GENERAL NOTES

DESIGN:

1. BUILDING CODE – INTERNATIONAL BUILDING CODE – 2012 EDITION

2. DESIGN LIVE LOADS
 2.A.ROOF
 MINIMUM ROOF LIVE LOAD = 20.0 PSF
 GROUND SNOW LOAD = 5.0 PSF
 SNOW EXPOSURE FACTOR (C_E) = 1.0
 SNOW IMPORTANCE FACTOR (I_S) = 1.0
 FLAT ROOF SNOW LOAD (P_F) = 5.0 PSF
 TOTAL DESIGN LIVE LOAD = 20 PSF

3. DESIGN DEAD LOADS
 3.A.ROOF
 MEMBRANE ROOF = 2.0 PSF
 INSULATION = 2.0 PSF
 METAL DECK = 2.0 PSF
 JOISTS = 3.0 PSF
 MECHANICAL AND ELECTRICAL = 4.0 PSF
 FIRE PROTECTION = 2.0 PSF
 MISCELLANEOUS = 1.0 PSF
 TOTAL DEAD LOAD = 16.0 PSF

4. DESIGN WIND LOADS
 ULTIMATE WIND SPEED = 115 MPH
 WIND LOAD IMPORTANCE FACTOR (I_W) = 1.0
 EXPOSURE = C

5. SEISMIC PARAMETERS
 SEISMIC IMPORTANCE FACTOR (I_E) = 1.0
 SEISMIC SITE CLASSIFICATION = D (ASSUMED)
 SEISMIC DESIGN CATEGORY = C
 S_S = 0.179g S_1 = 0.088g
 S_{DS} = 0.191g S_{D1} = 0.141g

 SEISMIC–FORCE RESISTING SYSTEM:
 BEARING WALL SYSTEM: ORDINARY REINFORCED MASONRY
 DESIGN BASE SHEAR: 0.128W
 SEISMIC RESPONSE COEFFICIENT: 0.128
 RESPONSE MODIFICATION FACTOR: 2.0
 ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE PROCEDURE

COMPONENT AND CLADDING WIND LOAD SCHEDULE

EFFECTIVE WIND AREA (SQ. FT.)	ROOF: CORNER ZONE (PSF)	ROOF: EDGE ZONE (PSF)	ROOF: INTERIOR ZONE (PSF)	WALL: EDGE ZONE (PSF)	WALL: INTERIOR ZONE (PSF)
≤ 10	+10.0/–54.6	+10.0/–36.2	+10.0/–21.7	+21.7/–28.9	+21.7/–23.4
20	+10.0/–45.2	+10.0/–32.3	+10.0/–21.1	+20.6/–27.0	+20.6/–22.4
50	+10.0/–32.8	+10.0/–27.3	+10.0/–20.3	+17.0/–21.6	+17.0/–18.7
100	+10.0/–23.4	+10.0/–23.4	+10.0/–19.7	+18.4/–22.4	+18.4/–20.2
≥ 500	+10.0/–23.4	+10.0/–23.4	+10.0/–19.7	+16.2/–18.0	+16.2/–18.0

NOTES:
1. WIDTH OF CORNER ZONES (EACH DIRECTION) AND EDGE ZONES ON MAIN ROOF IS 7'–6", SEE PLAN.
2. POSITIVE PRESSURES ACT TOWARDS THE SURFACE. NEGATIVE PRESSURES ACT AWAY FROM THE SURFACE.
3. LINEAR INTERPOLATE PRESSURES FOR EFFECTIVE WIND AREAS BETWEEN THOSE SCHEDULED OR USE PRESSURES FOR THE SMALLER EFFECTIVE WIND AREA.

FOUNDATIONS – GENERAL:

1. THE FOUNDATION HAS BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS MADE IN THE SOILS REPORT 49195082 DATED 07/22/2019 BY TERRACON. (1)
2. CONTINUOUS FOOTINGS & SPREAD FOOTINGS SHALL BEAR ON SOIL CAPABLE OF OF SUSTAINING A NET ALLOWABLE BEARING PRESSURE OF 2500 PSF UNDER SERVICE LIVE AND DEAD LOAD.
3. FOOTINGS MAY BE POURED INTO AN EARTH–FORMED TRENCH IF SOIL CONDITIONS PERMIT.
4. ALL BEARING MATERIAL SHALL BE INSPECTED BY THE INDEPENDENT TESTING AGENCY PRIOR TO CONCRETE PLACEMENT. THE INDEPENDENT TESTING AGENCY SHALL BE THE SOLE JUDGE AS TO THE SUITABILITY OF THE BEARING MATERIAL. FOOTING ELEVATIONS SHALL BE ADJUSTED AS REQUIRED.
5. BOTTOM OF EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 18 INCHES BELOW FINAL GRADE.
6. FOUNDATION WALLS THAT RETAIN EARTH SHALL BE BRACED AGAINST BACKFILLING PRESSURES UNTIL FLOOR SLABS AT TOP AND BOTTOM ARE IN PLACE.
7. WHERE FOUNDATION WALLS ARE TO HAVE EARTH PLACED ON EACH SIDE, PLACE FILL SIMULTANEOUSLY SO AS TO MAINTAIN A COMMON ELEVATION ON EACH SIDE OF THE WALL.

CONCRETE AND REINFORCING STEEL:

1. CONCRETE SHALL CONFORM TO ACI BUILDING CODE (318R–05) AND SHALL HAVE A 28–DAY COMPRESSIVE STRENGTH AND DENSITY, IN ACCORDANCE WITH THE FOLLOWING:

	STRENGTH PSI	DENSITY PCF	MAX W/C RATIO
INTERIOR SLABS, EXTERIOR SLABS	4000	145	0.45
CURBS, SIDEWALKS (AIR–ENTRAINED)	4000	145	0.50
ALL OTHER CONCRETE (U.N.O.)	3000	145	0.55

2. AIR ENTRAINMENT FOR EXTERIOR SLABS SHALL CONFORM TO TABLE 4.2.1. OF ACI 318–05, SEVERE EXPOSURE.
3. REINFORCING SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
4. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
5. MINIMUM CONCRETE COVER, UNLESS NOTED OTHERWISE:
 UNFORMED SURFACE IN CONTACT WITH THE GROUND. 3 IN.
 FORMED SURFACES EXPOSED TO EARTH OR WEATHER.
 #6 BARS AND LARGER 2 IN.
 #5 BARS AND SMALLER 1–1/2 IN.
 FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER:
 BEAMS, GIRDERS, AND COLUMNS 1–1/2 IN.
 SLABS, WALLS, AND JOISTS
 #11 BARS AND SMALLER 3/4 IN.
 #14 AND #18 BARS 1–1/2 IN.
6. LAP SPLICES SHALL BE IN ACCORDANCE WITH THE FOLLOWING TABLE, UNLESS NOTED OTHERWISE. WHERE CLASSES ARE NOT CALLED OUT ON DRAWINGS, USE CLASS "B" SPLICES. SPLICES SHALL BE STAGGERED AT LEAST 24 INCHES.

BAR SIZE	TENSION SPLICES (INCHES): TOP BARS A	TOP BARS B	OTHER BARS A	OTHER BARS B	COMPRESSION SPLICES (INCHES)
#3	16	21	12	16	12
#4	21	28	16	21	15
#5	27	35	21	27	19
#6	35	46	27	35	23
#7	48	62	37	48	26
#8	63	82	48	63	30
#9	80	104	61	80	34
#10	101	131	78	101	38
#11	125	162	96	125	42

 COMPRESSION DOWEL EMBEDMENT: 22 BAR DIAMETERS
 LAP WELDED WIRE FABRIC ONE SPACING OF CROSS WIRES PLUS 2".
7. BASE PLATES, ANCHOR BOLTS, SUPPORT ANGLES, ETC., BELOW GRADE SHALL BE COVERED WITH A MINIMUM OF 3" OF CONCRETE.

REINFORCED MASONRY:

1. REINFORCED MASONRY SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH, f'm, OF 1500 PSI. MASONRY UNITS SHALL BE NORMAL WEIGHT BLOCK CONFORMING TO ASTM C90, GRADE N, TYPE 1, AND SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI. MORTAR SHALL CONFORM TO ASTM C270, TYPE S, U.N. AND TYPE N FOR INTERIOR WALLS. GROUT SHALL CONFORM TO ASTM C476. GROUT SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2000 psi AND MAXIMUM DENSITY OF 115 pcf. SLUMP AT POINT OF PLACEMENT SHALL BE 9" +/– 1".
2. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
3. CONTINUOUS WIRE REINFORCING (JOINT REINFORCING) SHALL BE GALVANIZED TRUSS OR LADDER TYPE FORMED FROM 9 GAUGE COLD – DRAWN STEEL WIRE COMPLYING WITH ASTM A82. JOINT REINFORCING SHALL BE SPACED AT 16" O.C. VERTICALLY IN ALL MASONRY WALLS.
4. SEE DRAWINGS FOR LOCATIONS OF VERTICAL CONTROL JOINTS. HORIZONTAL BOND BEAM AND LINTEL REINFORCING SHALL BE CONTINUOUS ACROSS VERTICAL CONTROL JOINTS. JOINT REINFORCING SHALL BE STOPPED EITHER SIDE OF VERTICAL CONTROL JOINTS.
5. ALL REINFORCED CELLS, ALL CELLS BELOW GRADE AND ALL CELLS BELOW FINISH FLOOR SHALL BE GROUTED SOLID.
6. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL BLOCK CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICAL. DOWELS MAY BE GROUTED INTO A CELL IN VERTICAL ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING.
7. REINFORCING STEEL SHALL BE SECURED IN PLACE BEFORE GROUTING STARTS.
8. SPLICED REINFORCING SHALL BE LAPPED 50 BAR DIAMETERS OR 24 INCHES, WHICHEVER IS GREATER. SPLICED BARS SHALL BE WIRED TOGETHER.
9. VERTICAL BARS SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 200 DIAMETERS OF THE REINFORCING, NOR 10 FEET. BARS SHALL BE IN PLACE PRIOR TO GROUTING.
10. VERTICAL REINFORCING BARS SHALL HAVE A MINIMUM CLEARANCE OF 1/4 OF AN INCH FROM THE MASONRY FOR FINE GROUT AND 1/2 INCH FOR COARSE GROUT.
11. VERTICAL CELLS THAT WILL BE GROUTED SHALL HAVE A VERTICAL ALIGNMENT TO MAINTAIN A CONTINUOUS UNOBSTRUCTED CELL AREA NOT LESS THAN 3"x4".
12. GROUTING SHALL BE STOPPED 1–1/2" BELOW THE TOP OF A COURSE SO AS TO FORM A KEY AT THE POUR JOINT.
13. GROUTING OF MASONRY BEAMS OVER OPENINGS SHALL BE DONE IN ONE CONTINUOUS OPERATION.
14. ALL BOLTS, ANCHORS, ETC., INSERTED IN THE WALLS, SHALL BE GROUTED SOLID INTO POSITION.

STRUCTURAL STEEL:

1. STEEL SHALL CONFORM TO THE FOLLOWING GRADES:

ALL CHANNELS, ANGLES, PLATES, ETC. (UNO)	A36 (Fy=36)
WF BEAMS	A992 (Fy=50) U.N.
STRUCTURAL TUBE	A500 (Fy=46)
STEEL PIPE	A53 (Fy=35)
ANCHOR BOLTS	A307
BOLTS	A325
WELDING ELECTRODES	E70XX
THREADED ROD ANCHORS	A36
SHEAR STUDS	A108

2. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE (2010), EXCEPT AS MODIFIED IN THESE NOTES AND THE PROJECT SPECIFICATIONS.
3. THE STEEL STRUCTURE IS LATERAL UNSTABLE AND IS DEPENDENT UPON DIAPHRAGM ACTION OF THE METAL ROOF DECK AND ATTACHMENT TO THE WALL SYSTEM FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES. PROVIDE ALL TEMPORARY SUPPORTS REQUIRED FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES UNTIL THESE ELEMENTS ARE FULLY SECURED TO EACH OTHERB AND CAPABLE OF PROVIDING THIS SUPPORT.
4. THE FABRICATOR IS RESPONSIBLE FOR THE DESIGN OF ALL CONNECTIONS. CONNECTIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE SCHEMATIC AND ARE ONLY INTENDED TO SHOW THE RELATIONSHIP OF MEMBERS CONNECTED. CONNECTION DETAILS INDICATED ON THE DRAWINGS SHALL BE INCORPORATED INTO FABRICATOR'S CONNECTION DESIGN. SEE SPECIFICATIONS. ALL SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE FABRICATOR'S ENGINEER WITH THE ENGINEER'S SEAL FOR THE STATE WHERE THE STRUCTURE IS LOCATED. ENGINEER'S SEAL MAY BE QUALIFIED "FOR DESIGN OF CONNECTIONS ONLY."

WELDING:

1. ALL STRUCTURAL STEEL WELDING SHALL CONFORM TO AMERICAN WELDING SOCIETY STANDARD D1.1, LATEST EDITION.
2. REINFORCING STEEL WELDING SHALL CONFORM TO AWS D1.4. REINFORCING SHALL CONFORM TO ASTM A–706.
3. ALL STRUCTURAL STEEL AND REINFORCING STEEL WELDING ELECTRODES SHALL CONFORM TO AWS A5.1 OR A5.5 E–70XX.
4. FIELD WELDING SHALL BE SHOWN ON ERECTION DRAWINGS.
5. ALL FIELD FULL PENETRATION WELDS SHALL BE INSPECTED AND TESTED BY A TESTING AGENCY TO BE PAID BY OWNER.
6. ALL EXPOSED WELDED CONNECTIONS SHALL BE GROUND SMOOTH AND SUBJECT TO ARCHITECT APPROVAL. THE CONTRACTOR SHALL SUBMIT WRITTEN PROCEDURE AND DETAILS TO THE ARCHITECT FOR REVIEW PRIOR TO COMMENCING THE WORK.
7. PAINT ALL WELDS WITH RUST INHIBITIVE PAINT.

STEEL JOISTS:

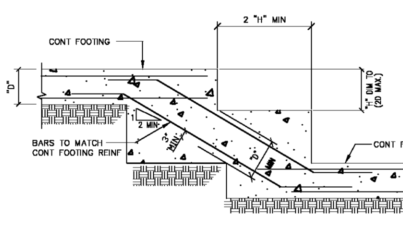
1. STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE "STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS, 'K–SERIES' (2010)" OR THE "STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS, 'LH–SERIES'(2010)" OF THE STEEL JOIST INSTITUTE (SJI).
2. STEEL JOISTS SHALL BE DESIGNED BY THE MANUFACTURER. THE MAUFACTURER'S ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN, ADEQUACY AND SAFETY OF ALL STEEL JOISTS. ALL SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE MANUFACTURER'S ENGINEER WITH THE ENGINEER'S SEAL FOR THE STATE WHERE THE STRUCTURE IS LOCATED.
3. UNLESS OTHERWISE NOTED, STEEL JOISTS SHALL BE DESIGNED AS SIMPLY SUPPORTED, UNIFORMLY LOADED TRUSSES WITH THE TOP CHORD BRACED AGAINST LATERAL BUCKLING. THE UNIFORM DESIGN LOAD SHALL BE THE TOTAL SAFE UNIFORMLY DISTRIBUTED LOAD AS SHOWN IN THE SJI STANDARD LOAD TABLE.
4. KCS (CONSTANT SHEAR) SERIES STEEL JOIST SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH SJI "K" SERIES JOIST WITH CAPACITIES LISTED IN VULCRAFT 2013 STEEL JOIST AND JOIST GIRDERS PUBLICATIONS.
5. WHEN NET UPLIFT FORCES DUE TO WIND ARE SHOWN ON THE DRAWINGS, THE MANUFACTURER SHALL DESIGN THE JOISTS, BRIDGING AND CONNECTIONS OF THE JOISTS TO THE SUPPORTING STRUCTURE FOR THE NET UPLIFT. A SINGLE LINE OF BOTTOM CHORD BRIDGING MUST BE PROVIDED NEAR THE FIRST BOTTOM CHORD PANEL POINTS WHENEVER UPLIFT DUE TO WIND FORCES IS SHOWN ON THE DESIGN DRAWINGS.
6. WHEN NONUNIFORM OR CONCENTRATED LOADS ARE SHOWN ON THE DRAWINGS, THE MANUFACTURER SHALL DESIGN THE JOISTS IN ACCORDANCE WITH PARAGRAPH 4.1 OF THE "STANDARD SPECIFICATION FOR OPEN WEB STEEL JOISTS, 'K–SERIES'" OR PARAGRAPH 103.1 OF THE "STANDARD SPECIFICATION FOR OPEN WEB STEEL JOIST, 'LH–SERIES'"
7. STEEL JOIST BEARING AT EXTERIOR WALLS SHALL BE DESIGNED TO RESIST 900 POUNDS LATERAL FORCE PERPENDICULAR TO THE JOIST AND APPLIED AT THE TOP OF THE JOIST BEARING. STIFFENERS AT THE JOIST BEARING SHALL BE ADDED IF REQUIRED.
8. STEEL JOIST BRIDGING SHALL BE PROVIDED IN ACCORDANCE WITH THE SJI SPECIFICATION. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE PLACED AND STEEL JOIST ENDS FIXED PRIOR TO THE APPLICATION OF ANY LOADS. BRIDGING THAT TERMINATES AT, OR IS INTERRUPTED BY, STRUCTURAL STEEL BEAMS, MASONRY WALLS OR CONCRETE WALLS SHALL BE ATTACHED THERETO. COORDINATE BRIDGING LOCATIONS TO AVOID INTERFERENCE WITH ALL MECHANICAL, ELECTRICAL AND FIRE PROTECTION EQUIPMENT.
9. MINIMUM BEARING REQUIREMENTS FOR K–SERIES JOISTS, UNLESS NOTED OTHERWISE.
 2–1/2" ON STRUCTURAL STEEL
 4" ON STEEL BEARING PLATES OVER MASONRY OR CONCRETE
10. UNLESS NOTED OTHERWISE, K–SERIES JOISTS SHALL BE ATTACHED TO SUPPORTING STEEL WORK OR STEEL BEARING PLATES WITH (2) 1/8" FILLET WELD (ONE EACH SIDE) 1" LONG, WITH (2) 1/2" DIAMETER BOLT, OR WITH A COMBINATION OF (1) 1/8" FILLET WELD AND (1) 1/2" DIAMETER BOLT.
11. UNLESS NOTED K–SERIES JOISTS SHALL HAVE 2 1/2" DEEP SEAT, LH–SERIES 5" DEEP SEAT. WHERE STEEL JOIST OR GIRDER SLOPE EXCEEDS 1/4" PER FT., PROVIDE SLOPED BEARING SEAT.
12. MINIMUM BEARING REQUIREMENTS FOR LH–SERIES JOISTS, UNLESS NOTED OTHERWISE.
 4" ON STRUCTURAL STEEL
 6" ON STEEL BEARING PLATES OVER MASONRY OR CONCRETE
13. UNLESS NOTED OTHERWISE, LH–SERIES JOISTS SHALL BE ATTACHED TO SUPPORTING STEEL WORK OR STEEL BEARING PLATES WITH (2) 1/4" FILLET WELD (ONE EACH SIDE) 2" LONG, WITH (2) 3/4" DIAMETER BOLTS (ONE EACH SIDE).
14. STEEL JOISTS AT COLUMN CENTER LINES SHALL BE BOLTED TO STRUCTURAL STEEL WITH (2) 1/2" DIAMETER BOLTS OR (2) 3/4" DIAMETER BOLTS FOR LH–SERIES. WHERE STEEL JOISTS DO NOT OCCUR AT THE COLUMN, USE BOLTED CONNECTIONS FOR THE STEEL JOIST CLOSEST TO THE CENTER LINE.
15. HOLES IN STEEL JOIST CHORDS WILL NOT BE PERMITTED, EXCEPT FOR BOLTED CONNECTIONS AT THE BEARING END OF THE STEEL JOIST.
16. ALL ITEMS SUCH AS MECHANICAL EQUIPMENT, DUCT WORK, PIPES, CEILING, FIXTURES, ETC. THAT ARE TO BE SUPPORTED OR HUNG FROM THE STEEL JOIST SHALL BE FRAMED WITH AUXILIARY FRAMING TO THE PANEL POINTS OF THE STEEL JOIST. METHODS OF FRAMING THAT INDUCE BENDING TO THE STEEL JOIST CHORDS OR WEB MEMBERS WILL NOT BE PERMITTED.
17. THE CONTRACTOR SHALL VERIFY BY A CERTIFIED LETTER TO THE ARCHITECT, THAT THE FINAL LOCATION OF THE SPRINKLER SYSTEM MAIN BRANCH LINES MATCHES THE LOCATION SHOWN ON THE FIRE PROTECTION DRAWINGS. ANY DEVIATIONS FROM THIS SPECIFIED DRAWING MUST BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER IN WRITING. THE COST OF ADDITIONAL SUPPORTS DUE TO DEVIATIONS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
18. PLACEMENT OF SPRINKLER CONTRACTORS PIPING AND STEEL CONTRACTORS JOIST BRIDGING SHALL BE COORDINATED BY THE GENERAL CONTRACTOR. THE GENERAL CONTRACTOR SHALL MAKE PROVISIONS FOR THE STEEL ERECTOR TO ADJUST/ RELOCATE BRIDGING OR SPRINKLER CONTRACTOR TO ADJUST OR RELOCATE PIPING WHERE CONFLICTS OCCUR ALL CHANGES REQUIRED OF THE JOIST BRIDGING SHALL BE DESIGNED BY JOIST MANUFACTURER.
19. SPRINKLER PIPE SUPPORT CRITERIA:
 A. ALL SUPPORT CRITERIA LISTED BELOW FOR 6 INCH AND 8 INCH DIAMETER PIPING WAS BASED ON SCHEDULE 10 PIPING BEING USED. IF HEAVIER PIPING IS USED, THE STRUCTURAL ENGINEER OF RECORD MUST BE NOTIFIED.
 B. MAXIMUM SPACING OF SUPPORTS FOR 6 INCH AND 8 INCH DIAMETER PIPES SHALL BE 12'–6", BUT NO MORE THAN TWO JOIST SPACES FOR PIPES PERPENDICULAR TO JOISTS.
 C. PIPES SPANNING PERPENDICULAR TO STEEL JOIST:
 – ALL 6 INCH AND 8 INCH DIAMETER PIPES ADJACENT TO EACH OTHER SHALL BE SUPPORTED FROM ALTERNATE STEEL JOIST.
 D. PIPES SPANNING PARALLEL TO THE STEEL JOIST:
 1. 6 INCH DIAMETER PIPE HANGERS SHALL NOT BE PLACED CLOSER THAN 12 INCHES TO ANY STEEL JOIST.
 2. 8 INCH DIAMETER PIPE SHALL BE PLACED HALF–WAY (± 3 INCHES) BETWEEN THE SUPPORT JOISTS.

METAL ROOF DECK:

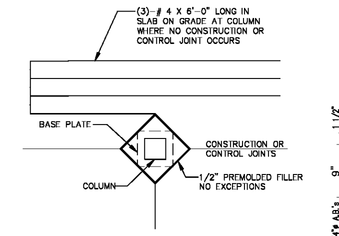
1. METAL ROOF DECK SHALL COMPLY WITH THE REQUIREMENTS OF THE STEEL DECK INSTITUTE "SPECIFICATIONS AND COMMENTARY FOR STEEL ROOF DECK" [illegible]
2. METAL ROOF DECK SHALL BE CONFIGURATION, DEPTH AND MINIMUM GAGE AS SHOWN ON THE DRAWINGS. MINIMUM YIELD STRENGTH SHALL BE 33,000 [illegible]. ATTACHMENT TO THE SUPPORTING STRUCTURE SHALL BE AS SHOWN ON THE DRAWINGS. SEE ROOF PLAN NOTES.
3. STEEL ROOF DECK AND CONNECTORS SHALL CONFORM TO FACTORY MUTUAL PUBLICATION I–28 AND I–28S, CLASS I–90 WINDSTORM RESISTANCE. INSTALLATION SHALL BE BY AN OSHA CERTIFIED OPERATOR IN ACCORDANCE WITH THE FASTENER MANUFACTURER'S INSTRUCTIONS AND INITIAL FASTENING UNDER THE SUPERVISION OF THE MANUFACTURER'S REPRESENTATIVE. STEEL ROOF DECK SHALL BE ATTACHED AS FOLLOWS:
 AT SUPPORTS: [illegible]5/8" PUDDLE WELDS.
 AT SIDELAPS: [illegible] TEK SCREWS.
4. DO NOT HANG OR SUPPORT ANY LOADS FROM METAL ROOF DECK.
5. METAL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS.

MISCELLANEOUS:

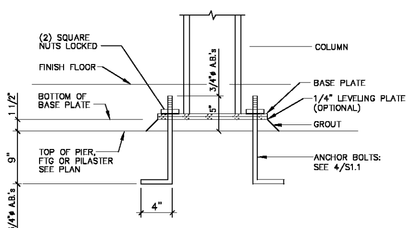
1. THESE GENERAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
2. STRUCTURAL DRAWINGS ARE INTENDED TO BE USED WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL AND FIRE SPRINKLER DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING SUCH REQUIREMENTS INTO THEIR SHOP DRAWINGS AND CONSTRUCTION.
3. NO OPENINGS SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE PROFESSIONAL OF RECORD.
4. NO CHANGE IN SIZE, MATERIAL OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE PROFESSIONAL OF RECORD.
5. OPENINGS 1'–4" AND LESS ON A SIDE ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SUCH OPENINGS.
6. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
7. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/ OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.
8. DO NOT SCALE THESE DRAWINGS, USE DIMENSIONS.
9. CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
10. THE CONTRACTOR SHALL INFORM THE PROFESSIONAL OF RECORD IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY OF SUCH DEVIATION BY THE PROFESSIONAL OF RECORD REVIEW OF SHOP DRAWINGS, PRODUCT DATA, ETC., UNLESS THE CONTRACTOR HAS SPECIFICALLY INFORMED THE PROFESSIONAL OF RECORD OF SUCH DEVIATION AT THE TIME OF SUBMISSION, AND THE PROFESSIONAL OF RECORD HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.
11. CONNECTIONS OF ALL ITEMS SUPPORTED BY THE STRUCTURE ARE THE RESPONSIBILITY OF THE DISCIPLINES WHO ARE MAKING THESE ATTACHMENTS. THESE ATTACHMENTS SHALL BE DESIGNED TO RESIST ALL GRAVITY, WIND, WIND UPLIFT, THERMAL LOADS, ETC.
12. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AND EXISTING DIMENSIONS AND CONDITIONS.
13. UNLESS NOTED, SUBMIT SHOP DRAWINGS OF ALL FABRICATED MATERIALS FOR REVIEW. DESIGN DRAWINGS SHALL NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS. SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS THEY WERE CHECKED, BEAR THE INITIAL OF THE CHECKER AND ARE STAMPED "APPROVED" BY THE GENERAL CONTRACTOR.



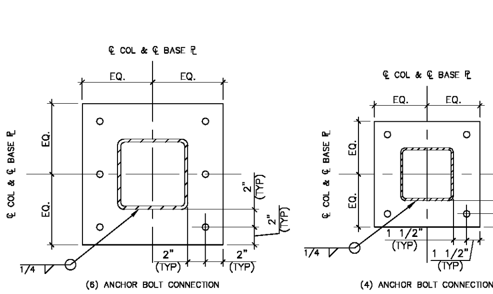
1 S1.1 TYPICAL STEPPED FOOTING DETAIL
SCALE: 1" = 1'–0"



2 S1.1 TYPICAL COLUMN ISOLATION DETAIL
SCALE: 1" = 1'–0"

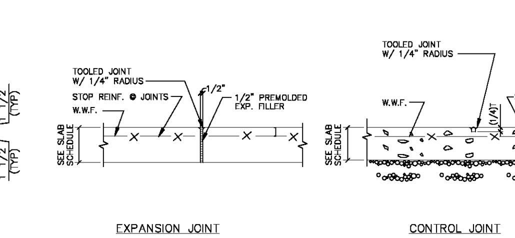


3 S1.1 TYPICAL COLUMN BASE
SCALE: 1" = 1'–0"

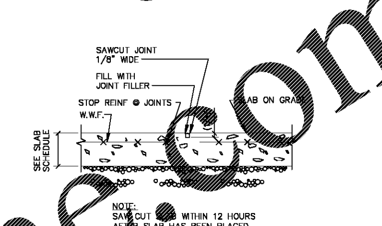


NOTE: SEE COLUMN SCHEDULE FOR COLUMN BASE PLATE AND ANCHOR BOLT SIZES. SEE DETAIL 3/S1.1 FOR ANCHOR BOLT LENGTHS.

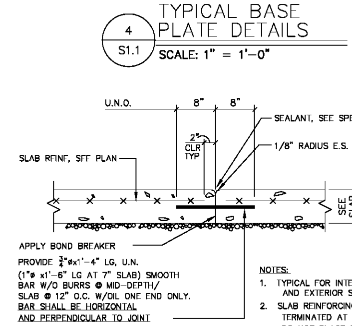
4 S1.1 TYPICAL BASE PLATE DETAILS
SCALE: 1" = 1'–0"



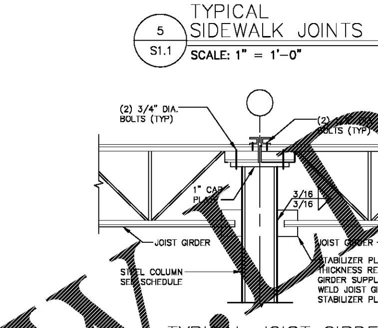
5 S1.1 TYPICAL SIDEWALK JOINTS
SCALE: 1" = 1'–0"



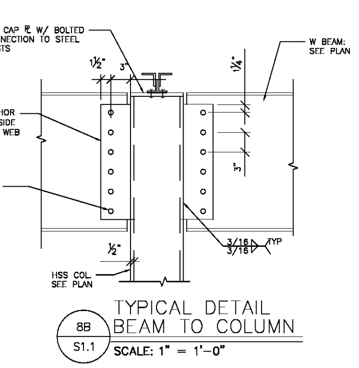
6 S1.1 TYPICAL SLAB CONTROL JOINTS
SCALE: 1" = 1'–0"



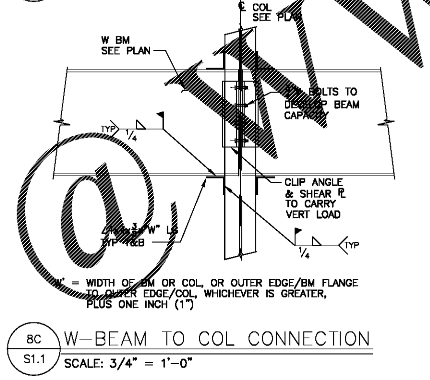
7 S1.1 TYPICAL SLAB CONSTRUCTION JOINTS
SCALE: 1" = 1'–0"



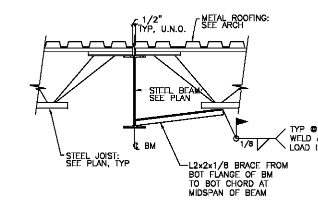
8A S1.1 TYPICAL JOIST GIRDER TO COLUMN CONNECTION
SCALE: 3/4" = 1'–0"



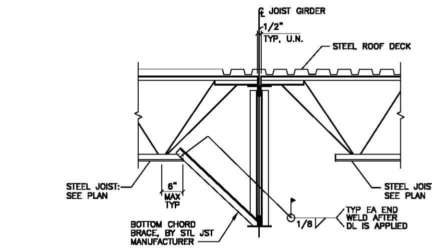
8B S1.1 TYPICAL DETAIL BEAM TO COLUMN
SCALE: 1" = 1'–0"



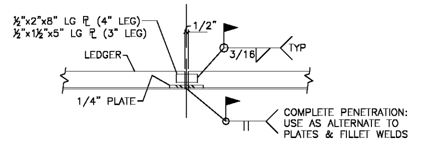
8C S1.1 W–BEAM TO COL CONNECTION
SCALE: 3/4" = 1'–0"



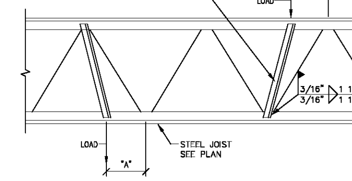
9 S1.1 TYP JOIST ON W BEAM
SCALE: 1" = 1'–0"



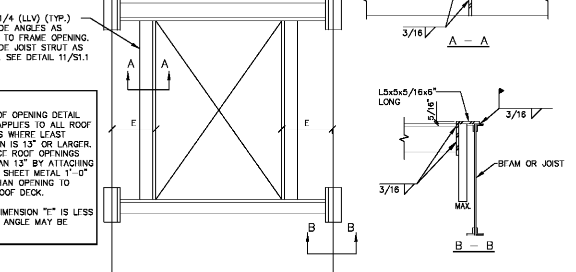
9A S1.1 SECTION
SCALE: 3/4" = 1'–0"



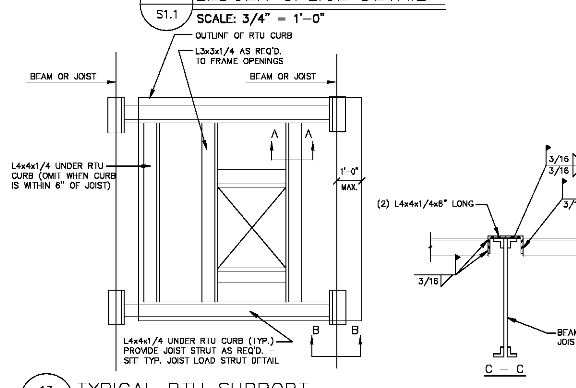
10 S1.1 LEDGER SPLICE DETAIL
SCALE: 3/4" = 1'–0"



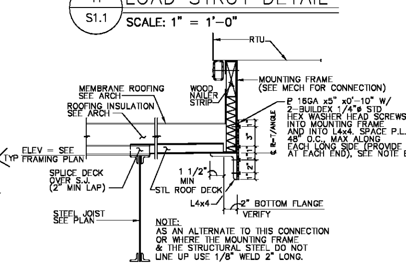
11 S1.1 TYPICAL JOIST LOAD STRUT DETAIL
SCALE: 1" = 1'–0"



12 S1.1 TYPICAL ROOF OPENING
SCALE: 1" = 1'–0"



13 S1.1 TYPICAL RTU SUPPORT
SCALE: 1" = 1'–0"



DETAILS SHOWN ON THIS SHEET ARE APPLICABLE THROUGHOUT THE DESIGN DRAWINGS. THESE DETAILS ARE DEFINED AS GENERAL STANDARDS THAT ARE USUALLY NOT IDENTIFIED BY SPECIFIC REFERENCE WITHIN THE DRAWINGS. THESE DETAILS MAY BE MODIFIED OR SUPERCEDED BY SPECIFIC DETAILS THAT ARE REFERENCED WITHIN THE DRAWINGS.

14 S1.1 TYPICAL HVAC CURB SUPPORT DETAIL
SCALE: 1" = 1'–0"

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1454 AND 1460 OLD SALEM ROAD CONYERS, GEORGIA

GENERAL NOTES AND TYPICAL DETAILS

Date:	Drawn By:	Sheet No.:
03/29/19	C.B.	S1.1
Project No.: 20182028.0	Rev'd. By: M.H.	(2)