

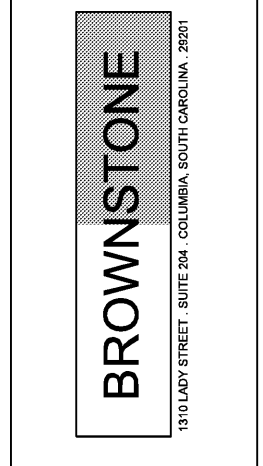
GENERAL NOTES

- 1.0 THIS PROJECT HAS BEEN DESIGNED FOR THE WEIGHTS AND MATERIALS INDICATED ON THE DRAWINGS AND FOR THE LIVE LOADS INDICATED IN THE DESIGN DATA. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE THE ALLOWABLE CONSTRUCTION LOADS AND TO PROVIDE PROPER DESIGN AND CONSTRUCTION OF FALSEWORK, FORMWORK, STAGING, BRACING, SHEETING AND SHORING, ETC.
1.1 COORDINATE THESE DRAWINGS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL DRAWINGS.
1.2 THE CONTRACTOR SHALL VERIFY AND COORDINATE ALL FLOOR, WALL AND ROOF OPENING SIZES AND LOCATIONS, EQUIPMENT PAD SIZES AND LOCATIONS, ANCHOR BOLT LAYOUTS, ETC WITH EQUIPMENT SELECTED. THE CONTRACTOR SHALL MAKE ANY REQUIRED MODIFICATIONS AT NO ADDITIONAL COST.
1.3 THE CONTRACTOR SHALL REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR SLEEVES, CURBS, INSERTS OR OPENINGS, ETC. NOT HEREIN INDICATED.
1.4 NOT USED
1.5 NOT USED
1.6 WORK NOT INCLUDED ON THE DRAWINGS BUT IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING PLACES ELSEWHERE ON THE DRAWINGS SHALL BE REPEATED.
1.7 IN CASE OF CONFLICT BETWEEN THE NOTES, DETAILS AND SPECIFICATIONS THE MOST RIGID REQUIREMENTS SHALL GOVERN.
1.8 NOT USED
1.9 SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF MASONRY AND DRYWALL NON-LOAD BEARING PARTITIONS. PROVIDE COMPRESSIBLE FIRESTOPPING AT TOP OF WALL AS REQUIRED BY ARCHITECTURAL DRAWINGS.
2.0 FOUNDATION NOTES
2.1 GEOTECHNICAL INFORMATION FOR THIS PROJECT WAS TAKEN FROM THE INVESTIGATION PERFORMED BY ECS CAROLINAS, LLP DATED NOVEMBER 5, 2014.
2.2 SEE THE SPECIFICATION REQUIREMENTS FOR EXCAVATION AND PREPARATION OF THE FOUNDATION AND THE SLAB ON GRADE SUBGRADE INCLUDING COMPACTION PROCEDURES.
2.3 FOOTING AND RETAINING WALLS ARE DESIGNED FOR A SOIL BEARING CAPACITY OF 3000 PSF.
2.4 FOUNDATION CONDITIONS NOTED DURING CONSTRUCTION WHICH DIFFER FROM THOSE DESCRIBED IN THE GEOTECHNICAL REPORT SHALL BE REPORTED TO THE GENERAL CONTRACTOR BEFORE FURTHER CONSTRUCTION IS ATTEMPTED. SEE PROJECT SPECIFICATIONS.
2.5 NO FOOTINGS OR SLABS SHALL BE POURED INTO OR AGAINST SUBGRADE CONTAINING FREE WATER, FROST, ICE OR LOOSE MATERIAL.
2.6 ALL SLAB ON-GRADE, TRENCH BOTTOMS AND OTHER ON-GRADE INTERIOR HORIZONTAL SURFACES SHALL BE PLACED OVER A REINFORCED 15 MIL VAPOR BARRIER OVER A 4" #57 STONE WATER BARRIER PLACED ON SUBGRADE PROPERLY PREPARED IN ACCORDANCE WITH THE CONTRACT SPECIFICATIONS. (UNO)
2.7 SEE PLUMBING, ELECTRICAL & CIVIL DRAWINGS FOR REQUIRED UNDERSLAB UTILITIES.
2.8 SEE SPECIFICATIONS FOR ALL WATERPROOFING DETAILS AND MATERIALS AS REQUIRED.
2.9 IF UNDERMINING OF FOOTINGS OCCURS, FILL VOIDS WITH 2500 PSI CONCRETE. DO NOT ATTEMPT TO REPLACE AND RECOMPACT SOIL.
3.0 CONCRETE
3.1 CONCRETE SHALL HAVE THE UNIT WEIGHT AND THE MINIMUM COMPRESSIVE STRENGTHS (f'c) AT 28 DAYS AS SHOWN ON THE CONCRETE MATERIALS SCHEDULE. (DWG 5002) SEE SPECIFICATIONS FOR FURTHER INFORMATION.
ENTRAN AIR TO PRODUCE TOTAL AIR CONTENT ACCORDING TO THE SPECIFICATIONS. FOR CONCRETE EXPOSED TO FREEZING TEMPERATURES (EXTERIOR FOOTINGS, SLAB TURNDOWNS, EXTERIOR SLABS AND SLABS-ON-GRADE, EXTERIOR RETAINING WALLS, AND EXTERIOR GRADE BEAMS.)
3.2 GROUT FOR BASE PLATES SHALL BE NON-SHRINKABLE GROUT AND SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS OF 5000 PSI, UNLESS NOTED OTHERWISE.
3.3 NO CALCIUM CHLORIDE SHALL BE USED IN ANY CONCRETE.
3.4 MIXING, TRANSPORTING AND PLACING OF CONCRETE SHALL CONFORM TO ACI-301-89.
3.5 ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI-318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", AND CONTRACT SPECIFICATIONS. WHEN THERE IS A CONFLICT BETWEEN ACI AND SPECIFICATIONS, THE MORE STRINGENT SHALL GOVERN.
3.6 CHAMFER ALL EXPOSED EXTERNAL CORNERS OF CONCRETE WITH 3/4" x 45 DEGREE CHAMFER, UNO.
3.7 CONCRETE REINFORCEMENT BARS SHALL CONFORM TO ASTM A615, GRADE 60, REINFORCING BARS SHALL NOT BE TACK WELDED, WELDED OR CUT, UNLESS INDICATED ON THE CONTRACT DOCUMENTS. ALL LAP SPICES SHALL BE CLASS "B" UNO.
3.8 HORIZONTAL FOOTING AND HORIZONTAL WALL REINFORCEMENT SHALL BE CONTINUOUS AND SHALL HAVE 90 DEGREE BENDS AND EXTENSIONS, OR CORNER BARS OF EQUIVALENT SIZE LAPPED WITH A CLASS B TENSION SPLICE AT CORNERS AND INTERSECTIONS. TOP BAR CRITERIA SHALL APPLY IF 12" OR MORE OF FRESH CONCRETE IS PLACED BELOW BAR.
3.9 SLABS-ON-GRADE SHALL HAVE CONSTRUCTION JOINTS OR CRACK CONTROL JOINTS AS SHOWN ON THE DRAWINGS. CONSTRUCTION JOINTS CAN BE USED AT CONTROL JOINT LOCATIONS AT CONTRACTORS OPTION. SEE SLAB PLANS & JOINT DETAILS FOR ADDITIONAL INFORMATION
3.10 SEE SPECIFICATIONS FOR ALL WATERPROOFING/DAMP-PROOFING DETAILS.
3.11 ALL WELDED WIRE FABRIC SHALL CONFORM TO THE STANDARDS OF ASTM A-185. SUPPLY IN FLAT SHEETS.
3.12 ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED, AND SPACED IN FORMS AND STOURED IN PLACE IN ACCORDANCE WITH THE PROCEDURES AND REQUIREMENTS OUTLINED IN THE LATEST EDITION OF THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", ACI 318, AND THE "MANUAL OF STANDARD PRACTICE FOR DETAILED REINFORCED CONCRETE STRUCTURES", ACI 315, LATEST EDITION.
3.13 SHOP DRAWINGS SHOWING REINFORCING DETAILS, INCLUDING STEEL SIZES, SPACING AND PLACEMENT, SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION.
3.14 ALL WELDED WIRE FABRIC SHALL BE LAPPED TWO (2) TIMES WITH MESH PANELS AND TIED SECURELY.
3.15 ALL DOWELS SHALL MATCH SIZE AND NUMBER OF MAIN REINFORCING UNLESS NOTED OTHERWISE ON DRAWINGS.
3.16 ADDITIONAL BARS SHALL BE PROVIDED AROUND ALL FLOOR AND WALL OPENINGS AS SHOWN ON THE DWGS.
3.17 SEE ARCHITECTURAL DRAWINGS FOR TYPE AND LOCATION OF ALL FLOOR FINISHES
3.18 THE CONTRACTOR SHALL COORDINATE ALL WALL/SLAB OPENINGS NOT SHOWN ON STRUCTURAL DRAWINGS. SEE ARCHITECTURAL, ELECTRICAL, PLUMBING AND CIVIL DRAWINGS.
3.19 UNLESS NOTED OTHERWISE ALL CURBS SHALL BE REINFORCED WITH AT LEAST 1 #4 CONTINUOUS AND #4 AT 18" OC DOWELS TO STRUCTURE BELOW.
3.20 THE SUB-CONTRACTOR SHALL VERIFY ALL OPENINGS, PAD SIZES, AND ANCHOR BOLTS WITH EQUIPMENT SELECTED.
3.21 FOR ALL WALLS & PIERS, PROVIDE DOWELS INTO FOOTING AT EACH VERT REINF BAR, U.N.O. DOWEL SIZE SHALL BE SAME AS VERT REINF.
3.22 ALL DEFORMED BAR ANCHORS SHALL BE TRS NELSON DIVISION OR EQUAL 3/4" DIA. U.N.O. CONFORMING TO ASTM A-496 WITH A MINIMUM TENSILE STRENGTH OF 80,000 PSI. ANCHOR DIMENSIONS SHALL BE IN ACCORDANCE WITH ASTM D-19. INSTALL ANCHORS IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS BY AUTOMATIC END WELDING AS INDICATED ON THE DRAWINGS. NO UNAUTHORIZED OR FIELD WELDING SHALL BE MADE WITHOUT AUTHORIZATION FROM THE MANUFACTURER.
3.23 ALL REINFORCING INDICATED TO BE WELDED SHALL BE IN ACCORDANCE WITH ASTM A706M. "LOW ALLOY STEEL DEFORMED BARS FOR CONCRETE REINFORCEMENT", ANY INSTALLATIONS USING MANUFACTURER'S EQUIPMENT SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
3.24 NOT USED
3.25 PROVIDE ADDITIONAL 2- #4 x 3'-0" REINFORCING BARS IN SLAB-ON-GRADE AT ALL RE-ENTRANT CORNERS. PLACE BARS AT MID-DEPTH OF SLAB WITH A CLEARANCE OF 2" FROM CORNER UNO.

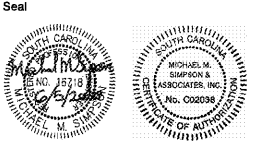
- 4.0 CONCRETE MASONRY
4.1 MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF THESE CONTRACT DOCUMENTS AND THE PROJECT SPECIFICATIONS.
4.2 THE SPECIFIED ULTIMATE COMPRESSIVE STRENGTH OF CONCRETE MASONRY (f'm) ON THE NET AREA IS A MINIMUM OF 1500 PSI.
4.3 PROVIDE TWO #5 BARS CONTINUOUS IN ALL BOND BEAMS, UNLESS OTHERWISE INDICATED IN THE DRAWINGS. REINFORCEMENT PLACED IN BOND BEAMS SHALL CONTINUOUS WITH STANDARD ACI HOOKS AT EACH END. PROVIDE STANDARD BAR SPLICES AS SPECIFIED. MAXIMUM BOND BEAM SPACING SHALL BE 8'-0" OC UNO.
4.4 PROVIDE VERTICAL REINFORCEMENT IN ALL WALLS AS SHOWN. PROVIDE VERTICAL BARS AT EACH END OF ALL WALLS AS SHOWN. PROVIDE VERTICAL BARS ON EACH SIDE OF OPENINGS IN ALL WALLS AS SHOWN. PROVIDE STANDARD BAR SPLICES AS SPECIFIED. ALL VERTICAL REINFORCEMENT EXTENDS FULL HEIGHT OF WALL. SEE TYPICAL MASONRY DETAILS DWG 55.01 AND OTHER SECTIONS AND DETAILS AS INDICATED ON DRAWINGS.
4.5 BLOCK CELLS THAT REQUIRE VERTICAL REINFORCING BARS AS INDICATED ON THE CONTRACT DRAWINGS AND/OR SPICES SHALL BE PLACED IN CENTERS OF BLOCK CELLS UNO. PROVIDE #5 BAR AT INDICATED SPACING MINIMUM VERTICAL REINFORCING BARS @ ALL INTERIOR NON-STRUCTURAL CMU WALLS UNO.
4.6 PROVIDE JOINT REINFORCEMENT AS INDICATED IN THE SPECIFICATIONS AND ON THE ARCHITECTURAL DRAWINGS.
4.7 PROVIDE CONTROL JOINTS AT MAJOR CHANGES IN WALL HEIGHT, CHANGES IN WALL THICKNESS, AT FLOOR CONTROL JOINTS, AT WALL OPENINGS, AND NEAR RETURN ANGLES OF L, T, AND U SHAPED STRUCTURES. CONTROL JOINT SPACING SHALL NOT EXCEED THE DISTANCES INDICATED ON THE ARCHITECTURAL DRAWINGS, OR A MAX. OF 20'-0"
4.8 GROUT FOR MASONRY SHALL BE NORMAL WEIGHT AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. GROUT SHALL CONFORM TO ASTM C476. GROUT LIFTS SHALL NOT EXCEED 4'-0".
4.9 FILL ALL BOND AND LINTEL BEAMS AND CELLS AT VERTICAL REINFORCEMENT WITH GROUT.
4.10 USE MORTAR TYPE AS INDICATED IN THE SPECIFICATIONS.
4.11 CONCRETE MASONRY UNITS SHALL BE AS INDICATED IN SPECIFICATIONS.
4.12 ALL CELLS, OPEN CAVITIES, AND AIR SPACES BELOW GRADE SHALL BE GROUTED.
4.13 BOND BEAMS AND REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS (UNO). MAXIMUM CONTROL JOINT SPACING SHALL BE AS INDICATED ON THE ARCHITECTURAL DRAWINGS, OR A MAX. OF 20'-0"
4.14 CONTRACTOR SHALL COORDINATE LOCATION OF ALL OPENINGS SEE ARCH. MECH., ELEC., AND PLUMBING DWGS. FOR SIZE AND LOCATION OF OPENINGS.
4.15 SEE DRAWING 5501 FOR TYPICAL MASONRY SECTIONS AND DETAILS.
4.16 MASONRY WALLS SHALL NOT BE BACK FILLED PRIOR TO THE MORTAR AND GROUT ATTAINING THEIR RESPECTIVE MAXIMUM DESIGN STRENGTHS PER SPECIFICATIONS.
4.17 ALL EXTERIOR STEEL INCLUDING LINTELS SHALL BE GALVANIZED
5.0 STEEL DECK
5.1 STEEL DECK SHALL BE ASTM A446 HAVING A MINIMUM YIELD STRENGTH OF 33,000 PSI AS PER THE STEEL DECK INSTITUTE DESIGN MANUAL.
5.2 STEEL DECK SHALL BE ERECTED IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS AND ERECTION LAYOUTS AND CONNECTED TO SUPPORTING MEMBERS AS INDICATED BELOW.
5.3 ROOF DECK
5.3.1 STEEL ROOF DECK SHALL BE 1 1/2", 20 GAUGE GALVANIZED WIDE RIB SEE ROOF FRAMING PLAN
5.3.2 ROOF DECK SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:
20 GAUGE 1 1/2" DECK
MOMENT OF INERTIA, I 0.222 IN/FT^4 WIDTH
SECTION MODULUS (TOP OF DECK), Sx 0.247 IN/FT^3 WIDTH
SECTION MODULUS (BOT. OF DECK) Sp 0.234 IN/FT^3 WIDTH
IN ADDITION TO MEETING THE MINIMUM REQUIREMENTS ABOVE, THE DECK MANUFACTURER SHALL DESIGN THE ROOF DECK AND ATTACHMENTS TO STEEL FOR THE ROOF LOADS, INCLUDING DECK UPLIFT. ALL ROOF DECK SHALL HAVE A MINIMUM 2-SPAN CONDITION.
5.3.3 1 1/2" ROOF DECK SHALL BE CONNECTED TO SUPPORTS WITH #10 SCREWS IN THE BOTTOM OF THE FLUTES USING A S.D.I. 36/77 PATTERN (UNO). DECK SIDELAPS SHALL BE FASTENED USING #10 SCREWS WITH MINIMUM 6-SIDE LAPS CONNECTIONS PER SPAN. ALL ENDLAPS SHALL BE A MINIMUM OF 2" AND SHALL OCCUR OVER SUPPORTS. MINIMUM DIAPHRAGM SHEAR STRENGTH Q = 400 LF (UNO)
5.3.4 DO NOT SUSPEND PIPES, DUCTS, OR CEILING FROM ROOF DECK.
6.0 STRUCTURAL STEEL
6.1 STRUCTURAL STEEL WELDED SHOPS AND PLATES SHALL CONFORM TO THE MATERIAL INFORMATION AND DIMENSION SHEET 02. DIMENSIONS AND PROPERTIES SHALL BE IN ACCORDANCE WITH ASTM
6.2 ANCHOR BOLTS SHALL CONFORM TO ASTM A36, OR A307, UNLESS NOTED OTHERWISE.
6.3 CONNECTIONS FOR STRUCTURAL STEEL MEMBERS SHALL BE 3/4" DIA A325-N, UNO AND SHALL CONFORM TO ASTM A325; NUTS SHALL CONFORM TO ASTM A307. WASHERS SHALL CONFORM TO ASTM F436. CONNECTION BOLTS SHALL HAVE A HARDENED WASHER PLACED UNDER THE ELEMENT TO BE TIGHTENED.
6.4 DETAILING OF STRUCTURAL STEEL CONNECTIONS MUST BE CONSISTENT WITH RECOGNIZED, PUBLISHED METHODS SUCH AS IN THE ASC "MANUAL OF STEEL CONSTRUCTION", 14TH EDITION; "ENGINEERING FOR STEEL CONSTRUCTION", OR "VOLUME II CONNECTIONS MANUAL OF STEEL CONSTRUCTION".
6.4.1 SECTION A7 OF ASC 14TH EDITION IS AMENDED SUCH THAT THE FABRICATION IS RESPONSIBLE FOR THE DESIGN AND DETAILING OF ALL CONNECTIONS.
6.5 CONNECTIONS SHALL BE DESIGNED TO DEVELOP A MINIMUM END REACTION OF 12.0 KIPS.
6.6 ALL MEMBERS AND CONNECTIONS ON THE CONTRACT DRAWINGS AND CONNECTIONS FOR ANY PORTION OF THE STRUCTURE NOT SHOWN SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA DETAILED AND SUBMITTED FOR APPROVAL AND SHOWN ON THE SHOP DRAWINGS.
6.7 ALTERNATIVE CONNECTION DETAILS MAY BE SUBMITTED ON SHOP DRAWINGS BY THE CONTRACTOR ONLY IF ACCOMPANIED BY COMPLETE STRUCTURAL CALCULATIONS PREPARED AND SEALED BY AN ENGINEER, LICENSED IN THE STATE OF SOUTH CAROLINA AND SUBMITTED FOR APPROVAL.
6.8 CALCULATIONS FOR DETAILS MUST SHOW A RATIONAL ANALYSIS OF A COMPLETE LOAD PATH, INCLUDING LOCAL EFFECTS ON WEBS, FLANGES, ETC. OF THE CONNECTED MEMBERS AND THE DEVICES (PLATES, SEATS, BRACKETS, BOLTS, WEBS, ETC) AFFECTING ALL CONNECTIONS. FAILURE TO SUBMIT SUCH CALCULATIONS FOR REVIEW CONCURRENT WITH SHOP DRAWING ERECTION PLANS AND DETAILS WILL BE CAUSE FOR REJECTION OF THAT SUBMITAL.
6.8.1 ALL SHEAR TAB CONNECTIONS SUBMITTED AS AN ALTERNATE FOR APPROVAL SHALL BE DESIGNED USING A FLEXIBLE SUPPORT CONDITION.
6.8.2 BEAM AND GIRDER CONNECTIONS SHALL BE DESIGNED SUCH THAT ALL ADDITIONAL STRESSES DUE TO CONNECTION ECCENTRICITY SHALL BE DEVELOPED BY THE CONNECTION AND NOT INDUCE ANY ADDITIONAL STRESSES INTO SUPPORTING MEMBERS.

- 6.9 STRUCTURAL STEEL DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO THE ASC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS - ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN" (JUNE 1, 1989), AND THE ASC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" (SEPTEMBER 1, 1996)
6.10 WELDING SHALL CONFORM TO THE AMERICAN WELDING SOCIETY STRUCTURAL WELDING CODE AWS D1.1. ELECTRODES FOR SHOP AND FIELD WELDS SHALL BE CLASS E70XX. ALL WELDING SHALL BE DONE BY QUALIFIED, CERTIFIED WELDERS PER THE ABOVE STANDARD.
6.11 SHOP AND FIELD TESTING OF WELDS AND BOLTS SHALL BE AS OUTLINED IN THE SPECIFICATIONS.
6.12 ALL WELDS NOT INDICATED SHALL BE A MINIMUM OF 1/4" ALL AROUND UNLESS NOTED OTHERWISE.
6.13 THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.
6.14 FOR ROOF OPENINGS, THE FABRICATOR SHALL VERIFY OPENING LOCATIONS WITH EQUIPMENT SELECTED AND MAKE ANY NECESSARY MODIFICATIONS AT NO ADDITIONAL COST. THE CONTRACTOR SHALL COORDINATE MECHANICAL UNITS AND OPENINGS & ARCHITECTURAL ITEMS REQUIRED FOR COMPLETE INSTALLATION OF WORK. IT IS THE RESPONSIBILITY OF FABRICATOR TO RECEIVE ALL NECESSARY INFORMATION PRIOR TO FABRICATION OF THE STEEL.
6.15 ALL STRUCTURAL STEEL SHALL BE PRIMED AS PER THE SPECIFICATIONS. FOR STEEL BEAMS THAT ARE PRIMED, THE TOP FLANGE RECEIVING STEEL STUDS SHALL NOT BE PRIMED PAINTED.
6.16 ALL PLATES NOT INDICATED SHALL BE 5/16" MIN THICKNESS. ALL ANGLES NOT INDICATED SHALL BE 3x3x5/16" MIN.
6.17 ALL EXTERIOR STEEL INCLUDING LINTELS SHALL BE GALVANIZED.
7.0 (NOT USED)
8.0 STEEL JOISTS
8.1 THE DETAILING, FABRICATION AND ERECTION OF STEEL JOISTS SHALL CONFORM TO THE LATEST STANDARD SPECIFICATIONS OF THE STEEL JOIST INSTITUTE. JOISTS SHALL EXTEND TO WITHIN 5/16" OF CENTERLINE OF SUPPORTING BEAMS AT ABUTTING JOISTS SPANS AND AT LEAST 2" PAST CENTERLINE OF SUPPORTING BEAMS ELSEWHERE AND IN ANY CASE JOIST ENDS SHALL BEAR A MINIMUM OF 2 1/2" ON STEEL SUPPORTS AND SHALL BE ANCHORED WITH A MINIMUM OF TWO 3/16"x1" FILLET WELDS (OR BOLTED CONNECTION) AND A MINIMUM OF 4" ON MASONRY OR CONCRETE SUPPORTS, BEARING ON A STEEL PLATE. BRIDGING SHALL BE AS SHOWN ON PLANS AS A MINIMUM AND WELDED TO INSIDE OF TOP AND BOTTOM CHORDS AND ANCHORED IN ACCORDANCE WITH SJI SPECIFICATIONS AT THEIR ENDS. EXTEND TOP AND BOTTOM CHORDS AS REQUIRED.
8.2 WHERE BOTTOM CHORD EXTENSIONS ARE REQUIRED, ALL DEAD LOADS SHALL BE IN PLACE PRIOR TO ATTACHMENT OF BOTTOM CHORD TO ADJACENT MEMBERS.
8.3 FOR CONCENTRATED LOADS GREATER THAN 500 LBS ON JOIST, NOT FALLING DIRECTLY OVER A JOIST CHORD PANEL POINT, PROVIDE WEB REINFORCING ALONG THE SUPPORT POINT TO A BOTTOM CHORD PANEL POINT. USE L 2x2x3/8" AT EACH SIDE OF JOIST AND USE 3/16" FILLET WELDS. LEAST THICKNESS OF MATERIAL USED (WHICHEVER IS LESS), U.N.O. MAXIMUM POINT LOADS SHALL NOT EXCEED 500 LBS U.N.O.
8.4 WHERE JOISTS ARE ADJACENT TO WIDE FLANGE BEAMS, ALL LINES OF BRIDGING SHALL BE ANCHORED TO THE WIDE FLANGE BEAMS.
8.5 ALL JOISTS SHALL BE PROVIDED WITH SINGLE LINES OF BOTTOM CHORD BRIDGING FOR UPLIFT AT FIRST BOTTOM CHORD PANEL POINT AT EACH END OF JOIST, FOR CLARITY THIS BRIDGING IS SHOWN ON THE ROOF FRAMING PLAN.
8.6 CAMBER FOR JOISTS SHALL BE AS SPECIFIED BY JOIST MANUFACTURER TO SUSTAIN THE LOADS AND CONDITIONS AS SPECIFIED.
8.7 STEEL JOISTS OR ROOFS SHALL BE DESIGNED FOR A NET UPLIFT AS SPECIFIED. ADDITIONAL BRIDGING MAY BE ADDED IF REQUIRED BY STEEL JOIST MANUFACTURER.
OPENINGS FOR DUCTS, VENTS, MECHANICAL EQUIPMENT, ETC. ARE TO BE FRAMED PER DWG. 54.1 UNLESS OTHERWISE SHOWN. MECHANICAL CONTRACTOR TO PROVIDE EXACT SIZES AND LOCATIONS FOR OPENINGS TO BE COORDINATED BY THE JOIST FABRICATOR.
8.9 STEEL JOIST SIZES INDICATED ARE BASED UPON THE GRAVITY LOADS REQUIRED. SPECIAL JOISTS OR JOISTS OF LARGER SIZES SHALL BE PROVIDED AS REQUIRED BY THE STEEL JOIST MANUFACTURER TO ACCOMMODATE SITUATIONS AND LOADS INDICATED ON THE DRAWINGS, SUCH AS HANGER LOADS, UPLIFT LOADS, WIND LOADS, CHORD EXTENSION LOADS, ETC. ALL JOIST SIZES SHOWN ON DRAWINGS ARE GIVEN AS MINIMUM AND SHALL MEET THE MINIMUM UNIFORM LOAD CARRY CAPACITIES AS SPECIFIED BY THE STEEL JOIST INSTITUTE. STEEL JOISTS DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA.
8.10 JOISTS SHALL BE DESIGNED FOR LOAD REVERSAL AS REQUIRED.
8.11 JOISTS SHALL BE DESIGNED FOR CONCENTRATED LOADS APPLIED CONCURRENTLY WITH THE APPLICABLE LOAD CONDITIONS INCLUDING ROOF LIVE LOADS. BOTTOM CHORD PANEL POINTS SHALL BE DESIGNED FOR MAXIMUM HANGER LOADS OF 500 LBS APPLIED AT ANY PANEL POINT.
8.12 ALL STEEL JOISTS SHALL BE PRIMED UNLESS FIREPROOFED.
8.13 ALL BRIDGING SHOWN IS THE MINIMUM REQUIRED.
8.14 MINIMUM JOIST SEAT END BEARING BOUNDARY SHEAR 'Q' = 400 LF U.N.O. ALL K-SERIES JOIST SEATS SHALL BE 2 1/2" MIN. U.N.O. ALL LH-SERIES JOIST SEATS SHALL BE 5" MIN. U.N.O.
8.15 ALL BRIDGING SHOWN IS THE MINIMUM REQUIRED.
8.16 JOIST LOAD TO BE APPLIED AT CENTER OF CMU WALL SO THAT IT DOES NOT CREATE AN ECCENTRIC CONDITION (Y/P)
9.0 BRICK OR CMU LEDGE ANGLES/PLATES
9.1 ALL LEDGES SHALL BE 1/2" MIN THICKNESS AND SHALL EXTEND TO WITHIN 1" (UNO) OF OUTSIDE FACE OF WALL.
9.2 ALL LEDGES SHALL BE DISCONTINUOUS AT CONTROL AND EXPANSION JOINTS.
9.3 ALL LEDGES EXPOSED TO WEATHER SHALL BE GALVANIZED.
9.4 ALL LEDGES SHALL BE FIELD WELDED TO ACCOMMODATE VARIATIONS IN SWEEP, CAMBER, THICKNESS OF MORTAR JOINTS, CONSTRUCTION TOLERANCES, DEFLECTION OF MEMBERS DURING PLACEMENT OF DEAD LOAD, ETC. TO ENSURE PROPER ALIGNMENT AND PLACEMENT OF LEDGES WITH BRICK AND CMU MORTAR JOINTS, CONTROL & EXPANSION JOINTS, FINISHED FLOOR ELEVATIONS, ETC. ALL BENT PLATE LEDGES ARE INDICATED TO BE ONE PIECE. THE SUBCONTRACTOR SHALL VERIFY AND COORDINATE ALL CONDITIONS (INCLUDING COORDINATION WITH THE BRICK MASONRY CONTRACTOR) TO ENSURE PROPER INSTALLATION OF WORK. ANY MODIFICATIONS REQUIRED FOR COMPLETION OF WORK WILL BE AT NO ADDITIONAL COST TO THE OWNER.

- 10.0 (NOT USED)
11.0 METAL WALKWAY COVERS AND CANOPIES
11.1 DESIGN CALCULATIONS SHOWING ALL INPUT DATA AND REACTIONS SHALL BE SUBMITTED FOR REVIEW WITH THE METAL WALKWAY COVER/CANOPY SHOP DRAWINGS. ALL CALCULATIONS SHALL HAVE THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA. ALL FRAME REACTIONS TO BE FURNISHED TO ARCHITECT AND ENGINEER FOR COORDINATION WITH FOUNDATIONS.
11.2 ALL METAL WALKWAY COVER/CANOPY COMPONENTS SHALL BE DESIGNED BASED ON THE SPECIFICATIONS AND ON THE FOLLOWING LOADS AND CRITERIA:
DEAD LOADS:
FRAME, ROOFING AND PURLINS - PER MANUFACTURER
ADDITIONAL COLLATERAL LOADS:
4 PSF U.N.O.
LIVE LOADS:
20 PSF WITH STANDARD BUILDING CODES TRIBUTARY AREA LIVE LOAD REDUCTION ALLOWED U.N.O.
WIND:
PER IBC 2012 AND AMENDMENTS LISTED ABOVE AND 133 MPH WIND VELOCITY.
SEISMIC:
PER IBC 2012 AND AMENDMENTS LISTED ABOVE WITH COEFFICIENTS AND CATEGORIES FROM THE DESIGN LOADS AND CRITERIA NOTES.



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Client
ALLENDALE - FAIRFAX COUNTY SCHOOL DISTRICT

Project
ALLENDALE PRIMARY SCHOOL

Issued For
100% CONSTRUCTION DRAWINGS
JUNE 05, 2020

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Key Plan

Sheet Title
GENERAL NOTES

S-001
Drawn By ERB
Checked By JKC

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