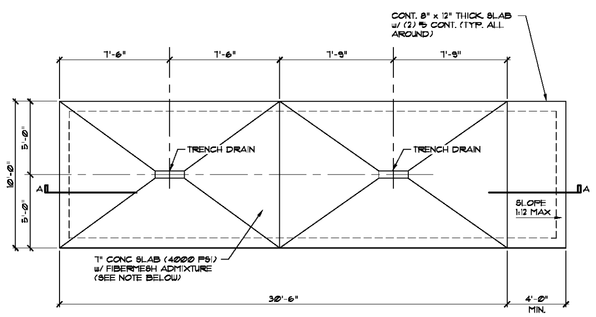
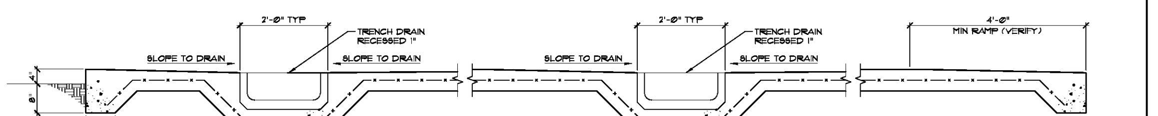


COMPACTOR SLAB FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

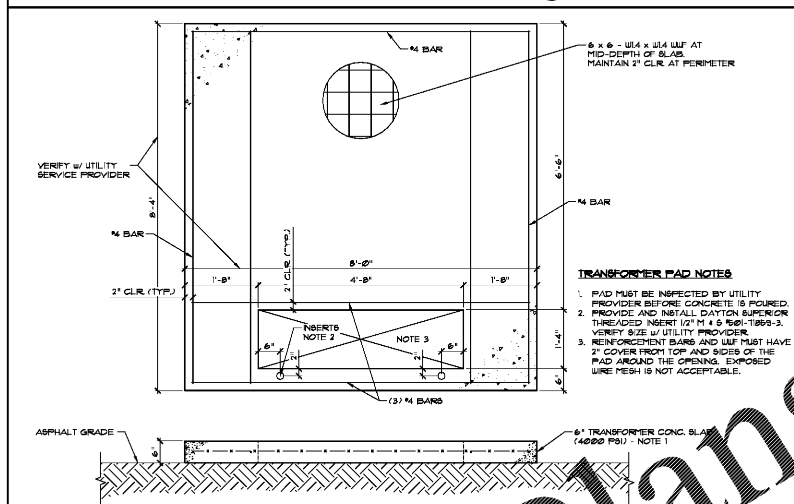
FIBER REINFORCEMENT SHALL BE 100% VIRGIN POLYPROPYLENE, FIBRILLATED FIBERS, SPECIFICALLY MANUFACTURED FOR USE AS CONCRETE SECONDARY REINFORCEMENT (ASTM C-1116). MINIMUM VOLUME OF FIBERGLASS ADMIXTURE PER CUBIC YARD OF CONCRETE IS 0.1 PERCENT (1.5 LBS.). THE TYPICAL FIBER LENGTH IS 1/2 TO 3/4 INCHES.



SECTION A-A

SCALE: 3/4" = 1'-0"

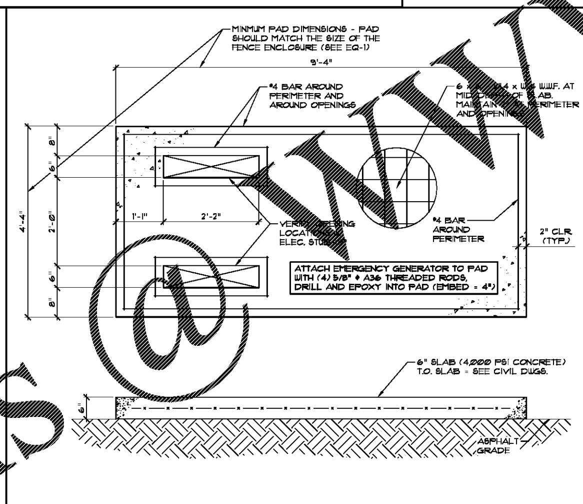
1 S-5 PLAN / SECTIONS
SCALE: AS NOTED



TRANSFORMER PAD NOTES

1. PAD MUST BE INSPECTED BY UTILITY PROVIDER BEFORE CONCRETE IS POURED.
2. PROVIDE AND INSTALL DAYTON SUPERIOR THREADED INSERT 1/2" M & S #501-71859-3. VERIFY SIZE w/ UTILITY PROVIDER.
3. REINFORCEMENT BARS AND WWF MUST HAVE 2" COVER FROM TOP AND SIDES OF THE PAD AROUND THE OPENING. EXPOSED WIRE MESH IS NOT ACCEPTABLE.

2 S-5 SECTION
SCALE: 3/4" = 1'-0"



3 S-5 SECTION
SCALE: 3/4" = 1'-0"

STRUCTURAL GENERAL NOTES

DESIGN CRITERIA

1. Building Code: 2015 International Building Code w/ Georgia State Amendments
2. Design Live Load:
   Roof = 30 PSF
3. Design Dead Loads:
   Roof = 20 PSF
4. Ground Snow Load (Pg) = 5 PSF
5. Ultimate Design Wind Speed = 115 MPH
   Nominal Design Wind Speed = 89 MPH
6. Risk Category II
7. Wind Exposure Category = C
8. Internal Pressure (GCpi) = (+/−) 0.18 − Enclosed
9. Seismic Importance Factor (Ie) = 1.0, Occupancy Category II
10. Mapped Spectral Response Accelerations:
    Ss = 0.210g
    S1 = 0.094g
11. Site Class = D
12. Spectral Response Coefficients:
    Sds = 0.224g
    Sd1 = 0.151g
13. Seismic Design Category = C
14. Basic Seismic Force Resisting System:
    Bearing Wall System
15. Seismic Response Coefficient (Cs) = 0.045g
16. Response Modification Factor (R) = 5
17. Analysis Procedure:
    Equivalent Lateral Force
18. Other Loads: Contractor shall submit loading information for all equipment not shown on the drawings, or differing from those shown on the drawings.

MISCELLANEOUS

1. The Contractor is responsible for the means and methods of the construction of the contents of these documents. This shall include, but is not limited to, temporary bracing, shoring, tie downs and other provisions which will ensure the safety at the jobsite until the entire structural system has been installed.
2. Do not scale these drawings. Coordinate all dimensions, elevations and openings with the architectural drawings and all other trades. Report to this office any discrepancies or omissions found in the contract document.
3. The general contractor shall coordinate all aspects of all drawings prior to fabrication of any structural components and final bidding.

SUBMITTALS

1. All shop drawings shall be submitted and approved prior to construction. Allow (10) working days for approval from this office. Drawings / submittals shall be signed and sealed by a Georgia licensed engineer.
2. Poured in Place Concrete:
   a) Proposed concrete mix design in accordance with ACI 301 Chapter 3.
   b) Detailed shop drawings of reinforcing bars showing number, size, and location.
   c) Formwork and shoring drawings as required by the Georgia Threshold Law.
3. Concrete Masonry Units:
   a) Detailed shop drawings of reinforcing bars showing number, size, and location.
   b) Type of units and compression test results.
4. Structural Steel: Detailed shop drawings showing all member sizes, welds, bolts, connection details, layout, etc, as required to fabricate and erect the steel framing.
5. Cold-Formed Steel: Detailed drawings showing layout, size, gauge, bracing, fastening and all accessory materials.

CONCRETE MASONRY UNITS

1. Blocks shall be normal weight Grade N, Type II hollow load bearing masonry units which conform to ASTM C90.
   a) Net compressive test strength of units = 1900 PSI
   b) f'm = 1500 PSI
2. Mortar shall be type M or S and conform to ASTM C270.
3. Fill all cells containing reinforcing with course grout.
4. Coarse grout shall conform to ASTM C476
   a) 3000 PSI at 28 days
   b) 3/8" aggregate
   c) 8"−10" slump
5. All work shall conform to ACI 530.1.
6. Reinforcing bars shall be lapped 48 diameters where spliced.
7. All vertical bars shall be held in position at the top and bottom and with a minimum clearance of 1/4" to the masonry walls and one diameter between bars.
8. Horizontal reinforcing shall confirm to ASTM A82.
9. Horizontal reinforcing shall be 9 gauge (U.N.O.) ladder type Dur−o−wall (or equal) at 16" o.c. and shall be lapped a minimum of 6" at splices.
10. Store masonry on pallets and cover with visqueen.
11. Masonry shall be placed in running bond with 3/8" face shell bedding mortar joints, vertical and horizontal.
12. Grouting options
    a) 4' high lifts with no observation holes.
    b) 8' high lifts with observation holes. Observation holes shall be 4" x 4" sawcuts at the base course of all reinforcing.
13. Space masonry control joints at a maximum of 34'−0" o.c. (U.N.O.).
14. Masonry inspection shall be provided per ACI 530.
15. Provide a 8" x 8" reinforced precast lintels at all openings less than 8'−0" o.c. (U.N.O.) with a minimum end bearing = 8".

STRUCTURAL STEEL

1. Codes and Standards
   a) AISC "Specifications for the Design, Fabrication and Erection of Structural Steel for Buildings", − Latest Edition.
   b) Welds shall conform to the American Welding Society, AWS D1.1 using E70 electrodes.
   c) Bolts and bolted connections shall conform to "Specifications for Structural Joints Using ASTM A325 or A490 Bolt" Use bearing type bolts with threads across the shear plane.
   d) Structural steel shapes, plates, channels, angles − ASTM A36
   e) W−shapes − ASTM A572 Grade 50
   f) Steel tubing − ASTM A501
   g) Steel pipe − ASTM A53, Type E or S, Grade B
   h) Anchor bolts − ASTM A307 (U.N.O.) w/ nuts and washers.
   i) Column splices − AISC "Structural Steel Detailing" Table A−7 Page 11−A12.
2. No splices shall be allowed in any structural steel member unless shown on approved shop drawings.
3. Framing connections shall be double angle type (UNO). Design connections for 55% of the total load capacity derived from the uniform load constants table, Part 2 of the ASIC Code.
4. Verify the location and size of all floor and roof openings with the appropriate sub−contractor.
5. All members except those to be field welded shall receive one coat of shop primer paint. See architectural drawings for preferred color.
6. Steel stairs shall be designed by a licensed engineer retained by the fabricator for a live load = 100 PSF.
7. No openings shall be allowed in steel members unless shown on the drawings.
8. Openings, copes, and other steel cutting shall have a 1/2" minimum radius.
9. All beams shall be fabricated and erected with the natural camber up.
10. Welds not designed shall be a fillet weld equal to 1/16" less than the least thick member, all welds shall be cleaned and painted.
11. Non−shrink grout (ASTM C 1107) to be 6000 psi (min), non−ferrous and non−corrosive.
12. A qualified testing laboratory shall be retained to perform the following tests:
    a) Inspect all steel members and connections
    b) Test 50% of full penetration welds.

Send copy of report to the owner, architect, engineer, and contractor.

COLD−FORMED STEEL

1. Codes and Standards:
   a) "The Specifications for the Design of Cold−Formed Steel Structural Members" by AISI
   b) "Code for Welding in Building Construction, D1.0" by the AWS
   c) 18 gauge and lighter = ASTM A446, Grade A minimum yield strength = 33,000 PSI.
   d) 16 gauge and heavier = ASTM A446, Grade D minimum yield strength = 50,000 PSI.
   e) Members shall be galvanized finish per ASTM A525 G60.
2. Provide manufacturer's recommended standard steel tracks, blocking, lintels, clips, bracing, screws, web stiffeners and accessories as needed to properly complete the framing.
3. Bridging shall be provided and installed per manufacturer's recommendations
4. Members shall not be spliced.
5. Provide members which conform to the properties shown on the drawings.
6. All framing members shall be designed by the manufacturer to support all live, dead and wind loads shown on the drawings with a maximum deflection equal to L/360 (U.N.O.).

POURED−IN−PLACED CONCRETE

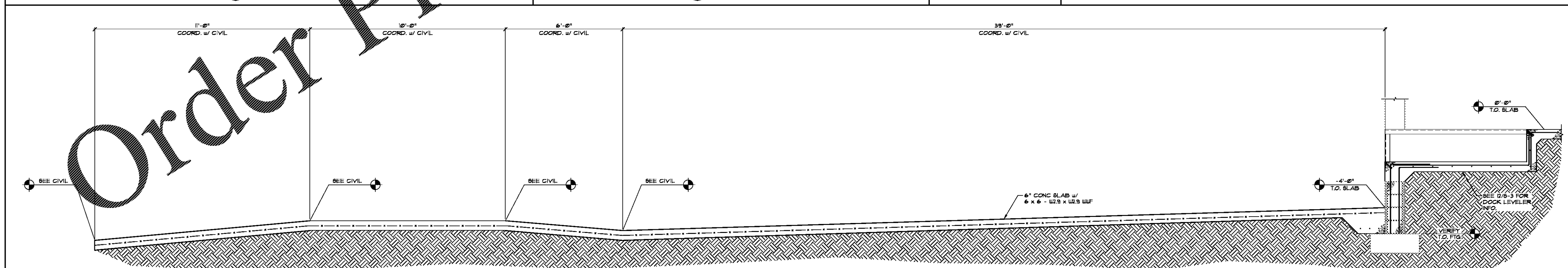
1. Codes and Standards:
   ACI 318 "Building Code Requirements for Reinforced Concrete"
   ACI 315 "Details and Detailing of Concrete Reinforcement"
   ACI 301 "Specifications for Structural Concrete for Buildings"
2. Concrete is to be normal weight conforming to ASTM [illegible]
   a) Portland Cement − ASTM C150, Type I
   b) Aggregates (3/4" maximum) − ASTM C33
   c) Air entraining − ASTM [illegible]
   d) Water Reducing − ASTM C494
   e) Flyash (20% maximum) − Class F ASTM [illegible]
   f) Water − Potable
   g) Slump Range − [illegible] − 5"
   h) Placement Time (Maximum) − [illegible]0 minutes from batch time
3. Minimum compressive strength at 28 days:
   a) Footings, slab on form deck, masonry fill − 3000 PSI
   b) Columns, beams, walls − 4000 PSI
4. Reinforcing steel is to be Grade 60 conforming to ASTM A615
   a) Minimum lap splice − 30 diameters (U.N.O.)

BAR SIZE	f'c	Splice Length
[illegible] & SMALLER	3000 psi	58 BAR Diameters
#6 & SMALLER	4000 psi	50 BAR Diameters
#7 & LARGER	3000 psi	72 BAR Diameters
#7 & LARGER	4000 psi	63 BAR Diameters

   b) Minimum bar cover:
      Footings, retaining wall − 3"
      Columns, beams, slabs − 1 1/2"
   c) Provide corner bars which match the horizontal bars at all wall footings and tie beams.
   d) Welding reinforcing if required shall conform to AWS D1.4.
5. Slab−on−Grade: Installation shall conform to ACI 302.1R.
   a) Welded wire fabric − ASTM A185, ASTM A497; lap mesh 6" minimum at joints
   b) Moisture barrier − Class A 15 mil minimum
   c) Compressive strength − 3000 PSI
   d) Sawcut joints and construction joints shall be cleaned and filled with liquid sealant as required by owner.
   e) Contractor shall have a thorough understanding of the owner's expectation of the slab−on−grade (cracks, levelness, etc.) and shall provide adequate equipment, labor and materials (including water−reducing agents, installation and curing prcedures, etc.) to assure a slab that will be acceptable to the owner.
   f) Contractor shall replace or repair (at his cost) any portion of the slab that is not acceptable to the owner.
6. Install ties, spacers, chairs, etc (per CRSI recommendations) necessary to securely hold reinforcing during concrete placement. Use plastic tips at all exposed surfaces.
7. Use internal vibrators to consolidate all concrete.
8. Concrete curing options
   a) Liquid membrane forming chemical compound conforming to ASTM C309.
   b) Continuous moisture in accordance with ACI 301.
9. Concrete shall be finished per architectural drawings.
10. A testing laboratory shall perform the following concrete tests for each 50 cubic yards. Send test results to the owner, architect, structural engineer and General Contractor.
    a) Slump test − ASTM C143
    b) Four Cylinder strength test − ASTM C39: test one cylinder after 7 days, test two after 28 days and hold one in reserve.
11. All beams shall be poured monolithically.
12. Exposed edges of columns and beams shall be chamfered 3/4" unless noted otherwise on architectural drawings.
13. Coat all forms with a commercial compound that will not bond or adversely affect the concrete.
14. The contractor is responsible for the proper design of all formwork and shoring. Design shall be performed by a licensed engineer.
15. Coordinate locations of all openings, embeds and accessories that are required by all trades. No opening or sleeve may be placed in beams or columns unless approved by the engineer.
16. Proper placement of all embeds, anchor bolts, and etc shall be verified prior to placing the concrete. Notify the engineer of any conflicts.

EXISTING BUILDING

1. The information shown on all the existing construction has been obtained from the best possible methods. The drawings do not show as−built conditions and may differ from the actual construction of the existing building. The contractor shall verify all information shown and notify the engineer of any variations.



4 S-5 SECTION
SCALE: 1/2" = 1'-0"

center for innovative structures, inc.
Structural Engineers

PUBLIX PROTOTYPE BASIS OF DESIGN
20M
EXPANDED
RECEIVED: 09.14.18

CORPORATE OFFICES, FACILITIES DESIGN
BOX 407, LAKELAND, FL 33802
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REVISIONS:

VAN GLADFELTER PE 23150

S-5

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Project No.: 20-133