUND AND RECTANGULAR): A500 GRADE B
ANCHOR RODS: ASTM F1554 GRADE 36
WELD ELECTRODES: SERIES E70
EXPANSION BOLTS: HILTI "KWIK BOLTS" OR APPROVED EQUAL.

2. SPECIFICATIONS: WELDING PERSONNEL AND PROCEDURES ARE TO BE QUALIFIED PER AWS D1.1-10. UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION AND ERECTION TO BE GOVERNED BY (ALL CODES - LATEST EDITION)

A. AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS.
B. AISC CODE OF STANDARD PRACTICE.
C. STRUCTURAL WELDING CODE, AWS D1.1- OF THE AMERICAN WELDING SOCIETY.

3. CONNECTIONS:

CONNECTIONS TO BE DESIGNED BY THE FABRICATOR TO DEVELOP FULL STRENGTH OF MEMBER OR FORCE SHOWN ON THE PLANS, WHICHEVER GOVERNS. FOLLOW INSTRUCTIONS ON DRAWINGS FOR GENERAL ARRANGEMENT OR PARTICULAR DETAILS.

4. GALVANIZING: ALL SHELF ANGLES, LINTELS IN EXTERIOR WALLS, AND ALL EXTERIOR STEEL EXPOSED TO THE ELEMENTS SHALL BE HOT DIP GALVANIZED

5. MISCELLANEOUS:

A. PROVIDE HOLES FOR OTHERS. IF OPENING IS NOT SHOWN ON THE STRUCTURAL DRAWINGS, OBTAIN PRIOR APPROVAL
B. STEEL SUPPORTING OR CONNECTED TO HVAC AND OTHER EQUIPMENT AND ROOF OPENINGS AS SHOWN ON THE DRAWINGS IS SHOWN FOR BIDDING PURPOSES ONLY. CONTRACTOR SHALL RECONCILE EXACT SIZE AND LOCATION BEFORE PROCEEDING WITH HIS WORK.
C. GROUT UNDER BEARING PLATES, BASE PLATES, AND SETTING PLATES TO BE NON-SHRINKING TYPE.
D. STEEL BELOW GRADE TO BE PROTECTED BY A MIN. OF 3 INCHES OF CONCRETE.
E. PROVIDE HEAVY WASHER AT ALL ANCHOR BOLTS.
F. EMBEDMENT LENGTH OF EXPANSION BOLTS IN TO SOLID MASONRY OR CONCRETE SHALL BE AS FOLLOWS:
1/2 INCH DIAMETER BOLTS -- 3 1/2 INCHES EMBEDMENT
3/4 INCH DIAMETER BOLTS -- 5 INCHES EMBEDMENT

STRUCTURAL LUMBER:

1. A. STUDS: STRUCTURAL LUMBER: DOUGLAS FIR-LARCH #2, OR APPROVED EQUAL:

Table with columns: SIZE, Fb, Ft, Fv, Fc, E. Rows for 2x4, 2x6, 2x8, 2x10, 2x12.

A. ROOF/WALL: ORIENTED STRAND BOARD: STRUCTURAL 1, EXPOSURE 1, EXTERIOR GLUE FOR ROOF AND WALLS PANEL IDENTIFICATION INDEX 24/16 - 5/8 INCH OR 24/0 - 1/2 INCH (WITH PLYWOOD CLIPS AT ROOF).

B. ROOF/WALL: PLYWOOD: C-PLUGGED, STRUCTURAL 1, EXPOSURE 1, EXTERIOR GLUE FOR ROOF AND WALL PANEL IDENTIFICATION INDEX 24/16-5/8 INCH OR 24/0-1/2 INCH (WITH PLYWOOD CLIPS AT ROOF).

C. SILL PLATES: NO. 2 SPRUCE-PINE-FIR, OR EQUAL FC=675 PSI, FV=70 PSI, EI=1,200,000 PSI

2. SPECIFICATIONS: UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION AND ERECTION SHALL BE GOVERNED BY THE LATEST REVISIONS OF:

A. NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENINGS.
B. U.S. PRODUCT STANDARD PS-1 FOR SOFTWOOD PLYWOOD - CONSTRUCTION AND INDUSTRIAL.

3. CONNECTIONS:

A. JOISTS TO BEAMS - 16 GA. GALVANIZED STD. JOIST HANGERS, UNLESS SHOWN OTHERWISE

B. PLYWOOD TO ROOF TRUSSES OR RAFTERS - NAILED - USE 10d RING SHANK NAILS AT 6 INCHES O/C AT PANEL EDGES AND 12 INCHES O/C AT INTERMEDIATE SUPPORTS. PROVIDE PLYWOOD CLIPS AT MID-SPAN OF PLYWOOD BETWEEN SUPPORTS.

4. ALL STRUCTURAL WOOD TO BE SURFACED FOUR (4) SIDES (S-4-S) AND A MAXIMUM MOISTURE CONTENT OF 18 PERCENT.

5. ALL LUMBER AND PLYWOOD IN CONTACT WITH CONCRETE, STUCCO, MASONRY OR OTHER CELESTIAL MATERIALS SHALL BE TREATED WITH AN E.P.A. ACCEPTABLE WOOD PRESERVATIVE (SUCH AS "AQCC" - ALKALINE-COPPER-QUATERNARY COMPOUND) (AZOLE TYPE A & B).

6. ALL WOOD CONNECTORS SHALL BE GALVANIZED STEEL OR RUST-PROOF PAINTED STEEL (U.O.C.). ALL GALVANIZED METAL CONNECTORS IN CONTACT WITH TREATED WOOD (ITEM #8) SHALL BE "TRIPLE ZINC 0-180" GALVANIZED (1" FIELD WELDS (INTERIOR OR EXTERIOR) OF SUCH CONNECTORS SHALL BE WIRE BRUSH CLEANED AND RUST PROOF PAINTED).

7. MISCELLANEOUS:

A. USE ONE LINE OF SOLID BLOCKING OR CROSS BRIDGING AT 8'-0" O/C MAX. FOR ALL JOISTS AND RAFTERS, USE SOLID BLOCKING AT JOIST AND RAFTER BEARING.
B. USE SOLID BLOCKING AT MID-HEIGHT FOR ALL EXTERIOR STUD WALLS AND INTERIOR PARTITIONS.
C. USE DOUBLE STUDS UNDER BEAM AND Lintel BEARING, UNLESS SHOWN OTHERWISE.

PREFABRICATED WOOD TRUSSES:

1. MATERIALS:
A. LUMBER: SEE "STRUCTURAL LUMBER" SECTION FOR WOOD INFORMATION.
B. METAL CONNECTOR PLATES: GALVANIZED SHEET STEEL, ASTM A446 (LATEST EDITION) GRADE "A" COATING CLASS G60 PER ASTM A525 (LATEST EDITION), MANUFACTURED WITH HOLES, FRINGS, NOTCHES, OR PROXIS UNIFORMLY SPACED AND FORMED. SEE "STRUCTURAL LUMBER" SECTION FOR GALVANIZED CONNECTIONS FOR TREATED WOOD.

2. DESIGN CRITERIA

A. LOADING:
TOP CHORD LIVE LOAD: 20 PSF
TOP CHORD DEAD LOAD: 20 PSF + MECH EQUIP.
BOTTOM CHORD DEAD LOAD: 10 PSF
NET WIND UPLIFT: 10 PSF

DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER, REGISTERED IN THE STATE OF THIS PROJECT, PERMITTED IN THE STATE OF DESIGN, RETAINED BY THE TRUSS MANUFACTURER.

C. TOP DESIGNERS SHALL BE SIGNED AND SEALED BY THE ENGINEER RESPONSIBLE FOR THE TRUSS DESIGN. IN ADDITION, SIGNED AND SEALED DESIGN CALCULATIONS FOR THESE TRUSSES SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.

D. MEMBER SIZES SHOWN ARE MINIMUM SIZES.

E. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360.

F. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.

3. MISCELLANEOUS:
A. BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 1/2" O BOLTS AT 4'-0" O.C. BOLT WEB MEMBERS TOGETHER WITH 1/2" O BOLTS AT 2'-0" O.C. AT CONCENTRATED LOADS, UNLESS OTHERWISE SPECIFIED BY THE TRUSS DESIGN ENGINEER.

B. VERIFY ALL DIMENSIONS, ELEVATIONS AND SLOPES PRIOR TO MANUFACTURING. REPORT ANY DISCREPANCIES IMMEDIATELY TO THE ARCHITECT.

C. WOOD TRUSSES SHALL BE DESIGNED AND FABRICATED TO CONFORM TO THE GEOMETRY SHOWN ON THE DRAWINGS. WEB CONFIGURATIONS ARE TO BE DETAILED AS REQUIRED BY THE DESIGNER/FABRICATOR.

D. PROVIDE 2x4 BOTTOM CHORD BRIDGING AT A MAXIMUM SPACING OF 10'-0" O.C.

COLD FORMED STEEL FRAMING NOTES:

1. COLD FORMED STEEL FRAMING STRENGTH CRITERIA: (1) 16 MIL-43 MIL - 33 KSI MIN. YIELD STRESS (2) 54 MIL-97 MIL - 50 KSI MIN. YIELD STRESS (3) RUNNER TRACK - 33 KSI MIN. YIELD STRESS (UNO)

2. ATTACH METAL FRAMING TO PRIMARY STRUCTURE WITH A DEFLECTION TRACK OR A STEEL NETWORK INC. VERTICAL DEFLECTION CLIPS CAPABLE OF ALLOWING 1" OF VERTICAL DEFLECTION IF WALL FRAMING IS SUPPORTED BY SLAB ON GRADE OR OTHER RIGID SUPPORT.

3. PLACE ALL COLD-FORMED STEEL STUD WALL BRIDGING HORIZONTALLY WITH A MAXIMUM VERTICAL SPACING OF FOUR FEET UNLESS NOTED OTHERWISE. AS AN OPTION, CONTINUOUS COLD-FORMED CHANNELS MAY BE POSITIONED THROUGH THE STUD PUNCH OUTS AS BRIDGING PROVIDED THE CHANNEL IS PROPERLY FASTENED TO EACH STUD. PROVIDE MANUFACTURERS RECOMMENDED BRIDGING TO BOTH FLANGES OF MEMBER IF FLANGE IS NOT PERMANENTLY BRACED WITH GYPSUM OR PLYWOOD SHEATHING.

4. PLUMB, ALIGN, AND SECURELY ATTACH STUDS TO THE FLANGES OF BOTH UPPER AND LOWER RUNNERS. SPLICES IN STUDS ARE NOT PERMITTED.

5. PROVIDE HEADERS AND SUPPORTING STUDS FOR FRAMING OF WALL OPENINGS PER MANUFACTURERS RECOMMENDATIONS BASED ON THE SPAN OF THE OPENING.

6. METAL STUD FASTENERS:

A. SCREW CONNECTIONS USE #10-16 KWIK-FLEX SELF-DRILLING SCREWS OR APPROVED EQUAL UNLESS OTHERWISE NOTED. 2 SCREWS PER CONNECTION MIN., UNLESS NOTED OTHERWISE OR A PRE-ENGINEERED TRUSS.

B. POWER DRIVEN FASTENERS: (1) FASTENING TO CONCRETE: USE 0.145" DIA. DOME HEAD NAIL TYPE "X-2P" BY HILTI OR APPROVED EQUAL UNLESS NOTED OTHERWISE. MIN. EMBEDMENT = 1 1/4" MIN. EDGE DISTANCE = 2" MIN. SPACING = 3" MAX. SPACING = 1' MIN. (2) FASTENING TO STEEL: USE 0.145" DIA. DOME HEAD KNURLED SHANK FASTENER TYPE "X-EDN" BY HILTI OR APPROVED EQUAL UNLESS NOTED OTHERWISE. EMBEDMENT = FULL PENETRATION MIN. EDGE DISTANCE = 1/2" MIN. SPACING = 1 1/2" MAX. SPACING 12" (3) PINS SHALL BE LOCATED 1/2" FROM OUTSTANDING LEG OF CLIP ANGLES WHERE ATTACHING TO STRUCTURAL STEEL ANGLES.

SPECIAL INSPECTION NOTES:

A. THE SPECIAL INSPECTOR SHALL BE ENGAGED BY THE OWNER. SPECIAL INSPECTOR SHALL BE FULLY QUALIFIED, APPROVED BY THE BUILDING OFFICIAL, REGISTERED BY APPLICABLE REGISTRATION BOARD IF REQUIRED BY THE LOCAL BUILDING OFFICIAL, AND SHALL BE ACCEPTABLE TO THE ARCHITECT.

B. THE SPECIAL INSPECTOR SHALL PROVIDE VERIFICATION OF CONSTRUCTION QUALITY THROUGH INSPECTIONS AND TESTING. THE SPECIAL INSPECTOR SHALL CERTIFY THAT ALL WORK REQUIRING INSPECTION IS IN CONFORMANCE WITH ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS, BUILDING CODE REQUIREMENTS AND LOCAL BUILDING DEPARTMENT REQUIREMENTS.

C. SPECIAL INSPECTIONS ARE REQUIRED FOR THE ITEMS NOTED IN THE STATEMENT OF SPECIAL INSPECTIONS AND THE 2018 IBC CHAPTER 17. THE CONTRACTOR SHALL OBTAIN A COPY OF THE STATEMENT OF SPECIAL INSPECTIONS AND NOTIFY THE SPECIAL INSPECTOR WHEN WORK IS READY TO BE INSPECTED.

D. FAILURE TO NOTIFY THE SPECIAL INSPECTOR PRIOR TO OBSERVING AN ITEM REQUIRING INSPECTION MAY RESULT IN THE CONTRACTOR REMOVING OTHER WORK TO ALLOW INSPECTION. THIS WORK SHALL BE REMOVED AND REPLACED AT THE CONTRACTORS EXPENSE. FAILURE TO HAVE REQUIRED ITEMS NOTED IS CONSIDERED A VIOLATION OF THE WORK.

E. PREMATURE INSPECTION FOR DEFECTS WILL RESULT IN ADDITIONAL INSPECTION WITH ALL EXPENSES AND FEES PAID FOR BY THE CONTRACTOR.

F. SEE SHEET SS 1 FOR SPECIAL INSPECTION CHECKLIST.

2 COMPONENTS & CLADDING WIND LOAD SCHEDULES

1/8" = 1'-0"

ROOF PRESSURE ZONES



WALL PRESSURE ZONES



COMPONENT AND CLADDING (MAIN ROOF)

Table with columns: ZONE, AREA (SF), MAX (+) (PSF), MAX (-) (PSF), REMARKS. Rows for Zone 1, 2, 3.

COMPONENT AND CLADDING (WALLS)

Table with columns: ZONE, AREA (SF), MAX (+) (PSF), MAX (-) (PSF), REMARKS. Rows for Zone 4, 5.

WIND TABLE NOTES:

1. LOADS BASED ON ASCE 7-16 ULTIMATE LOADS

2. s = 4 FT

ROBERISON LOIA ROOF ARCHITECTS & ENGINEERS logo and contact information.

Professional Engineer seal for Alfred P. McPeters, No. 25834, State of Georgia.

BURGER KING ARCHITECTS & CONSTRUCTION logo and contact information.

Revision table with columns: NO., DATE, DESCRIPTION.

STRUCTURAL GENERAL NOTES title block.

DATE: 11/03/2020

PROJECT NUMBER: 20197

SHEET NUMBER: S0.1