

General Notes



hendon thuckestein architects, PC
 architecture . planning . interior design
 2126 Morris Avenue
 Birmingham, AL 35203
 Phone (205) 322-1731
 Fax (205) 322-1778
 email info@hplusha.com
 www.hplusha.com

- 1. GENERAL**
- 1.1. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS ARE A PORTION OF THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR AND SUBCONTRACTORS SHALL REFERENCE AND COORDINATE WITH ALL OTHER DISCIPLINES' DRAWINGS. ANY DISCREPANCIES OR OMISSIONS SHALL BE REPORTED TO THE STRUCTURAL ENGINEER AND ARCHITECT.
- 1.2. DESIGN CRITERIA:
- A. CODES AND SPECIFICATIONS:
- GENERAL BUILDING CODE: INTERNATIONAL BUILDING CODE, 2015 EDITION.
 - DESIGN LOAD CRITERIA: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, AMERICAN SOCIETY OF CIVIL ENGINEERS, ASCE 7-10.
 - CONCRETE: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, AMERICAN CONCRETE INSTITUTE, ACI 318-14.
 - STRUCTURAL STEEL: SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AISC 360-10.
 - COLD-FORMED METAL FRAMING: NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, AMERICAN IRON AND STEEL INSTITUTE.
 - TIMBER: NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, AMERICAN FOREST & PAPER ASSOCIATION/AMERICAN WOOD COUNCIL (2008 EDITION).
- B. DESIGN LOADS (PSF):
- DEAD LOADS: ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS SHALL BE REPORTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOAD-CARRYING CAPACITY OF THE STRUCTURE.
 - LIVE LOADS: ROOF _____ 20
 - SNOW LOAD: GROUND SNOW LOAD (Pg) _____ 0.0
 - WIND LOADS: BASIC WIND SPEED (3 - SECOND GUST) _____ 120 MPH
 WIND IMPORTANCE FACTOR (Iw) _____ 1.0
 BUILDING CATEGORY _____ II
 WIND EXPOSURE CATEGORY _____ B
 INTERNAL PRESSURE COEFFICIENT _____ 0.18
 WALL COMPONENT AND CLADDING WIND PRESSURE- SEE SHEET S1.5.
 - SEISMIC LOADS: SEISMIC IMPORTANCE FACTOR (Ie) _____ 1.0
 MAPPED SPECTRAL RESPONSE ACCELERATIONS:
 S_s _____ 0.093
 S₁ _____ 0.059
 SITE CLASS (ASSUMED) _____ D
 SITE COEFFICIENTS:
 F_a _____ 1.600
 F_v _____ 2.400
 DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS:
 S_{ds} _____ 0.099
 S_{d1} _____ 0.094
 SEISMIC DESIGN CATEGORY _____ B
 BASIC SEISMIC-FORCE-RESISTING SYSTEM: LIGHT-GAGE METAL STUD WALLS WITH WOOD PANEL SHEATHING.
 DESIGN BASE SHEAR _____ 1.2 KIPS
 SEISMIC RESPONSE COEFFICIENT (Cs) _____ 0.015
 RESPONSE MODIFICATION FACTOR (R) _____ 6 1/2
 OVER-STRENGTH FACTOR (Ω_o) _____ 3
 DEFLECTION AMPLIFICATION FACTOR (Cd) _____ 4
 ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD
- 1.3. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO FABRICATION/CONSTRUCTION. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES PRIOR TO FABRICATION/CONSTRUCTION.
- 1.4. INSPECTIONS/OBSERVATIONS:
- A. SPECIAL INSPECTIONS ARE REQUIRED FOR THIS PROJECT IN ACCORDANCE WITH INTERNATIONAL BUILDING CODE. REFER TO PROJECT SPECIFICATIONS AND DRAWINGS.
- B. STRUCTURAL OBSERVATION BY STRUCTURAL ENGINEER'S OFFICE:
- STRUCTURAL OBSERVATION IS VISUAL OBSERVATION OF THE IN-PLACE STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED CONSTRUCTION ELEMENTS AT THE TIME OF THE OBSERVATION.
 - CONTRACTOR SHALL NOTIFY STRUCTURAL ENGINEER AND ARCHITECT, PER THE SCHEDULE STATED BELOW, WHEN SUCH ITEMS HAVE PROGRESSED TO THE POINT WHERE THEY WILL BE IN PLACE AND READY FOR REVIEW. FAILURE TO NOTIFY MAY REQUIRE REMOVAL OF COMPLETED CONSTRUCTION.
- | NOTIFY PRIOR TO THE FOLLOWING SCHEDULED TASKS | REQUIRED DAYS NOTIFICATION |
|-----------------------------------------------|----------------------------|
| ROOF TRUSSES ERRECTED | 7 DAYS |
- C. OBSERVATION BY THE STRUCTURAL ENGINEER'S OFFICE DOES NOT REPLACE INSPECTIONS AND TESTING BY THE TESTING AGENCY OR SPECIAL INSPECTOR.

- 1.5. SUBMITTALS:
- A. REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. ALL SHOP DRAWINGS MUST BE REVIEWED AND "APPROVED" BY THE CONTRACTOR PRIOR TO SUBMITTAL.
- B. HARD COPY SHOP DRAWING SUBMITTALS: SUBMIT ALL SHOP DRAWINGS ON THREE PRINTS ONLY. ONE PRINT WILL BE RETURNED TO THE CONTRACTOR. ALL PRINTS REQUIRED BY THE CONTRACTOR ARE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE MADE AFTER APPROVED SHOP DRAWINGS ARE RETURNED. IF ADDITIONAL PRINTS ARE SUBMITTED, THEY WILL BE RETURNED UNMARKED.
- C. ELECTRONIC SHOP DRAWING SUBMITTALS: SUBMIT ALL ELECTRONIC SHOP DRAWINGS IN .PDF FORMAT. REVIEWED SHOP DRAWINGS WILL BE RETURNED IN .PDF FORMAT. ALL PRINTS REQUIRED BY THE CONTRACTOR ARE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE MADE AFTER APPROVED SHOP DRAWINGS ARE RETURNED.
- D. RESUBMITTED SHOP DRAWINGS: RESUBMITTED SHOP DRAWINGS SHALL HAVE ALL CHANGES SINCE THE PREVIOUS SUBMISSION IDENTIFIED BY CLOUDING OR OTHER CLEAR COMMUNICATION. RE-REVIEWED SHOP DRAWINGS WILL ONLY BE REVIEWED FOR IDENTIFIED CHANGES.
- E. SHOP DRAWINGS: THE CONTRACTOR SHALL SUBMIT FOR STRUCTURAL ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS. ITEMS MARKED (*) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED.
- CONCRETE MIX DESIGNS
 - CONCRETE REINFORCING
 - STRUCTURAL STEEL
 - COLD-FORMED METAL FRAMING
 - SHOP FABRICATED WOOD TRUSSES (*)
- F. DESIGN CALCULATIONS: THE CONTRACTOR SHALL SUBMIT FOR STRUCTURAL ENGINEER'S RECORD, DESIGN CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED FOR THE FOLLOWING ITEMS:
- SHOP FABRICATED WOOD TRUSSES
- 1.6. ALL DETAILS SHOWN ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS, UNLESS NOTED.
- 1.7. THE CONTRACTOR IS RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
- 1.8. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED ROOFS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT LOADS DO NOT EXCEED THE DESIGN LIVE LOAD.
- 2. FOUNDATION**
- 2.1. A GEOTECHNICAL ENGINEER SHALL PROVIDE COMPACTED FILL REQUIREMENTS FOR THE BUILDING PAD AND REVIEW THE FOUNDATION BEARING SURFACE TO VERIFY THE ASSUMED ALLOWABLE BEARING PRESSURE NOTED. DO NOT PLACE CONCRETE PRIOR TO GEOTECHNICAL ENGINEER'S APPROVAL.
- 2.2. ASSUMED MAXIMUM ALLOWABLE BEARING PRESSURES (PSF):
 COLUMN FOOTINGS _____ 2000
 CONTINUOUS WALL FOOTINGS _____ 1500
- 2.3. THE CONTRACTOR SHALL OBTAIN A COPY OF THE GEOTECHNICAL REPORT FROM THE OWNER AND FOLLOW ALL REQUIREMENTS AND RECOMMENDATIONS.
- 2.4. ALL FOUNDATION BEARING SURFACES SHALL BE REVIEWED BY THE GEOTECHNICAL ENGINEER PRIOR TO POURING CONCRETE TO ENSURE THEIR COMPLIANCE WITH PRESSURES NOTED. ALL FOOTING ELEVATIONS ARE ESTIMATED AND MAY BE ADJUSTED IN THE FIELD BY THE GEOTECHNICAL ENGINEER.
- 2.5. COMPACTED FILL SHALL MEET THE REQUIREMENTS NOTED IN THE GEOTECHNICAL REPORT.
- 2.6. PROVIDE 4" COMPACTED GRANULAR FILL BENEATH ALL EARTH SUPPORTED SLABS. PROVIDE 1/4" MIL VAPOR RETARDER BETWEEN BOTTOM OF SLAB AND TOP OF GRANULAR FILL.
- 3. CONCRETE**
- 3.1. CONCRETING OPERATIONS SHALL COMPLY WITH ACI STANDARDS.
- 3.2. MINIMUM CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS (PSI), TYPE OF CONCRETE, MAXIMUM W/C (WATER/CEMENTITIOUS MATERIALS RATIO), AIR CONTENT, SLUMP AND CONCRETE USE:
- | STRENGTH | TYPE | W/C | AIR | SLUMP | USE |
|----------|------------|------|------|----------|---------------|
| 3000 | NORMAL WT. | 0.50 | 3-5% | 3" TO 5" | UNLESS NOTED |
| 4500 | NORMAL WT. | 0.45 | 1-3% | 3" TO 5" | SLAB ON GRADE |
- 3.3. REINFORCING BARS: ASTM A615 GRADE 60.
- 3.4. WELDED WIRE REINFORCEMENT (WWR): ASTM A185. MINIMUM LAP AND EMBEDMENT TO BE THE GREATER OF ONE CROSS WIRE SPACING PLUS 2" OR 6".
- 3.5. REINFORCING STEEL SHOWN IN SECTIONS AND DETAILS ARE A SCHEMATIC INDICATION THAT REINFORCING EXISTS. SEE SCHEDULES, SECTION NOTES AND GENERAL NOTES FOR ACTUAL REINFORCING REQUIRED.
- 3.6. REINFORCING BAR PLACING ACCESSORIES IN ACCORDANCE WITH ACI MANUAL OF STANDARD PRACTICE. WHERE CONCRETE IS EXPOSED IN FINISHED BUILDING, PROVIDE ACCESSORIES WITH RUSTPROOF LEGS.

- 3.7. DETAIL REINFORCEMENT IN ACCORDANCE WITH ACI 315. REINFORCEMENT SHALL NOT BE WELDED UNLESS NOTED OR APPROVED BY THE STRUCTURAL ENGINEER.
- 3.8. ALL SPLICES SHALL BE CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.
- 3.9. ALL REINFORCING MARKED "CONTINUOUS" SHALL BE SPLICED WITH CLASS "B" TENSION LAP SPLICE, UNLESS NOTED.
- 3.10. CONCRETE COVERAGE OF REINFORCEMENT, UNLESS NOTED:
 FOOTINGS _____ 2" TOP & 3" BOTTOM & SIDES
- 3.11. EARTH SUPPORTED SLABS: 4" THICK, REINFORCED WITH 6X6 W2.9/W2.9 WWR AT MID-DEPTH OF SLAB, UNLESS NOTED.
- 4. STRUCTURAL STEEL**
- 4.1. FABRICATE AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
- 4.2. THE STEEL FRAME IS "NON-SELF-SUPPORTING". ADEQUATE TEMPORARY SUPPORT MUST BE PROVIDED BY THE CONTRACTOR UNTIL THE LATERAL LOAD RESISTANCE AND STABILITY OF THE COMPLETED STRUCTURE IS IN PLACE. LATERAL LOAD RESISTANCE AND STABILITY OF THE BUILDING IN THE COMPLETED STRUCTURE IS PROVIDED AS FOLLOWS:
- ROOF DIAPHRAGM: WOOD SHEATHING
 - LATERAL LOAD RESISTING SYSTEM: LIGHT-GAGE METAL STUD WALLS WITH WOOD PANEL SHEATHING.
- 4.3. STRUCTURAL STEEL: ASTM A992 FOR WIDE FLANGE BEAMS ASTM A36 FOR STIFFENER PLATES, BASE PLATES, COLUMN CAP PLATES, BEAM CONNECTION PLATES AND STEEL ANGLES.
- 4.4. HOLLOW STRUCTURAL SECTIONS: ASTM A500, GRADE B.
- 4.5. WELDED CONNECTIONS: E70XX ELECTRODES, MINIMUM SIZE FILLET WELD 3/16".
- 4.6. HEADED ANCHOR RODS: ASTM F1554 GRADE 36 ANCHOR AND HEAVY HEX BOLT UNLESS INDICATED.
- 4.7. BOLTED CONNECTIONS: BEARING TYPE A325-N IN ACCORDANCE WITH AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM ANCHOR A490 BOLTS". BOLTS THROUGH 4" WIDE BEAM FLANGES SHALL BE 5/8" DIAMETER. OTHER BOLTS SHALL BE 3/4" DIAMETER. USE SINGLE LIGHT BEARING CONNECTIONS FOR ALL BOLTED CONNECTIONS.
- 4.8. BOLTS SHOWN IN SECTIONS AND DETAILS ARE A SCHEMATIC INDICATION THAT BOLTS MAY BE USED. ANCHOR NUMBER, UNLESS SPECIFIED, TO BE IN ACCORDANCE WITH AISC.
- 4.9. ALL STRUCTURAL STEEL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR TO RESIST FORCES INDICATED.
- 4.10. ALL NON-COMPOSITE BEAM CONNECTIONS SHALL BE "SIMPLE SHEAR CONNECTIONS", UNLESS NOTED. WHERE BEAM REACTIONS AND/OR DESIGN FORCES ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS, THE CONNECTIONS SHALL BE DESIGNED TO SUPPORT A REACTION EQUAL TO ONE-HALF THE TOTAL UNIFORM LOAD CAPACITY FROM THE MAXIMUM TOTAL UNIFORM LOAD TABLE MULTIPLIED BY A FACTOR OF 1.2 FOR GIVEN SHAPE, SPAN, AND GRADE OF STEEL.
- 4.11. WHERE STEEL BEAMS ARE CONTINUOUS OVER COLUMNS, PROVIDE WEB STIFFENER PLATES EACH SIDE OF BEAM WEB, OF THICKNESS EQUAL TO BEAM FLANGE THICKNESS, LOCATED IN ALIGNMENT WITH COLUMN WEB, FLANGES OR CENTER LINE OF TUBES AND PIPE COLUMNS.
- 5. COLD-FORMED METAL FRAMING (LOAD BEARING STUDS)**
- 5.1. STRUCTURAL PROPERTIES OF STUDS SHALL BE COMPUTED IN ACCORDANCE WITH AISC "SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS."
- 5.2. SUBMIT THE FOLLOWING:
- PRODUCT DATA: FOR EACH TYPE OF COLD-FORMED METAL FRAMING PRODUCT AND ACCESSORY UTILIZED.
 - SHOP DRAWINGS: SHOW LAYOUT, SPACINGS, SIZES, THICKNESS, AND TYPES OF COLD-FORMED METAL FRAMING; FABRICATIONS; AND FASTENING AND ANCHORAGE DETAILS, INCLUDING MECHANICAL FASTENERS. SHOW REINFORCING CHANNELS, OPENING FRAMING, SUPPLEMENTAL FRAMING, STRAPPING, BRACING, BRIDGING, SPLICES, ACCESSORIES, CONNECTION DETAILS, AND ATTACHMENT TO ADJOINING WORK.
- 5.3. LOAD-BEARING STEEL STUDS SHALL BE EQUAL TO SSMA TYPE 600S200-54 (50 KSI) SPACED @ 12", UNLESS NOTED. AT EXTERIOR WALLS, PROVIDE THREE ROWS OF LATERAL BRACING AT PANEL EDGES. ELSEWHERE PROVIDE THREE ROWS OF LATERAL BRACING PER MANUFACTURER'S RECOMMENDATIONS.
- 5.4. STUD TRACK SHALL BE EQUAL TO SSMA TYPE 600T200-54. TRACK SPLICE SHALL OVERLAP MINIMUM OF 8" WITH (10) FASTENERS PER SPLICE. SCREW WOOD DOUBLE TOP PLATE TO METAL STUD TRACK WITH (2) SSMA #12X4" SCREWS AT 6". SCREWS SHALL PENETRATE THROUGH BOTH WOOD TOP PLATES AND METAL TRACK AND BE STRENGTHENED FROM TOP TO BOTTOM.
- 5.5. ATTACH STUDS TO METAL TRACKS WITH (2) #10-16 SCREWS AT EACH END.
- 5.6. VERTICAL STUDS SHALL BE 100% END BEARING.
- 5.7. PROVIDE WALL BRACING, CONNECTION DETAILS AND WINDOW HEADERS AS RECOMMENDED BY THE STUD MANUFACTURER FOR LOAD BEARING STUDS.
- 5.8. VERTICAL STUDS INTERRUPTED BY WALL OPENINGS SHALL BE LOCATED EQUALLY ON EACH SIDE OF THE OPENING. PROVIDE EVEN NUMBERS OF FULL HEIGHT STUDS ON EACH SIDE OF OPENING. AT ALL GROUPED STUDS, SCREW STUD FLANGES TOGETHER EACH SIDE WITH SSMA #10 SCREWS AT 6"; STAGGER TO AVOID DRILLING INTO IN-PLACE SCREWS.

- 5.9. SUBMIT SHOP DRAWINGS INDICATING COLD-FORMED MEMBER SIZES AND LOCATIONS. PROVIDE PLAN LAYOUT INDICATING SPACING AND LOCATIONS OF GROUPED STUDS. PROVIDE ELEVATIONS OF OPENINGS SHOWING SIZES AND LAYOUT OF GROUPED STUDS AND HEADERS KEYS TO THE LAYOUT PLAN. PROVIDE PRODUCT DATA FOR MEMBERS.
- 5.10. COORDINATE LAYOUT OF METAL STUDS WITH LOCATION OF ELECTRICAL OUTLETS. SEE ARCHITECTURAL AND ELECTRICAL DRAWINGS FOR LOCATIONS.
- 5.11. ATTACH BOTTOM STUD TRACK TO FOUNDATION WITH HILTI X-U PAF EMBED 1 1/4" AT 6" SPACING.
- 5.12. SHEET STEEL: ASTM A1003/A1003M STRUCTURAL GRADE 5 METAL L COATED OF GRADE AND COATING WEIGHT AS FOLLOWS:
- GRADE:
 - STUDS OR JOISTS 43 MIL (18 GAUGE) OR THINNER SHALL HAVE A MINIMUM YIELD STRENGTH OF 33 KSI
 - STUDS OR JOIST 54 MIL (16 GAUGE) OR THINNER SHALL HAVE A MINIMUM YIELD STRENGTH OF 50 KSI, UNLESS NOTED OTHERWISE
 - ALL TRAKS ACCESSORIES SHALL HAVE A MINIMUM YIELD STRENGTH OF 33 KSI, UNLESS NOTED OTHERWISE.
 - COATING: 160/2.0Z GALVANIZED COATING (ASTM A653)
- 5.13. NO MATCHING OR COPING OF STUDS IS ALLOWED, UNLESS DETAILED WITH THIS DRAWING PACKAGE.
- 5.14. SPLICING OF WALL STUDS AND HEADERS IS NOT ALLOWED.
- 5.15. ALL WELDS SHALL CONFORM TO THE AMERICAN WELDING SOCIETY (AWS/AWS D1.3) STANDARDS AND MUST BE PERFORMED BY AN ANSI/AWS CERTIFIED WELDER.
- 5.16. ALL WELD SIZES ARE TO BE CONSIDERED AS EFFECTIVE WELD SIZES AND MUST BE INCREASED TO ACCOUNT FOR ANY GAPS OR SKEWS BETWEEN MEMBERS AS REQUIRED BY ANSI/AWS D1.3.
- 5.17. ALL SCREWS SHALL HAVE A MINIMUM 3/4" CLEARANCE FROM EDGE OF STEEL. SCREWS SHALL HAVE A MINIMUM OF 3/4" ON CENTER SPACING.
- 5.18. ALL FASTENERS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.
- 5.19. POWDER - ACTUATED FASTENERS (PAF): FASTENER SYSTEM OF TYPE SUITABLE FOR APPLICATION INDICATED, FABRICATED FROM CORROSION RESISTANT MATERIAL, WITH CAPABILITY TO SUSTAIN, WITHOUT FAILURE, A LOAD EQUAL TO 10 TIMES DESIGN LOAD, AS DETERMINED BY TESTING PER ASTM E1190 CONDUCTED BY A QUALIFIED, INDEPENDENT TESTING AGENCY.
- 5.20. FOR SCREWS, A MINIMUM OF 1.5 x SCREW DIAMETER CLEARANCE MUST BE MAINTAINED FROM ALL EDGES OF STEEL MEMBERS. A MINIMUM OF 3.0 x SCREW DIAMETER ON-CENTER SPACING MUST BE MAINTAINED BETWEEN ADJACENT SCREWS.
- 5.21. SUGGESTED WELD METAL AND PROCESS FOR SHOP WELDING ARE 60 ksi WELD METAL STRENGTH (MIN., U.N.O.). SUGGESTED METHODS FOR FIELD WELDING ARE 1/8" (U.N.O.) E60xx (MIN., U.N.O.) ELECTRODE-SMAW, OR "GLASSLESS" MIG. MINIMUM WELD THROAT THICKNESS (T) MUST MATCH OR EXCEED THE BASE STEEL THICKNESS OF THE THINNEST CONNECTED PART UNLESS NOTED OTHERWISE.
- 5.22. IN WELDING, THE ZINC COATING ON STEEL FRAMING WILL BE BURNED AWAY; THEREFORE, A ZINC RICH PAINT MUST BE APPLIED TO THE WELD AREA TO PROVIDE CORROSION RESISTANCE.
- 5.23. ALL CONCRETE SCREW FASTENERS SHALL BE THOSE AS MANUFACTURED AND TESTED BY HILTI OR EQUAL. THE FOLLOWING EDGE DISTANCE AND FASTENERS SPACINGS APPLY (UNLESS SPECIFICALLY NOTE OTHERWISE ON DRAWINGS):
- 5.24. GALVANIZED REPAIRS: PREPARE AND REPAIR DAMAGED GALVANIZED WITH ZINC RICH PAINT, ACCORDING TO ASTM A780 AND MANUFACTURER'S WRITTEN INSTRUCTIONS.
- 5.25. PROVIDE FINAL PROTECTION AND MAINTAIN CONDITIONS, IN A MANNER ACCEPTABLE TO MANUFACTURER AND INSTALLER, THAT ENSURES THAT COLD FORMED METAL FRAMING IS WITHOUT DAMAGE OR DETERIORATION AT TIME OF SUBSTANTIAL COMPLETION.
- 6. WOOD CONSTRUCTION**
- 6.1. WOOD FRAMING MEMBERS: VISUALLY GRADED DIMENSIONED #2 SOUTHERN PINE.
- 6.2. ALL PRESSURE TREATED LUMBER SHALL BE PRESSURE TREATED WITH ALKALINE COPPER QUATERNARY (ACQ) IN ACCORDANCE WITH AMERICAN WOOD PROTECTION ASSOCIATION (AWPA) STANDARD.
- ACQ RETENTION:
 - 0.40 LBS/FT3 GROUND CONTACT
 - 0.25 LBS/FT3 ABOVE GROUND
 - ALL FASTENERS, NAILS AND OTHER METAL PRODUCTS USED WITH LUMBER PRESSURE TREATED WITH ACQ SHALL BE HOT-DIP GALVANIZED, STAINLESS STEEL OR AS RECOMMENDED BY THE ACQ MANUFACTURER. PRESSURE TREATED LUMBER SHALL NOT BE IN DIRECT CONTACT WITH ALUMINUM PRODUCTS.
- 6.3. ALL MANUFACTURED WOOD FRAMING CONNECTORS TO BE BY SIMPSON STRONG-TIE COMPANY, INC. OR APPROVED EQUAL. ALL CONNECTORS SHALL BE FASTENED TO FRAMING MEMBERS FILLING THE REQUIRED NUMBER OF CONNECTOR HOLES WITH THE TYPE AND SIZE FASTENERS SPECIFIED BY THE MANUFACTURER.
- 6.4. ROOF SHEATHING: 19/32" PLYWOOD, APA STRUCTURAL I RATED SHEATHING, EXPOSURE 1. PANEL IDENTIFICATION INDEX 40/20. LONG DIMENSION OF PANEL PERPENDICULAR TO SUPPORTS WITH JOINTS STAGGERED.
- 6.5. ROOF SHEATHING NAILING, UNLESS NOTED: 10D HOT-DIPPED GALVANIZED COMMON NAILS AT 4 INCHES AT DIAPHRAGM BOUNDARIES, 6 INCHES AT ALL FOUR PANEL EDGES AND 12 INCHES AT INTERMEDIATE SUPPORTS. ALL UNSUPPORTED PANEL EDGES SHALL BE FULLY BLOCKED.

Order Plans @ WWW.AIAIDLINE.COM

RELEASES / DESCRIPTION / DATES	
NOT FOR CONSTRUCTION	<input type="checkbox"/>
RELEASED FOR CONSTRUCTION	<input checked="" type="checkbox"/>
DATE	06.23.2020
DRAWN	GVA
CHECKED	WAW
APPROVED	ROW
PROJECT NUMBER	20012.01
SHEET TITLE	GENERAL NOTES
DRAWING NO.	S1.1