

STRUCTURAL ABBREVIATIONS

Table of structural abbreviations including ANCHOR BOLT, ARCHITECTURALLY EXPOSED STRUCTURAL STEEL, ABOVE FINISHED FLOOR, ALUMINUM, APPROXIMATE, ARCHITECTURAL ARCHITECT, AVERAGE BUILDING, BEARING, BOTTOM, BEARING, BETWEEN, CARTRIDGE, COLD-FORMED STEEL FRAMING, CAST-IN PLACE, CONTROL JOINT, CEILING, CLEAR, CMU, CONCRETE MASONRY UNIT, CONCRETE, CONCRETE ON CONSTRUCTION, CONTINUOUS, CENTER, DEFORMED BAR ANCHOR, DOUBLE, DIAMETER, DIAGONAL, DIMENSION, DRAWING, DRYWALL, EACH, EACH WAY, EXPANSION JOINT, ELEVATION, ELECTRICAL, ELEVATOR, EDGE OF DECK, EDGE OF SLAB, EDGE OF WALL, EACH WAY, EXISTING, EXPANSION, FLOOR PLAN, FINISHED FLOOR, FINISHED, FLOOR, FACE OF BRICK, FACE OF CONCRETE, FRAMING, FIRE RESISTANT TREATED, FOOT, GAUGE, GALVANIZED, GRADE BEAM, GENERAL CONTRACTOR, GRADE, HEAD, HOOK, HORIZONTAL.

DESIGN LOAD DATA

Table of design load data including classification of building risk category, floor live loads (uniform and concentrated), roof live loads, roof snow load, wind design data, seismic design data, and basic seismic force resistant system.

GENERAL

1. ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE 2018 (IBC). 2. THE STRUCTURAL DRAWINGS ARE INTENDED TO BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS AND THE DRAWINGS OF THE OTHER ENGINEERING DISCIPLINES. 3. THE CONTRACT DOCUMENTS ARE COMPLEMENTARY AND WHAT IS REQUIRED BY ONE SHALL BE AS BINDING AS IF REQUIRED BY ALL. IN THE CASE OF A CONFLICT, DISAGREEMENT, OR AMBIGUITY, PROVIDE THE BETTER QUALITY. IN THE CASE OF A CONFLICT, DISAGREEMENT, OR AMBIGUITY, PROVIDE THE GREATER QUANTITY OF WORK. 4. VERIFY AND COORDINATE MECHANICAL UNIT SUPPORTS AND OPENINGS WITH EQUIPMENT PURCHASED FOR THE PROJECT. COORDINATE REQUIREMENTS FOR SLEEVES, HANGERS, RISERS, AND ANCHORS AND ALL OTHER ITEMS TO BE SET IN STRUCTURAL WORK. SPECIAL INSPECTIONS ARE REQUIRED BY THE IBC SECTION 1704. REFER TO THE STATEMENT OF SPECIAL INSPECTION REQUIREMENTS FOR THIS PROJECT AND THE PROJECT SPECIFICATIONS REPORT FOR SPECIFIC INSPECTION REQUIREMENTS. REFER TO SPECIFICATION SECTION #1400 FOR GENERAL INSPECTION REQUIREMENTS.

FOUNDATIONS

1. FOUNDATIONS ARE DESIGNED TO BEAR ON ORIGINAL UNDISTURBED SOIL OR CONTROLLED COMPACTED FILL WITH AN ALLOWABLE BEARING CAPACITY OF 2,000 PSF, IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEERING REPORT PREPARED BY TERRACON DATED APRIL 13, 2020. 2. THE GEOTECHNICAL ENGINEER FOR THE OWNER'S TESTING AGENCY SHALL VERIFY BEARING CAPACITY AND SUITABILITY OF SUBGRADE PRIOR TO PLACING FOUNDATIONS AND GRADE SLABS. 3. SELECT AND PLACE CONTROLLED COMPACTED FILL UNDER DIRECT SUPERVISION OF THE GEOTECHNICAL ENGINEER FOR THE OWNER'S TESTING AGENCY. 4. FOOTING STEPS FOR UNDERSLAB UTILITIES INDICATED ON FOUNDATION PLANS SHALL BE CONSIDERED APPROXIMATE. COORDINATE FOOTINGS WITH ACTUAL LOCATION, SIZE AND DEPTH OF ALL UNDERGROUND PIPE (AND CONDUIT). REFER TO "FOOTING STEP" DETAIL TO STEP WALL FOOTING DOWN TO ALLOW UNDERSLAB PIPING TO PASS ABOVE THE FOOTING. ALTERNATELY, REFER TO "FOOTING SLEWIC" AND "PIPE TRENCH BACKFILL AT FOOTING" DETAILS TO ALLOW UNDERSLAB PIPING TO PASS BELOW THE TOP OF THE WALL FOOTING. 5. AVOID INFLUENCE OF PIPE TRENCH PARALLEL TO WALL FOOTING AND/OR ADJACENT TO COLUMN FOOTING. REFER TO "PIPE TRENCH DETAIL". 6. PROTECT FOOTINGS AND GRADE SLABS FROM FROST. HEAVE UNIL BUILDING IS PERMANENTLY ENCLOSED. 7. BRACE WALLS PLUMB WHICH ARE SUBJECTED TO UNBALANCED BACKL UNTIL PERMANENTLY STABILIZED BY STRUCTURE.

CONCRETE

1. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 - BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND ACI 308 - STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE. 2. CONCRETE SHALL BE NORMAL WEIGHT AND SHALL OBTAIN ULTIMATE 28 DAY COMPRESSIVE STRENGTH (F'c) AS FOLLOWS: FLOOR: 4,000 PSI; BEARING WALL SYSTEM: 4,000 PSI; INTERMEDIATE REINFORCED MASONRY SHEAR WALLS: 4,000 PSI. 3. ALL EXTERIOR CONCRETE SHALL BE AIR-ENTRAINED. 4. REINFORCING STEEL SHALL BE AS FOLLOWS: REINFORCING BARS: ASTM A615, GRADE 60, DEFORMED; WELDED WIRE FABRIC: ASTM A1064, SHEET TYPE ONLY; WELDED REINFORCING BARS: ASTM A796 LOW ALLOY STEEL REINFORCING BARS, DEFORMED; DEFORMED BAR ANCHORS (DBA): ASTM A1064, DEFORMED; WELDING PER AWS D1.4 STRUCTURAL WELDING CODE - REINFORCING STEEL.

FIBER REINFORCING

1. SYNTHETIC MACRO-FIBER REINFORCING MAY BE SUBSTITUTED FOR WELDED WIRE FABRIC IN SLAB ON GRADE AND IN SLAB ON COMPACTED FILL FLOOR. 2. DOSAGE RATE SHALL COMPLY WITH MANUFACTURER'S RECOMMENDATIONS, AND NOT LESS THAN 1 LB / CU YD. 3. FIBER SHALL BE ADDED AT THE CONCRETE BATCH PLANT.

FLOWABLE FILL

CONTROLLED LOW STRENGTH FILL (CLSF) SHALL BE REFERRED TO AS FLOWABLE FILL. MAY BE SUBMITTED FOR APPROVAL AS A CONTROLLED FILL AT FOUNDATION UNDERCUT LOCATIONS. THE CLSF MIXTURE SHALL BE PROPORTIONED TO PRODUCE AN UNCONFINED COMPRESSIVE STRENGTH OF 100 PSI MINIMUM TO 300 PSI MAXIMUM.

STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE FOLLOWING AISC DOCUMENTS: AISC 360 - SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS; AISC 358 - SPECIFICATION FOR STANDARD PRACTICE FOR STEEL BOLDS AND BRIDGES; AISC 355 - SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS. 2. STRUCTURAL STEEL SHALL COMPLY WITH THE FOLLOWING SPECIFICATIONS: WIDE FLANGE SHAPES AND ANGLES: ASTM A992 (Fy=50 KSI); MISCELLANEOUS SHAPES, PLATES & BARS (1/4" THICK): ASTM A572 (Fy=50 KSI); HOLLOW STRUCTURAL SECTIONS (HSS): ASTM A500, GRADE C (Fy=50 KSI); SQUARE & RECTANGULAR: ASTM A500, GRADE C (Fy=50 KSI); ROUND: ASTM A500, GRADE C (Fy=50 KSI); HIGH STRENGTH BOLTS (CONVENTIONAL): ASTM F1554, TYPE 1; ASTM F1554, TYPE 2; ASTM F1554, TYPE 3; HEAVY HEX NUTS: ASTM A563; TWIST OFF TENSION CONTROL BOLTS: ASTM F1928, GRADE 1155; ANCHOR RODS: ASTM F1554, OR 55 INCLUDE SUPPLEMENT S1; WELDING ELECTRODES: E7018 (LOW HYDROGEN); HEADED SHEAR STUDS: ASTM A615, TYPE 1; THREADED ROD: ASTM A36.

1. UNLESS NOTED OTHERWISE, CONNECTIONS SHALL BE DESIGNED IN ACCORDANCE WITH AISC MANUAL OF STEEL CONSTRUCTION, AS SIMPLE CONNECTIONS USING ALLOWABLE STRENGTH DESIGN (ASD). CONNECTIONS FOR NON-COMPOSITE BEAMS SHALL BE DESIGNED FOR THE UNIFORM LOAD CAPACITY INDICATED IN THE ALLOWABLE UNIFORM LOAD TABLES, PART 2, OF THE AISC MANUAL. CONNECTIONS FOR COMPOSITE BEAMS SHALL BE DESIGNED FOR THE REACTONS INDICATED ON THE PLANS. 2. BOLTED JOINTS SHALL BE "SNUG TIGHTENED", UNLESS OTHERWISE INDICATED. 3. WELDING SHALL BE IN ACCORDANCE WITH AWS D1.1 "STRUCTURAL WELDING CODE - STEEL". 4. WHERE STRUCTURAL STEEL IS EXPOSED BELOW GRADE, PROVIDE MINIMUM 3" CONCRETE COVER OR COAT WITH BITUMINOUS MASTIC. 5. STRUCTURAL STEEL EXPOSED TO WEATHER IN THE FINISHED WORK SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A153, UNLESS NOTED OTHERWISE. 6. ARCHITECTURALLY EXPOSED STRUCTURAL STEEL INDICATED THIS AESS, SHALL CONFORM TO THE REQUIREMENTS OF SECTION 10 OF THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES. ALL FABRICATION AND CONNECTIONS OF COMPOSITE EXPOSED AND VISIBLE IN THE FINISHED WORK SHALL BE MADE WITH CONTINUOUS WELDS. INTERMITTENT WELDS ARE ACCEPTABLE FOR NON-EXPOSED OR NON-VISIBLE LOCATIONS. WELD SIZE SHALL BE AS REQUIRED FOR STRUCTURAL STRENGTH BUT NOT LESS THAN 1/8" FILLER. HOLES BURNED THROUGH STRUCTURE DURING WELDING SHALL NOT BE ALLOWED. REPLACEMENT OF DRILL WILL BE REQUIRED.

STEEL JOISTS

1. ALL STEEL JOIST WORK SHALL CONFORM TO THE LATEST EDITION OF THE STEEL JOIST INSTITUTE (SJI) STANDARD SPECIFICATIONS. 2. STEEL JOISTS SHALL BE DESIGNED USING ALLOWABLE STRENGTH DESIGN. 3. PROVIDE JOIST BRIDGING IN ACCORDANCE WITH SJI SPECIFICATIONS, OSHA REQUIREMENTS, AND AS REQUIRED FOR JOIST DESIGN. 4. ROOF JOISTS AND BRIDGING SHALL BE DESIGNED FOR A NET UPLIFT OF 1X PSF. 5. SPECIAL JOISTS, INDICATED "SP" ON FRAMING PLAN, ARE REQUIRED FOR CRITICAL LOCATIONS. 6. DETAILING AND ERECTION OF OPEN WEB STEEL JOISTS SHALL COMPLY WITH OSHA REQUIREMENTS. 7. STEEL ROOF JOISTS SHALL BE PROVIDED WITH NO CAMBER. 8. DESIGN ALL JOISTS, NOS. L14 FOR 500 LB CONCENTRATED LOAD (SERVICE LOAD) AT ALL DIAGONAL STRUTS FROM JOIST CHORDS, UNLESS NOTED OTHERWISE. 9. TOP CHORD OF COMPOSITE JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 200 POUNDS DEAD LOAD, LOCATED AT ANY POINT ALONG JOIST LENGTH, IN ADDITION TO ALL OTHER LOADS. 10. BOTTOM CHORD OF COMPOSITE JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 200 POUNDS DEAD LOAD, LOCATED AT ANY POINT ALONG JOIST LENGTH, IN ADDITION TO ALL OTHER LOADS. 11. BOTTOM CHORD OF NOS. L14 AND L1 SERIES JOISTS SHALL BE DESIGNED FOR A POINT LOAD OF 500 POUNDS DEAD LOAD, LOCATED AT ANY POINT ALONG JOIST LENGTH, IN ADDITION TO ALL OTHER LOADS. REFER TO TYPICAL CONCENTRATED LOAD ON STEEL JOIST DETAIL FOR REQUIREMENTS REGARDING CRITICAL JOIST CHORD CONNECTIONS.

STEEL DECK

1. ALL STEEL DECK WORK SHALL CONFORM TO THE LATEST EDITION OF THE STEEL DECK INSTITUTE (SDI) DESIGN MANUAL FOR COMPOSITE DECKS, FLOOR DECKS AND ROOF DECKS; AND AMERICAN IRON AND STEEL INSTITUTE (AISI) "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS". 2. WELDING SHALL BE IN ACCORDANCE WITH AWS D1.3, "STRUCTURAL WELDING CODE - SHEET STEEL". 3. PERMANENT LOADS SHALL NOT BE SUSPENDED FROM STEEL ROOF DECK UNLESS APPROVED BY ENGINEER OF RECORD. 4. STEEL DECK SHALL BE INSTALLED WITH A MINIMUM OF 3 CONTINUOUS SPANS. UNLESS NOTED OTHERWISE, ANY LOCATION NOT MEETING THESE CONDITIONS SHALL BE SPECIFICALLY IDENTIFIED ON THE STEEL DECK SHOP DRAWINGS. 5. REFER TO "STEEL DECK SCHEDULE" FOR DECK TYPES AND FASTENING REQUIREMENTS.

CONCRETE MASONRY (CMU)

1. ALL MASONRY WORK SHALL CONFORM TO THE REQUIREMENTS OF TMS 602 - BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES WITH COMMENTARY AND TMS 602 - SPECIFICATIONS FOR MASONRY STRUCTURES WITH COMMENTARY. 2. NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY (F'c) SHALL BE 2,000 PSI, DETERMINED IN ACCORDANCE WITH THE UNIT STRENGTH METHOD, PER TMS 602 UNLESS NOTED OTHERWISE. 3. CONCRETE MASONRY UNITS (CMU) SHALL CONFORM TO ASTM C90, AND SHALL BE MADE WITH LIGHTWEIGHT AGGREGATE. 4. MORTAR FOR CMU SHALL CONFORM TO ASTM C270, TYPE S, UNLESS NOTED OTHERWISE. MORTAR FOR CMU SHALL CONFORM TO ASTM C270, TYPE M (2500 PSI) AT ALL INTERIOR AND EXTERIOR CMU SECURITY WALLS AND PARTITIONS. 5. GROUT SHALL CONFORM TO ASTM C970 AND SHALL BE PROPORTIONED TO OBTAIN MINIMUM ULTIMATE 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI. 6. PLACE GROUT IN ACCORDANCE WITH TMS 602. ALLOW A MINIMUM OF 24 HOURS FOR MASONRY TO SET PRIOR TO PLACING GROUT. 7. FILL JOINTS OF COMPOSITE WALLS SOLID WITH MORTAR AS THE WALLS PROGRESS. BOND WITHES OF COMPOSITE WALLS TOGETHER USING HORIZONTAL JOINT REINFORCING @ 16" ON CENTER, UNLESS NOTED OTHERWISE. 8. PROVIDE VERTICAL REINFORCING STEEL OF SIZE AND SPACING INDICATED. LAP SPlice LENGTHS SHALL BE AS FOLLOWS: #4 BAR AND SMALLER: 26 INCHES; #5 BAR: 34 INCHES; #6 BAR: 38 INCHES; #7 BAR: 45 INCHES. 9. PROVIDE POSITIONERS TO HOLD VERTICAL WALL REINFORCING STEEL IN PROPER ALIGNMENT. 10. REINFORCING STEEL SHALL COMPLY WITH ASTM A615, GRADE 60. 11. MASONRY WALLS OF HOLLOW UNITS WHICH CHAMFERED THICKNESS SHALL HAVE A CONTINUOUS SOLID OR GROUT FILLED COURSE BELOW THE TRANSITION. IF WALL THICKNESS IS GREATER ABOVE THE TRANSITION, THE COURSE ABOVE THE TRANSITION SHALL ALSO BE GROUTED SOLID. 12. FILL CMU CELLS WITH GROUT FROM TOP OF FOOTING TO TOP OF SLAB ON GRADE ELEVATION. 13. MASONRY WALL CONTROL JOINTS ARE NOT INDICATED ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL DRAWINGS FOR JOINT LOCATIONS AND DETAILS. COORDINATE JOINT LOCATIONS TO AVOID BEAM BEARING LOCATIONS AND SHEAR WALLS. DO NOT BREAK BOND BEAM REINFORCEMENT AT CONTROL JOINTS.

COLD FORMED STEEL FRAMING

1. ALL STRUCTURAL COLD FORMED STEEL FRAMING (CFSF) SHALL COMPLY WITH AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS". 2. CFSF (STRUCTURAL) INCLUDES ALL EXTERIOR WALLS, SOFFITS, BULKHEADS, TRUSSES, RAFTERS, JOISTS AND CEILING JOISTS (IF SELF-SUPPORTING). PROVIDE ENGINEERING DESIGN OF ALL CFSF'S, AND SUBMIT DESIGN CALCULATIONS, ERECTION DRAWINGS AND DETAIL DRAWINGS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF SOUTH CAROLINA. REFER TO SECTION 05000 FOR ADDITIONAL INFORMATION. 3. CFSF (STRUCTURAL) INCLUDES ALL EXTERIOR WALLS, SOFFITS, BULKHEADS, TRUSSES, RAFTERS, JOISTS AND CEILING JOISTS (IF SELF-SUPPORTING). PROVIDE ERECTION DRAWINGS AND DETAIL DRAWINGS. PROVIDE ENGINEERING DESIGN OF ALL PRE-FABRICATED CFSF'S TRUSSES AND PANELIZED WALL ASSEMBLIES, AND SUBMIT DESIGN CALCULATIONS AND SHOP DRAWINGS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF SOUTH CAROLINA. FOR THOSE SYSTEMS, REFER TO SECTION 05000 FOR ADDITIONAL INFORMATION. 4. CFSF (NON-STRUCTURAL) INCLUDES INTERIOR NON-LOAD BEARING STUD WALLS AND SUSPENDED CEILING FRAMING SYSTEM. REFER TO SECTION 09210 FOR ADDITIONAL INFORMATION. 5. ALL FRAMING MEMBERS, BRIDGING AND ACCESSORIES SHALL BE FORMED FROM STEEL SHEET HAVING A GALVANIZED COATING IN ACCORDANCE WITH ASTM A563. 6. ALL C- SHAPED FRAMING MEMBERS SHALL HAVE A MINIMUM FLANGE WIDTH OF 1/8 INCHES. 7. MINIMUM YIELD STRENGTH SHALL BE AS FOLLOWS: Fy = 33,000 PSI; 16 GAUGE AND 20 GAUGE; Fy = 50,000 PSI; 18 GAUGE, 14 GAUGE AND 12 GAUGE.

POST INSTALLED ANCHORS & BOWELS

1. INSTALL ALL ANCHORS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDED PROCEDURES AT NOT LESS THAN THE MINIMUM EDGE DISTANCES INDICATED IN THE MANUFACTURER'S LITERATURE. SUBMIT MANUFACTURER'S PRODUCT DATA FOR REVIEW. 2. ALL ANCHORS INCLUDING BROADCAST ROSS WASHERS SHALL BE ZINC PLATED IN ACCORDANCE WITH ASTM A663, FOR SERVICE IN CONCRETE. 3. SECURE ANCHORS SHALL BE ONE OF THE FOLLOWING: A. "ADHESIVE ANCHORS" OR "ADHESIVE DOWELS" SHALL BE INSTALLED IN SOLID CONCRETE SHALL UTILIZE ONE OF THE FOLLOWING ADHESIVE SYSTEMS, OR APPROVED EQUAL: HYBRID (FAST CURE): AC108A BY DEWALT; ACRYLIC TIC BY SIMPSON STRONG-TIE ANCHORING SYSTEMS; HT-HY 270, BY HLT. EPOXY (SLOW CURE): PURE 1100, BY DEWALT; SET 47 BY SIMPSON STRONG-TIE ANCHORING SYSTEMS; HT R 500-V3 EPOXY ADHESIVE, BY HLT. B. "ADHESIVE ANCHORS" INSTALLED IN SOLID GROUT FILLED CMU SHALL UTILIZE ONE OF THE FOLLOWING ADHESIVE SYSTEMS, OR APPROVED EQUAL: HT-HY 270, BY HLT; AC 100+ GOLD, BY DEWALT; ACRYLIC TIC BY SIMPSON STRONG-TIE ANCHORING SYSTEMS. C. SCREEN TUBE ANCHORS: INSTALLED IN HOLLOW CMU SHALL UTILIZE ONE OF THE FOLLOWING ADHESIVE SYSTEMS, OR APPROVED EQUAL: HT-HY 270, BY HLT; AC 100+ GOLD, BY DEWALT; ACRYLIC TIC BY SIMPSON STRONG-TIE ANCHORING SYSTEMS.

TEMPORARY SHORING

1. PROVIDE TEMPORARY SHORING AND BRACING TO MAINTAIN THE EXISTING STRUCTURE IN PROPER ALIGNMENT UNTIL PERMANENT CONSTRUCTION AND LATERAL BRACING IS IN PLACE. 2. THE TEMPORARY SHORING DIAGRAMS ARE CONCEPTUAL ONLY. DESIGN OF TEMPORARY SHORING SHALL BE PROVIDED BY THE CONTRACTOR. DESIGN CALCULATIONS AND SHORING DRAWINGS SHALL BE SUBMITTED FOR REVIEW AND SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF SOUTH CAROLINA. 3. CAREFULLY EVALUATE THE SITUATION WHICH EXISTS PRIOR TO COMMENCEMENT OF WORK. NOTIFY THE ARCHITECT IF ANY CONDITIONS ARE DETECTED WHICH MAY AFFECT THE STABILITY OF THE EXISTING STRUCTURE OR THE SHORING. 4. MONITOR THE PERFORMANCE OF THE TEMPORARY SHORING AT ALL TIMES DURING THIS WORK AND HAVE ADDITIONAL SHORING READILY AVAILABLE ON SITE IN THE EVENT OF DETECTION OR OTHER MOVEMENT OF THE SHORING. 5. REFER TO THE PHASING PLAN ON ARCHITECTURAL DRAWINGS. WORK SHALL BE COORDINATED TO PROHIBIT ACCESS TO FLOOR AREAS ABOVE THIS WORK DURING DEMOLITION AND UNTIL FINAL SUPPORT IS COMPLETED.

RENOVATION

1. EXISTING CONSTRUCTION INDICATED ON THE STRUCTURAL DRAWINGS IS BASED ON INFORMATION OBTAINED FROM THE ORIGINAL DESIGN DRAWINGS AND ON LIMITED OBSERVATIONS OF EXISTING CONDITIONS. THIS INFORMATION, INCLUDING STRUCTURAL COMPONENT TYPE, SIZE AND ORIENTATION HAS NOT BEEN CONFIRMED IN ALL CASES, AND MAY NOT MATCH AS-BUILT. EXISTING CONSTRUCTION AND EXISTING CONDITIONS AND DIMENSIONS RELAYING TO THE PROPOSED NEW WORK SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION AND CONSTRUCTION OF STRUCTURAL ELEMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT. 2. EXISTING CONSTRUCTION IS INDICATED USING A LIGHTER LINE WEIGHT THAN PROPOSED NEW CONSTRUCTION IN PLANS AND SECTIONS.

STRUCTURAL MATERIALS LEGEND

Legend for structural materials including Earth, Cast in Place Concrete, Clay Brick, Hollow Concrete Block, Split Face Concrete Block, GROUT FILLED CONCRETE BLOCK, Precast Concrete, and Porous Fill or Granular Base Course.

LEGEND FOR SECTION AND DETAIL MARKS

Legend for section and detail marks including Section and Detail (where drawn), Section where cut, and Detail where cut.

PLAN LEGEND

Legend for plan symbols including Centerline, Joist Bearing Elevation, Beam Bearing Plate, Column Base Plate, Wood Header, Wood Joist, Truss, Wood Post, Concrete Pier, Joist Substitute, Special Joist, Wall Footing Step, Top of Footing Elevation, Work Point, Top of Slab Elevation, Lintel, Column Footing, Top of Steel Bearing Elevation, Indicated Top of Steel, Indicated Top of Structural Member, Wall Footing, Thickened Slab, Steel Joist Bottom Chord Extension, Welded, Steel Beam Moment Connection, Existing, Transfer Force, CMU Wall Reinforcing Size and Spacing, Change in Slab Elevation.

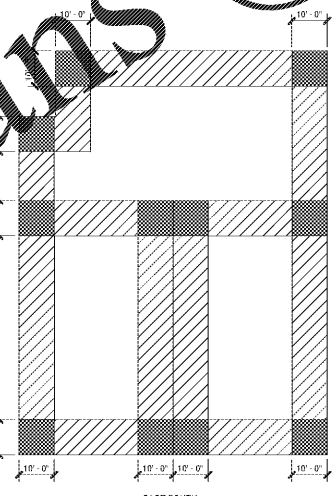


Table of Gross Wind Pressure for different zones: Zone 1: +16 PSF / -21 PSF; Zone 2: +16 PSF / -39 PSF; Zone 3: +16 PSF / -73 PSF.

ROOF WIND PRESSURE DIAGRAM NOTES: 1. ROOF IS ZONE 1, UNO. 2. ZONE 2 IS REPRESENTED BY [diagonal lines]. 3. ZONE 3 IS REPRESENTED BY [cross-hatch]. 4. (+) INDICATES PRESSURES ARE ACTING TOWARDS ROOF (INWARDS); (-) INDICATES PRESSURES ARE ACTING AWAY FROM (OUTWARDS). 5. PRESSURES INDICATED ARE GROSS PRESSURES FOR ALLOWABLE STRESS DESIGN PER ASCE 7 FOR AN EFFECTIVE AREA OF 10 SF OR LESS. THE EFFECTIVE WIND AREA SHALL BE IN ACCORDANCE WITH ASCE 7. 6. ROOF DEAD LOAD SHALL BE TAKEN AS 16 PSF FOR UPLIFT RESISTANCE.

STEEL DECK SCHEDULE

Table for Steel Deck Schedule: Deck Type 1, 1/4" x 26 GAUGE WIDE AISI 330 ROOF DECK, GALVANIZED. FASTEN TO ALL SUPPORTS WITH #5" DIAMETER RIBBLE WELDS AT 36" PATTERN, AND AT 1' OC AT ALL EDGES AND END LAPS. FASTEN SCALERS WITH #10 TEK SCREWS AT MID SPAN AND NOT GREATER THAN 36" OC. AT CFSF SUPPORT FRAMING, FASTEN WITH #10 TEK SCREWS 1/4" OC OF WELDS (SPACING TO MATCH WELDING SPACING NOTED ABOVE).

MOSELEY ARCHITECTS



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Table with columns: PROJECT NO., DATE, REVISIONS, DATE, DESCRIPTION.

GENERAL NOTES AND LEGENDS

S0.0.1

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