STRUCTURAL NOTE

COORDINATION

1. STRUCTURAL DRAWINGS SHALL BE USED WITH AND COORDINATED WITH ARCHITECTURAL DRAWINGS AND OTHER CONTRACT DOCUMENTS.

2. THE PROJECT ARCHITECT SHALL BE RESPONSIBLE FOR REVIEW/COORDINATING ALL DIMENSIONS, ELEVATIONS, AND DETAILS SHOWN ON THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS.

3. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL OF THE CONTRACT DOCUMENTS AND LATEST ADDENDA AND FOR SUBMITTING SUCH DOCUMENTS TO SUBCONTRACTORS AND MATERIAL SUPPLIERS PRIOR TO THE SUBMITTAL OF SHOP DRAWINGS, FABRICATION OF ANY STRUCTURAL MEMBERS, AND ERECTION IN THE FIELD.

4. THE GENERAL CONTRACTOR SHALL COMPARE THE STRUCTURAL DRAWINGS AND OTHER CONTRACT DRAWINGS AND REPORT ANY DISCREPANCY BETWEEN AND WITHIN EACH SET OF DRAWINGS WITH THE PROJECT ARCHITECT AND THE STRUCTURAL ENGINEER PRIOR TO THE FABRICATION AND INSTALLATION OF ANY STRUCTURAL MEMBERS.

5. THE DRAWINGS SHOW GENERAL AND TYPICAL SECTIONS/DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY SHOWN, SIMILAR SECTIONS/DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO THE APPROVAL OF THE ENGINEER.

6. THE STRUCTURAL MEMBERS OF THIS PROJECT HAVE BEEN DESIGNED BY THE STRUCTURAL ENGINEER TO RESIST THE REQUIRED CODE GRAVITY AND LATERAL FORCES THAT COULD OCCUR IN THE FINAL COMPLETED STRUCTURE ONLY. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL REQUIRED BRACING DURING CONSTRUCTION AND TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PROCESS UNTIL THE STRUCTURE IS TIED TOGETHER AND COMPLETED.

7. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE METHODS, TECHNIQUES AND SEQUENCES OF PROCEDURES TO PERFORM THE WORK. THE SUPERVISION OF THE WORK IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

8. LOADS APPLIED TO THE STRUCTURE DURING CONSTRUCTION SHALL NOT EXCEED THE SAFE LOAD–CARRYING CAPACITY OF THE STRUCTURAL MEMBERS. THE LIVE LOADS USED FOR THE DESIGN OF THE STRUCTURE ARE INDICATED IN THE GENERAL NOTES. DO NOT APPLY ANY CONSTRUCTION LOADS UNTIL STRUCTURAL FRAMING IS PROPERLY INSTALLED AND ALL TEMPORARY BRACING IS IN PLACE.

9. ALL ASTM AND OTHER REFERENCES ARE PER THE LATEST EDITIONS UNLESS NOTED OTHERWISE.

10. EQUIPMENT PADS SHALL BE PROVIDED BY THE MECHANICAL, ELECTRICAL, OR PLUMBING CONTRACTORS REQUIRING THE PAD.

11. COORDINATE THE EXACT SIZE AND LOCATION OF ALL SLEEVES AND OPENINGS THROUGH CONCRETE WALLS, CONCRETE SLABS, OR MASONRY WALLS WITH ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS.

12. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. CONTRACTOR SHALL REVIEW, APPROVE, AND SIGN EACH SHEET PRIOR TO SUBMISSION. THE STRUCTURAL ENGINEER'S REVIEW SHALL BE FOR CONFORMANCE WITH THE DESIGN CONCEPT AND GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS. THE ENGINEER'S REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW, CHECK, AND COORDINATE THE SHOP DRAWINGS PRIOR TO SUBMISSION. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF THE SHOP DRAWINGS AS THEY PERTAIN TO SIZES, DETAILS, DIMENSIONS, ETC. CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS.

13. CONTRACTOR SHALL VISIT THE SITE PRIOR TO BID TO ASCERTAIN CONDITIONS WHICH MAY ADVERSELY AFFECT THE WORK OR COST THEREOF.

14. WHERE CONFLICTS OCCUR BETWEEN GENERAL NOTES AND SPECIFICATIONS, THE MOST STRINGENT REQUIREMENT SHALL APPLY.

15. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS DURING THE WORK. THE ENGINEER WILL NOT ADVISE NOR ISSUE DIRECTION AS TO SAFETY PRECAUTIONS AND PROGRAMS.

FOUNDATIONS

1. FOUNDATION DESIGN OF FOOTINGS BASED ON AN ASSUMED SOIL BEARING PRESSURE OF 2000 PSF DUE TO LACK OF GEOTECHNICAL INFORMATION.

2. ALL FOOTINGS AND SLAB ON GRADE SHALL BEAR ON UNDISTURBED RESIDUAL SOIL OR STRUCTURAL COMPACTED FILL AS PER SOIL REPORT RECOMMENDATIONS ALL FOUNDATION EXCAVATIONS SHALL BE EVALUATED BY THE GEOTECHNICAL ENGINEER/TESTING AGENCY PRIOR TO PLACING CONCRETE FOR FOUNDATIONS.

3. COORDINATE FOUNDATION WORK WITH EXISTING UTILITIES. FOUNDATION SHALL BE LOWERED WHERE REQUIRED TO AVOID UTILITIES. NOTIFY PROJECT ARCHITECT AND STRUCTURAL ENGINEER TO PROVIDE REINFORCED CONCRETE PIER FOR COLUMN FOOTING.

4. UNLESS NOTED OTHERWISE, COLUMN CENTERLINES SHALL BE CENTERLINES OF COLUMN FOOTINGS.

CONCRETE

1. CONCRETE SHALL BE PROPORTIONED TO MEET THE REQUIREMENTS OF THE FOLLOWING:

ELEMENT	28–DAY STRENGTH (PSI)	SLUMP RANGE (IN.)	UNIT WEIGHT (PCF)
COLUMN FOOTINGS	3000	3–5	150
WALL FOOTINGS	3000	3–5	150
SLAB ON GRADE	3000	3–4	150
EXTERIOR CONCRETE	4500	3–5	150

2. PORTLAND CEMENT SHALL BE ASTM C 150, TYPE I. FLY ASH SHALL BE ASTM C 618, CLASS F AND SHALL NOT EXCEED 25% OF CEMENT BY WEIGHT. NORMAL WEIGHT AGGREGATE SHALL BE ASTM C 33.

3. CONCRETE AGGREGATE GRADATION SHALL BE IN ACCORDANCE WITH ASTM C33 SPECIFICATION, "SPECIFICATION FOR CONCRETE AGGREGATE". FINE AGGREGATE SHALL CONSIST OF NATURAL SAND OR A COMBINATION THEREOF, WITH A FINENESS MODULUS BETWEEN 2.3 AND 3.1. COURSE AGGREGATE CONTENT IS TO BE BETWEEN 35% AND 45% BY WEIGHT OR VOLUME OF THE TOTAL AGGREGATE CONTENT. LARGER COURSE AGGREGATE MIXES UP TO #67 ARE ACCEPTABLE FOR FLOOR SLAB CONCRETE TO MINIMIZE SHRINKAGE CRACKING.

4. FLY ASH SHALL NOT BE PERMITTED IN CONCRETE PLACED SUBJECT TO COLD WEATHER PLACEMENT PROCEDURES.

5. CONCRETE EXCEEDING THE SPECIFIED SLUMP RANGES SHALL BE RETURNED. DO NAT ADD WATER TO THE CONCRETE MIX AT THE JOB SITE WITHOUT THE WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER.

6. ALL REINFORCING STEEL SHALL BE ASTM A615 GRADE 60 UNLESS NOTED OTHERWISE. ALL WELDED WIRE FABRIC (W.W.F.) SHALL BE ASTM A82 AND A185 COLD DRAWN STEEL WIRE. W.W.F. SHALL BE DELIVERED TO THE JOB SITE IN FLAT SHEETS (NO ROLLS). PLACE SHEETS ON BOLSTERS AT 48" MAXIMUM TO LOCATE IN UPPER THIRD OF SLAB.

CONCRETE (CONT'D)

7. LAP CONTINUOUS REINFORCING BARS 36 BAR DIAMETERS UNLESS NOTED OTHERWISE. PROVIDE CORNER BARS IN ALL WALLS AND FOOTINGS.

8. BAR SUPPORTS, DESIGN, DETAIL, FABRICATION, AND PLACING OF REINFORCING BARS SHALL BE IN ACCORDANCE WITH THE ACI CODE AND DETAILING MANUAL AND CRSI'S "MANUAL OF STANDARD PRACTICE".

9. MINIMUM CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3"
CONCRETE EXPOSED TO EARTH OR WEATHER:	
No. 6 THROUGH No. 18 BARS	2"
No. 5 AND SMALLER	1 1/2"
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLABS, WALLS, AND JOISTS:	
No. 14 AND No. 18 BARS	1 1/2"
No. 11 AND SMALLER	3/4"

10. CONCRETE DESIGN AND REINFORCEMENT SHALL BE IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318–02) AND WITH "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT": (ACI 315–02). CONCRETE PLACED DURING HOT WEATHER AND COLD WEATHER SHALL MEET THE RECOMMENDATIONS OF ACI/PCA/TCA.

11. CONCRETE MIXES SHALL BE DESIGNED IN ACCORDANCE WITH ACI 301. WATER SHALL NOT BE ADDED TO THE CONCRETE MIX AT THE JOB SITE WITHOUT THE PRIOR WRITTEN PERMISSION OF THE STRUCTURAL ENGINEER.

12. UNLESS OTHERWISE SHOWN ON ARCHITECTURAL DRAWINGS, PROVIDE 3/4" CHAMFER AT ALL COLUMN, WALL SLAB, AND BEAM EDGES THAT ARE EXPOSED TO VIEW IN THE FINAL STRUCTURE.

SLAB ON GRADE

1. CONTROL JOINTS FOR SLAB ON GRADE SHALL BE LOCATED AS SHOWN ON PLAN, WITH A MAXIMUM SPACING OF 2 1/2 TIMES THE SLAB THICKNESS IN FEET. JOINTS SHALL BE FORMED USING SAW CUTS 1/8" WIDE (MAXIMUM) BY T/4 (1 1./4" MIN.) DEEP. SAW CUT AS SOON AS PRACTICAL AND WITHIN 12 HOURS AFTER PLACING CONCRETE. JOINTS SHALL BE FILLED WITH SEMI–RIGID EPOXY JOINT FILLER (CONSPEC POLUREA JOINTFILL OR EQUIVALENT).

2. SIDEWALKS AND OTHER EXTERIOR SLABS ARE NOT SHOW ON THE STRUCTURAL DRAWINGS. SEE ARCHITECTURAL, SITE, AND CIVIL DRAWINGS FOR LOCATIONS, DIMENSIONS, AND ELEVATIONS.

3. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF DEPRESSED SLAB AREAS AND DRAINS. SLOPE SLAB TO DRAINS WHERE INDICATED.

4. ALL INTERIOR AND EXTERIOR FLOOR SLABS ARE TO RECEIVE ONE (1) COAT OF EVAPORATION REDUCER (CONSPEC AQUAFILM OR EQUIVALENT) APPLIED TO FRESHLY PLACED CONCRETE IMMEDIATELY AFTER SCREEDING AND/OR AFTER THE FIRST FLOATING OPERATION. EVAPORATION REDUCER IS NOT RECOMMENDED FOR USE DURING COLD WEATHER PLACEMENT.

5. FLOOR SLAB AREAS ARE TO RECEIVE TWO (2) COTS OF 25% MIN. SOLIDS ACRYLIC HARDENER AND SEAL (CONSPEC INTRASEAL OR EQUIVALENT). APPLICATION IS TO CONFORM TO MANUFACTURER'S SPECIFICATIONS. FIRST COAT IS FOR CURING, SECOND COAT IS FOR SEALING AND DUST PROOFING AFTER BUILDING CONSTRUCTION COMPLETION.

6. FLOOR SLAB MAY RECEIVE DENSIFIER APPLICATION (NOX–CRETE DURONEX, CONSPEC INTRASEAL, INTRASEAL, OR ASHFORD FORMULA, OR EQUIVALENT) IN PLACE OF ACRYLIC FLOOR SEALER. DENSIFIERS DO NOT CONFORM WITH ASTM C309 AND MAY REQUIRE A CURING COMPOUND PRIOR TO APPLICATIONS OF DENSIFIER. CURING COMPOUND REQUIREMENT IS TO BE BASED ON CLIMATE CONDITIONS DURING TIME OF CONCRETE PLACEMENT. CONTRACTOR TO CONTACT ENGINEER FOR RECOMMENDATIONS.

7. DUE TO THE LACK OF SPECIFIC GEOTECHNICAL INFORMATION AND/OR SPECIFIC FLOOR LOADING REQUIREMENTS, THIS SLAB ON GRADE HAS BEEN DESIGNED USING A SUBGRADE MODULES – K = 120 PSI AND DESIGN LOADING OF 100 PSF.

8. SEE GEOTECHNICAL REPORT/GEOTECHNICAL ENGINEER FOR 10 MIL VAPOR RETARDER AND UNDERSLAB DRAINAGE FILL REQUIREMENTS.

MASONRY

1. MASONRY CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE "SPECIFICATION FOR MASONRY STRUCTURES (ACI 530.1–02)".

2. ALL HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C–90, LIGHTWEIGHT. MINIMUM NET COMPRESSIVE STRENGTH (f'm) SHALL BE 1,500 PSI. FILL BLOCK CELLS WHERE REBAR OCCURS SOLID WITH 3,000 PSI GROUT. SUBMIT VERIFICATION OF ALL MATERIALS TO ARCHITECT FOR APPROVAL.

3. ALL BRICK MASONRY SHALL CONFORM TO ASTM C–216 GRADE SW, TYPE FBS. MINIMUM NET COMPRESSIVE STRENGTH (f 'm) SHALL BE 3,000 PSI. SUBMIT VERIFICATION OF ALL MATERIALS TO ARCHITECT FOR APPROVAL.

4. MORTAR SHALL BE PORTLAND CEMENT–LIME MIX (PORTLAND CEMENT SHALL COMPLY WITH ASTM C 150, TYPE I OR III, AND HYDRATED LIME COMPLYING WITH ASTM C 207) OR MORTAR CEMENT ASTM C 1329 – TYPE S, THE USE OF MASONRY–CEMENT IS STRICTLY FORBIDDEN.

5. GROUT FOR UNIT MASONRY SHALL COMPLY WITH ASTM C 476 (SLUMP 8 TO 11 INCHES). AGGREGATE FOR MORTAR SHALL COMPLY WITH ASTM C 144 AND AGGREGATE FOR GROUT SHALL COMPLY WITH ASTM C404. COMPRESSIVE STRENGTH SHALL BE GREATER THAN OR EQUAL TO 2,000 PSI OR f'm, WHICHEVER IS GREATER. TESTING SHALL BE DONE IN AN ABSORBENT MOLD IN ACCORDANCE WITH ASTM C 1019.

6. MASONRY JOINT REINFORCEMENT SHALL COMPLY WITH ASTM A–951 AND SHALL BE HOT DIPPED GALVANIZED, CARBON STEEL. BRICK TIES SHALL CONFORM TO SEISMIC DESIGN CATEGORY D AND E SHALL HAVE A CONTINUOUS 3/16" DIAMETER GALVANIZED WIRE ROD EMBEDDED INTO THE BRICK MORTAR JOINT (HECKMAN SEISMIC POS–I–TITE OR EQUIVALENT). BRICK TIES USED IN SEISMIC CATEGORY F SHALL HAVE A CONTINUOUS 1/4" DIAMETER GALVANIZED WIRE ROD EMBEDDED IN THE MORTAR JOINT. PROVIDE IN LENGTHS NOT LESS THAN 10 FEET IN LENGTH WITH PREFABRICATED CORNER AND TEE UNITS. FOR MULTIWYTHE MASONRY, PROVIDE ADJUSTABLE 2–PIECE UNITS. PROVIDE CONTINUITY AT CORNERS AND WALL INTERSECTIONS BY USING PREFABRICATED "L" AND "T" SECTIONS. LAP REINFORCEMENT A MINIMUM OF 6". SPACE REINFORCEMENT NOT MORE THAN 16" O.C. PROVIDE REINFORCEMENT NOT MORE THAN 8" ABOVE OR BELOW WALL OPENINGS AND EXTENDING 24" BEYOND OPENINGS. CUT REINFORCEMENT AT CONTROL AND EXPANSION JOINTS UNLESS NOTED OTHERWISE.

7. THE MASONRY CONTRACTOR SHALL PROVIDE ALL TEMPORARY WALL BRACING DURING CONSTRUCTION.

8. THE MINIMUM QUALITY ASSURANCE PROGRAM FOR NON–ESSENTIAL FACILITIES SHALL COMPLY WITH TABLE 1.14.2.OF ACI 530–02.

STRUCTURAL STEEL

1. ALL W–SHAPE STRUCTURAL STEEL SHALL BE ASTM A992. ALL OTHER STRUCTURAL SHAPES SHALL BE ASTM A–36. SQUARE OR RECTANGULAR HSS SHAPES SHALL CONFORM TO ASTM A–500 GRADE B, ROUND HSS SHAPES SHALL CONFORM TO ASTM A–500, GRADE B, STRUCTURAL STEEL PIPE PIPE COLUMNS SHALL CONFORM TO ASTM A–501 OR ASTM A–53, TYPE E OR S, GRADE B. DESIGN DETAILING, FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH THE AISC CODE AND DETAILING MANUAL. NO STRUCTURAL MEMBERS SHALL BE SPLICED EXCEPT AS SHOWN ON APPROVED SHOP DRAWINGS.

COLD FORMED METAL FRAMING

1. COLD FORMED STEEL FRAMING SHALL BE AS PER THE LATEST EDITION OF AISI.

2. ALL METAL STUDS SHALL BE CLARK (OR EQUIVALENT). ALL METAL STUDS SHALL HAVE A MINIMUM Fy = 50 KSI YIELD STRENGTH FOR 16 GAUGE AND HEAVIER AND Fy = 33 KSI FOR 18 GAUGE AND LIGHTER, AND ALL STUD RUNNERS SHALL HAVE A MINIMUM Fy = 33 KSI YIELD STRENGTH. ALL CLIP ANGLES AND ACCESSORIES SHALL HAVE A MINIMUM Fy = 40 KSI YIELD STRENGTH. SCREW FASTENERS SHALL BE #12–14 SCREWS UNLESS NOTED OTHERWISE. POWDER ACTUATED FASTENERS SHALL BE HILTI DX FASTENERS (OR EQUIVALENT). SUBMIT SHOP DRAWINGS, SHOWING LAYOUT OF STUDS AND STRUCTURAL FRAMING, INCLUDING ARRANGEMENT, DIMENSIONS, MATERIALS, STRESS VALUES, CONNECTORS, ANCHORAGE, AND RELATION TO ADJACENT WORK TO ARCHITECT FOR APPROVAL.

3. ALL STUDS SHALL BE LATERALLY BRACED WITH MECHANICAL BRACING INSTALLED AT 48" O.C. MAXIMUM.

4. CONTINUOUS CT RUNNERS SHALL BE OF THE SAME GAUGE (OR HEAVIER) AS THAT OF THE FRAMING MEMBER BEING CONNECTED UNLESS NOTED OTHERWISE.

TIMBER

1. A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF THE PROJECT SHALL DESIGN WOOD TRUSSES. DESIGNS SHALL BE SEALED AND SIGNED BY HIM/HER AND SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL. FABRICATION SHALL BE BY A MEMBER OF THE TRUSS PLATE INSTITUTE, INC. SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF TRUSSES AND STRUCTURAL FRAMING INCLUDING ARRANGEMENT, DIMENSIONS, MATERIALS, GRADES, STRESS VALUES, CONNECTORS, ANCHORAGE, AND RELATION TO ADJACENT WORK TO ARCHITECT FOR APPROVAL. TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING MINIMUM LOADS:

TOP CHORD DEAD LOAD	10.0 PSF
BOTTOM CHORD DEAD LOAD	10.0 PSF
TOP CHORD LIVE LOAD	20.0 PSF

2. ALL TEMPORARY TRUSS BRACING REQUIRED FOR ERECTION, AS PER THE GUIDELINES SET FORTH BY THE TRUSS PLATE INSTITUTE PUBLICATION "HIB–91", SHALL BE PERMANENTLY ATTACHED AND REMAIN IN PLACE TO SERVE AS PERMANENT TRUSS BRACING UNLESS NOTED OTHERWISE.

3. TRUSS MEMBERS AND COMPONENTS SHALL NOT BE CUT, NOTCHED, DRILLED, OR ALTERED IN ANY MANNER WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER RESPONSIBLE FOR DESIGNING THE TRUSSES.

4. TRUSS CONFIGURATIONS SHOWN ARE SCHEMATIC ONLY, AND MAY BE ALTERED AS REQUIRED. COORDINATE TRUSS CONFIGURATIONS WITH ARCHITECTURAL REQUIREMENTS AND OTHER TRADES.

5. WOOD TRUSSES SHALL BE ERECTED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY AND PERMANENT BRACING AS REQUIRED FOR THE SAFE ERECTION AND PERFORMANCE OF THE TRUSSES. PLYWOOD/OSB ROOF SHEATHING SHALL RUN CONTINUOUS UNDER ALL VALLEY/OVERBUILD TRUSSES, TYPICAL.

6. ALL TIMBER CONNECTORS, ANCHORS, FASTENERS, TIES, STRAPS, BASES, CAPS, ETC. SHALL BE SIMPSON "STRONG–TIE" (OR EQUIVALENT). CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S REQUIREMENTS. ALL CONNECTORS IN CONTACT WITH PRESSURE TREATED LUMBER SHALL MEET THE REQUIREMENTS OF ASTM A653 (CLASS G185) OR ASTM A153.

7. ALL FRAMED LUMBER SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE STRESS STRESSES – UNLESS NOTED OTHERWISE (SURFACED AT 19% MOISTURE CONTENT):

ELEMENT	FENDING Fb – PSI	SHEAR Fv – PSI	COMPRESSION Fc – PSI	MODULUS OF ELASTICITY E – PSI
2"–4" THICK 2"–4" WIDE	1,500	90	1,650	1,600,00
2"–4" THICK 5"–6" WIDE	1,250	90	1,600	1,600,00
2"–4" THICK 8" WIDE	1,200	90	1,550	1,600,00
2"–4" THICK 10" WIDE	1,050	90	1,500	1,600,00
2"–4" THICK 12" WIDE	975	90	1,450	1,600,00

8. ALL LOAD BEARING TIMBER WALL STUDS SHALL BE SOUTHERN PINE (SURFACE AT 19% MOISTURE CONTENT) STUD GRADE, TYPICAL UNLESS NOTED OTHERWISE.

9. ALL TIMBER IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED.

10. ALL TIMBER CONNECTIONS SHALL NOT BE LESS THAN THOSE SPECIFIED IN TABLE 2304.9.1 OF THE INTERNATIONAL BUILDING CODE UNLESS NOTED OTHERWISE.

11. GENERAL CONTRACTOR SHALL COORDINATE LOCATION OF TRUSSES WITH OTHER TRADES – SHIFT TRUSSES A MAXIMUM OF 3 1/2" AS REQUIRED.

12. ALL NON TONGUE AND GROOVE PLYWOOD/OSB PANELS SHALL HAVE 1/8" GAP AT ALL PANEL EDGES. PROVIDE SIMPSON PSCL (OR EQUIVALENT) PLYWOOD CLIPS AT 24" O.C. AT PANEL EDGES OF ALL ROOF PLYWOOD/OSB SHEATHING.

Bojangles Resturant
J. E. Briscoe Way & Madison Blvd.
Madison, AL 35758

Design Loads per IBC 2018 and NC state building code 2018:
Floor Live = 100 psf
Floor Dead = 50 psf
Roof Top Chord Live = 20 psf
Roof Bottom Chord Live = 0 psf
Roof Top Chord Dead = 10 psf + HVAC load
Roof Bottom Chord Dead = 15 psf

Snow load data:
P_g = 10 psf
C_e = .9
I_s = 1.0
C_t = 1.0

Wind design data:
V_{ult} = 115 psf
V_{asd} = 90 psf
Risk category = II
Internal pressure coef. = .18/-.18
Component and cladding loads

	Zone 1	Zone 2	Zone 3	Zone 4	Zone 5
10 sqft	16/-23.8	16/-39.9	16/-60.1	23.8/-25.8	23.8/-31.9
20 sqft	16/-23.2	16/-35.7	16/-49.8	22.7/-24.7	22.7/-29.7
50 sqft	16/-22.4	16/-30.1	16/-36.1	21.3/-23.3	21.3/-26.9
100 sqft	16/-21.8	16/-25.8	16/-25.8	20.2/-22.2	20.2/-24.7
500 sqft				17.7/-19.8	17.7/-19.8

Minimum wind pressure for components and cladding loading = 16 psf

Seismic design data:
Risk category = II
I_e = 1.0
S_s = .26 g
S_1 = .117 g
Site class = D
S_{ds} = .276 g
S_{d1} = .185 g
Design category = C

R-2002

DATE	RELEASE
04.30.20	FOR CONSTRUCTION
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CHECKED BY: RZ
DRAWN BY: W.A.A.C.

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ALABAMA REGISTERED NO. 18566 PROFESSIONAL ENGINEER M. D. QUINN 4.30.2020

J.E. BRISCOE WAY & MADISON BLVD. PROJECT: R–2002
MADISON, MADISON COUNTY ALABAMA DATE: 04/17/20

Bojangles' RESTAURANT

STRUCTURAL NOTES

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