

DESIGN AND CODE INFORMATION:

- ALL CONSTRUCTION SHALL CONFORM TO THE 2012 INTERNATIONAL BUILDING CODE.
- VERIFY EXISTING CONDITIONS AND DIMENSIONS. NOTIFY THE ENGINEER IMMEDIATELY OF ANY CONDITIONS THAT DO NOT COMPLY WITH THE PLANS AND SPECIFICATIONS. STRUCTURAL DRAWINGS MUST BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS.
- THE STRUCTURAL ENGINEER OF RECORD HAS NOT BEEN EMPLOYED TO PERFORM SITE OBSERVATIONS OF THIS PROJECT.
- THESE CONTRACT DRAWINGS SHALL NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS.
- THE STRUCTURE IS NOT STABLE UNTIL ALL LOAD BEARING AND SHEARING WALLS ARE ERECTED AND ROOF TRUSS MEMBERS ARE ERECTED, CONNECTIONS ARE COMPLETELY MADE, THE ROOF DECK ATTACHED TO THE ROOF FRAMING AND THE CONCRETE FLOORS PLACED AND ATTAINING A MINIMUM OF 75% OF THE SPECIFIED 28-DAY STRENGTH. UNTIL SUCH TIME, TEMPORARY BRACING IS REQUIRED. THE DESIGN ADEQUACY OF TEMPORARY BRACING AND SHORING IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- FOR LOCATIONS OF MISCELLANEOUS ITEMS (OPENINGS, BENT PLATES, INSERTS, ETC.) AFFECTING STRUCTURAL WORK, SEE ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.
- IBC SECTION 1603 DESIGN LOAD DATA:

FLOOR LIVE LOAD: 100 PSF
 ROOF LIVE LOAD: 20 PSF
 ROOF DEAD LOAD: 18 PSF

GROUND SNOW LOAD: 20 PSF
 FLAT-ROOF SNOW LOAD: $P_f = 12.8$ PSF
 SNOW EXPOSURE FACTOR: $C_e = 0.9$
 SNOW IMPORTANCE FACTOR: $I_s = 1.00$
 THERMAL FACTOR: $C_t = 1.0$

WIND SPEED: 115 MPH
 RISK CATEGORY: II
 WIND EXPOSURE: B
 INTERNAL PRESSURE COEFFICIENT: ± 0.18
 COMPONENT & CLADDING PRESSURE:
 ROOF: +18.0 PSF / -60.1 PSF
 WALL: +23.6 PSF / -31.9 PSF

SEISMIC DESIGN DATA:
 RISK CATEGORY: II
 IMPORTANCE FACTOR: $I_a = 1.00$
 $S_a = 0.111$ $S_1 = 0.082$
 SITE CLASS: D
 $S_{m1} = 0.119$ $S_{m2} = 0.10$
 SEISMIC DESIGN CATEGORY: B
 SEISMIC FORCE-RESISTING SYSTEM:
 #2 - LIGHT-FRAME (WOOD) WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE
 BASE SHEAR: $8.815 \times S_a \times K$
 SEISMIC RESPONSE COEFFICIENT: $C_s = 0.018$
 RESPONSE MODIFICATION COEFFICIENT: $R = 8.5$
 ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE (ASCE 7-10 SECTION 12.8)

LUMBER FRAMING:

- ALL WOOD CONSTRUCTION SHALL CONFORM TO THE AMERICAN FOREST AND PAPER ASSOCIATION (AFPA) NDS-91.
- STUDS IN LOAD BEARING WALLS MAY BE DOUGLAS FIR, SOUTHERN YELLOW PINE, OR SPRUCE (#2 OR CONSTRUCTION GRADE) UNLESS OTHERWISE NOTED.
- ALL PLYWOOD SHEATHING SHALL BE APA RATED PANELS. SEE PLAN.
- UNLESS SPECIFICALLY NOTED OTHERWISE, ALL NAILS ARE TO BE COMMON NAILS.

CONCRETE MASONRY:

- MASONRY CONSTRUCTION SHALL CONFORM TO THE CURRENT EDITION OF THE "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530).
- REINFORCED CONCRETE MASONRY SHALL BE INSPECTED BY A TESTING AGENCY IN ACCORDANCE WITH ACI 530 TABLE 1.14.2 - LEVEL 2 QUALITY ASSURANCE.
- CONCRETE MASONRY SHALL HAVE A MINIMUM DENSITY OF 125 PCF AND SHALL HAVE A MINIMUM PRISM STRENGTH f'_m OF 1,500 PSI.
- GROUT FOR FILING CONCRETE MASONRY CELLS SHALL CONFORM TO STANDARD SPECIFICATIONS FOR "MORTAR AND GROUT FOR REINFORCED MASONRY", ASTM C-478 AND SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE PRISM STRENGTH (f'_m) OF 1,800 PSI. THE SLUMP SHALL BE BETWEEN 9 INCHES AND 11 INCHES. WHERE THE MINIMUM CLEAR DIMENSION OF ANY CONTINUOUS VERTICAL CELL IS 3 INCHES OR LESS, USE FINE GROUT; OTHERWISE USE COARSE (PEA GRAVEL) GROUT.
- MORTAR FOR CONCRETE MASONRY SHALL BE TYPE "S" CONFORMING TO THE REQUIREMENTS OF ASTM C-270.

- MASONRY CONSTRUCTION SHALL BE BUILT IN LIFTS NOT TO EXCEED 4 FEET PRIOR TO GROUTING BLOCK CELLS. KEY THE NEXT GROUT LIFT INTO THE PRIOR LIFT BY STOPPING THE FIRST LIFT 2 INCHES BELOW THE TOP OF THE BLOCK.
- ALL REINFORCING BARS IN FILLED CELLS SHALL BE DOVELED INTO THE FOOTINGS WITH STANDARD 90° HOOKS AND SHALL EXTEND TO WITHIN 2 INCHES OF THE TOP OF THE FOUNDATION WALLS.
- UNLESS NOTED OTHERWISE, REINFORCEMENT IN WALLS SHALL BE PLACED IN THE CENTER OF THE WALL.
- WHETHER OR NOT SHOWN ON THE DRAWINGS, ALL MASONRY WALLS SHALL BE REINFORCED WITH (1) #5 BAR VERTICAL AT EACH END, INTERSECTION AND AT EACH JAMB AND AT A MAXIMUM OF 48 INCHES ON CENTER ALONG THE LENGTH OF THE WALL. ALL VERTICAL REINFORCED CELLS SHALL BE FILLED WITH 1,800 PSI (MINIMUM) GROUT.
- PROVIDE DUR-O-WAL D/A 310 TRUSS REINFORCING AT EVERY COURSE AT OR BELOW 0.00' AND AT 16 INCHES ON CENTER ABOVE 0.00'.
- ALL WALLS 8'-0" AND HIGHER SHALL RECEIVE A HORIZONTAL BOND BEAM WITH (1) #5 BAR CONTINUOUS AT THE TOP OF THE WALL.

FOUNDATION NOTES:

- FOUNDATION DESIGN IS BASED ON THE FOLLOWING ASSUMPTIONS. A GEOTECHNICAL ENGINEER SHALL BE EMPLOYED PRIOR TO THE START OF CONSTRUCTION TO INVESTIGATE SUBSURFACE CONDITIONS.
- FOOTINGS ARE DESIGNED TO BEAR ON UNIFORM SOIL WITH A MINIMUM BEARING CAPACITY OF 2,000 PSF. THE DESIGN ASSUMES THAT DIFFERENTIAL AND TOTAL SETTLEMENT ARE WITHIN ACCEPTED TOLERANCES FOR THE TYPE OF CONSTRUCTION USED.
- THE SOIL BEARING CAPACITY AND CONSISTENCY FOR THE BUILDING PAD AREA SHALL BE VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER WHEN FOUNDATION EXCAVATIONS HAVE BEEN CARRIED DOWN TO THE PROPOSED FOOTING BEARING ELEVATIONS. SEE PLAN FOR MINIMUM FOOTING BEARING ELEVATIONS.
- WHERE FOOTING EXCAVATIONS ARE TO REMAIN OPEN AND MAY BE EXPOSED TO RAINFALL, THE EXCAVATIONS SHALL BE UNDERCUT AND A 3-INCH THICK MUD MAT OF 2,000 PSI CONCRETE SHALL BE PLACED TO PROTECT THE BEARING SOILS.
- WHERE FOOTING STEPS ARE REQUIRED, THEY SHALL BE NO STEEPER THAN 1 VERTICAL TO 2 HORIZONTAL UNLESS SHOWN OTHERWISE ON THE PLANS.
- ALL ITEMS TO BE EMBEDDED IN CONCRETE (ANCHOR BOLTS, REINFORCING, ETC.) SHALL BE IN PLACE PRIOR TO POURING OF CONCRETE. PROVIDE SUFFICIENT BRACING AS REQUIRED TO ENSURE THESE ITEMS REMAIN IN THE PROPER LOCATION AND ALIGNMENT.
- THE CONTRACTOR SHALL COORDINATE THE STRUCTURAL DRAWINGS WITH REQUIREMENTS SET FORTH ON THE GRADING PLAN AND IN THE GEOTECHNICAL INVESTIGATION. THE CONTRACTOR SHALL COORDINATE BETWEEN THE STRUCTURAL DRAWINGS AND THE SHOP DRAWINGS PROVIDED BY THE STEEL FABRICATOR. THE CONTRACTOR SHALL CONTACT THE ENGINEER IMMEDIATELY FOR CLARIFICATION OF ANY DISCREPANCIES PRIOR TO BEGINNING WORK.
- BACKFILLING AGAINST FOUNDATION WALLS WILL NOT BE PERMITTED UNTIL THE INTERIOR FLOOR SLAB IS IN PLACE, OR ADEQUATE TEMPORARY BRACING IS PROVIDED.

REINFORCED CONCRETE:

- ALL CONCRETE WORK SHALL CONFORM TO THE CURRENT EDITION OF "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318).
- REINFORCING SHALL BE GRADE 60 DEFORMED BARS PER ASTM A-615. WELDED WIRE FABRIC SHALL BE SUPPLIED IN FLAT SHEETS ONLY AND SHALL MEET THE REQUIREMENTS SET FORTH IN ASTM A-185.
- UNLESS NOTED OTHERWISE, THE MINIMUM 28-DAY COMPRESSIVE STRENGTH FOR ALL CAST IN PLACE CONCRETE SHALL BE AS FOLLOWS:
 EXTERIOR SLABS ON GRADE - 3,500 PSI
 ALL OTHER CONCRETE - 3,000 PSI
- ALL REINFORCING BAR LAP SPICES SHALL BE CLASS "B" SPICES PER THE CURRENT EDITION OF ACI 318 UNLESS NOTED OTHERWISE.
- MINIMUM CLEAR CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS FOLLOWS:
 MASONRY WALLS - LOCATE IN CENTER OF WALL
 SLAB ON GRADE - 2"
 PIERS (FORMED EDGES) - 1 1/2"
 FOOTINGS (FORMED EDGES) - 2"
 FOOTINGS (CAST AGAINST EARTH) - 3"
- LONGITUDINAL REINFORCING STEEL IN FOOTINGS AND BOND BEAMS SHALL BE CONTINUOUS AROUND CORNERS. SEE TYPICAL CONTINUITY DETAILS.
- ALL CONCRETE SHALL BE VIBRATED BY MECHANICAL VIBRATORS.
- UNLESS DIRECTED OTHERWISE BY THE OWNER, CONCRETE SLABS SHALL BE FINISHED TO THE FOLLOWING FLATNESS CRITERIA:
 SPECIFIED OVERALL F NUMBERS: FLATNESS FF = 20
 LEVEL FL = 15
 MINIMUM LOCAL F NUMBERS: FLATNESS FF = 15
 LEVEL FL = 10
- ALL ITEMS TO BE EMBEDDED IN CONCRETE (ANCHOR BOLTS, REINFORCING, ETC.) SHALL BE IN PLACE PRIOR TO POURING OF CONCRETE. PROVIDE SUFFICIENT BRACING AS REQUIRED TO ENSURE THESE ITEMS REMAIN IN THE PROPER LOCATION AND ALIGNMENT.

SHEAR WALL NOTES:

SEE DETAIL #3 ON SHEET S-3 FOR SHEAR WALL CONSTRUCTION DETAILS.

SHEAR WALL SHEATHING TO BE APA RATED PANELS (OSB) IN THE THICKNESS TABULATED. PANELS SHALL SPAN A MINIMUM OF (3) STUDS AND SHALL BE ORIENTED VERTICALLY STARTING AT THE TOP OF THE PARAPET. WALL STUDS SHALL BE SPACED AT A MAXIMUM OF 16" O.C.

SHEAR WALL PANELS TO BE FASTENED ALONG PANEL EDGES USING THE FASTENER SIZE AND SPACING AS TABULATED. PANEL FIELD NAILING TO BE AT 12" O.C. ALL PANEL EDGES TO BE SUPPORTED BY WALL STUDS AND/OR HORIZONTAL 2x BLOCKING.

SHEAR WALL SILL PLATE TO BE P.T. 2x6 ANCHORED WITH 1/2" HEADED ANCHOR BOLTS. SEE DETAILS FOR EMBEDMENT AND SPACING REQUIREMENTS.

SHEAR WALLS TO TERMINATE IN END POST AS TABULATED. PROVIDE SIMPSON STRONG-TIE HOLD DOWN AND PRE-ASSEMBLED ANCHOR BOLT (PAB) AT EACH END OF EACH SHEAR WALL AS TABULATED. DIAMETER OF HOLD-DOWN ANCHOR AND DEPTH OF EMBEDMENT TO BE AS TABULATED.

NOTE:

- TOP OF FOOTING ELEVATION TO BE 20" BELOW 0.00' UNLESS NOTED OTHERWISE.
- FOOTINGS ARE SIZED FOR A MIN. ALLOWABLE BEARING PRESSURE OF 2,000 psf.
- REINFORCE ALL MASONRY WALLS W/ (1) #5 BAR VERT. @ EACH END, INTERSECTION, @ EACH JAMB & @ 48" O.C. ALONG THE LENGTH OF THE WALL. FILL VERTICAL REINFORCED CELLS W/ GROUT. REINFORCING TO DOWEL UP FROM FOOTING AND EXTEND TO TOP OF WALL. PROVIDE DUR-O-WAL D/A 310 EXTRA HEAVY TRUSS REINFORCING @ EVERY COURSE AT OR BELOW 0'-0" AND AT 16" O.C. ABOVE 0'-0". ALL WALLS 8'-0" AND HIGHER TO RECEIVE HORIZ. BOND BEAM W/ (1) #5 BAR CONT. AT THE TOP OF THE WALL. CONCRETE MASONRY UNITS SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 1,500 PSI.
- ALL ITEMS TO BE EMBEDDED IN CONCRETE (ANCHOR BOLTS, REINFORCING, ETC.) SHALL BE IN PLACE PRIOR TO POURING OF CONCRETE. PROVIDE SUFFICIENT BRACING AS REQUIRED TO ENSURE THESE ITEMS REMAIN IN THE PROPER LOCATION AND ALIGNMENT.
- THE CONTRACTOR SHALL COORDINATE THE STRUCTURAL DRAWINGS WITH REQUIREMENTS SET FORTH ON THE GRADING PLAN AND IN THE GEOTECHNICAL INVESTIGATION. THE CONTRACTOR SHALL CONTACT THE ENGINEER FOR CLARIFICATION OF ANY DISCREPANCIES PRIOR TO BEGINNING WORK.
- ALL REINFORCING BARS SHALL BE GRADE 60 STEEL PER ASTM A-615. THE MINIMUM LENGTH OF ANY LAP SPICE SHALL BE:

BAR SIZE	SPICE LENGTH
#3	24 INCHES
#4	36 INCHES
#5	48 INCHES
#6	60 INCHES
- BOLTS AND NUTS FOR WOOD CONNECTIONS SHALL BE GRADE A PER ASTM A307. WASHERS SHALL BE PER ASTM F844.
- WOOD PLATE SHALL BE ANCHORED TO THE FOUNDATION WALL WITH 5/8" ANCHOR BOLTS WITH 1 3/4" O.D. WASHERS AT 48"
- BOLTS, WASHERS, NUTS, NAILS, SCREWS, HANGERS, HOLD-DOWNS, ETC. IN CONTACT WITH PRESERVATIVE TREATED LUMBER SHALL BE GALVANIZED.
- SEE DETAIL S/S-3 FOR FOUNDATION STEP DETAIL AS REQUIRED.

COLD WEATHER CONCRETING:

CONCRETE MUST BE MAINTAINED AT A MINIMUM TEMPERATURE OF 50° F AND KEPT MOST FOR A MINIMUM OF 7 DAYS AFTER PLACEMENT. THE CONTRACTOR SHALL PROVIDE ADEQUATE EQUIPMENT TO ENSURE THAT CONCRETE IS HEATED AND PROTECTED FROM FREEZING UNTIL ADEQUATELY CURED. ALL FORMS, FILLERS AND REINFORCING SHALL BE FREE FROM ICE, SNOW AND FROST. THE GROUND ON WHICH CONCRETE IS TO BE PLACED SHALL BE FREE FROM ICE, SNOW AND FROST.

ALTERNATELY, CONCRETE MUST BE PROTECTED FROM FREEZING FOR A MINIMUM OF 3 DAYS. THE CONCRETE MUST BE AT A MINIMUM TEMPERATURE OF 55° F AND A MAXIMUM TEMPERATURE OF 75° F WHEN PLACED. THE CONCRETE MUST BE PROTECTED FROM FREEZING, MAINTAINED AT A MINIMUM SURFACE TEMPERATURE OF 55° F AND KEPT MOST FOR A MINIMUM OF 3 DAYS. PROTECTION MAY BE REDUCED TO 2 DAYS IF TYPE III PORTLAND CEMENT (ASTM C 150) IS USED IN THE MIX.

SUFFICIENT COMBUSTION HEATERS, TENTING, INSULATION BLANKETS, ETC. SHALL BE PROVIDED TO PROTECT CONCRETE AS NOTED ABOVE. NO MORE CONCRETE SHALL BE PLACED THAN CAN BE PROTECTED WITH THE AVAILABLE EQUIPMENT AND SUPPLIES. HEATERS SHALL BE PLACED AND DIRECTED TO AVOID AREAS OF OVERHEATING OR DRYING OF THE CONCRETE SURFACE. THE CONCRETE SURFACE SHALL NOT BE EXPOSED TO AIR TEMPERATURES GREATER THAN 75°.

AT THE END OF THE PROTECTION PERIOD, THE SURFACE TEMPERATURE OF THE CONCRETE SHALL NOT DECREASE AT A RATE GREATER THAN 50° IN A 24-HOUR PERIOD UNTIL THE TEMPERATURE OF THE CONCRETE SURFACE IS WITHIN 20° OF THE AMBIENT (AIR) TEMPERATURE. AMBIENT AIR, PROTECTION AIR AND CONCRETE TEMPERATURES SHALL BE RECORDED AT A MINIMUM FREQUENCY OF TWICE PER 24-HOUR PERIOD.

FROST PROTECTION	
FROST DEPTH	36 INCHES
TOP OF FOUNDATION BELOW 0.00'	2 INCHES (MIN)

COLUMN/POST SCHEDULE		
MARK	SIZE	COMMENTS
(1) S-3	(2) 2x6	(2) 2x TRIMMER STUD, SEE NOTE 2 FOR REQUIRED KING STUDS
(2) S-1	(3) 2x6	(3) 2x TRIMMER STUD, SEE NOTE 2 FOR REQUIRED KING STUDS

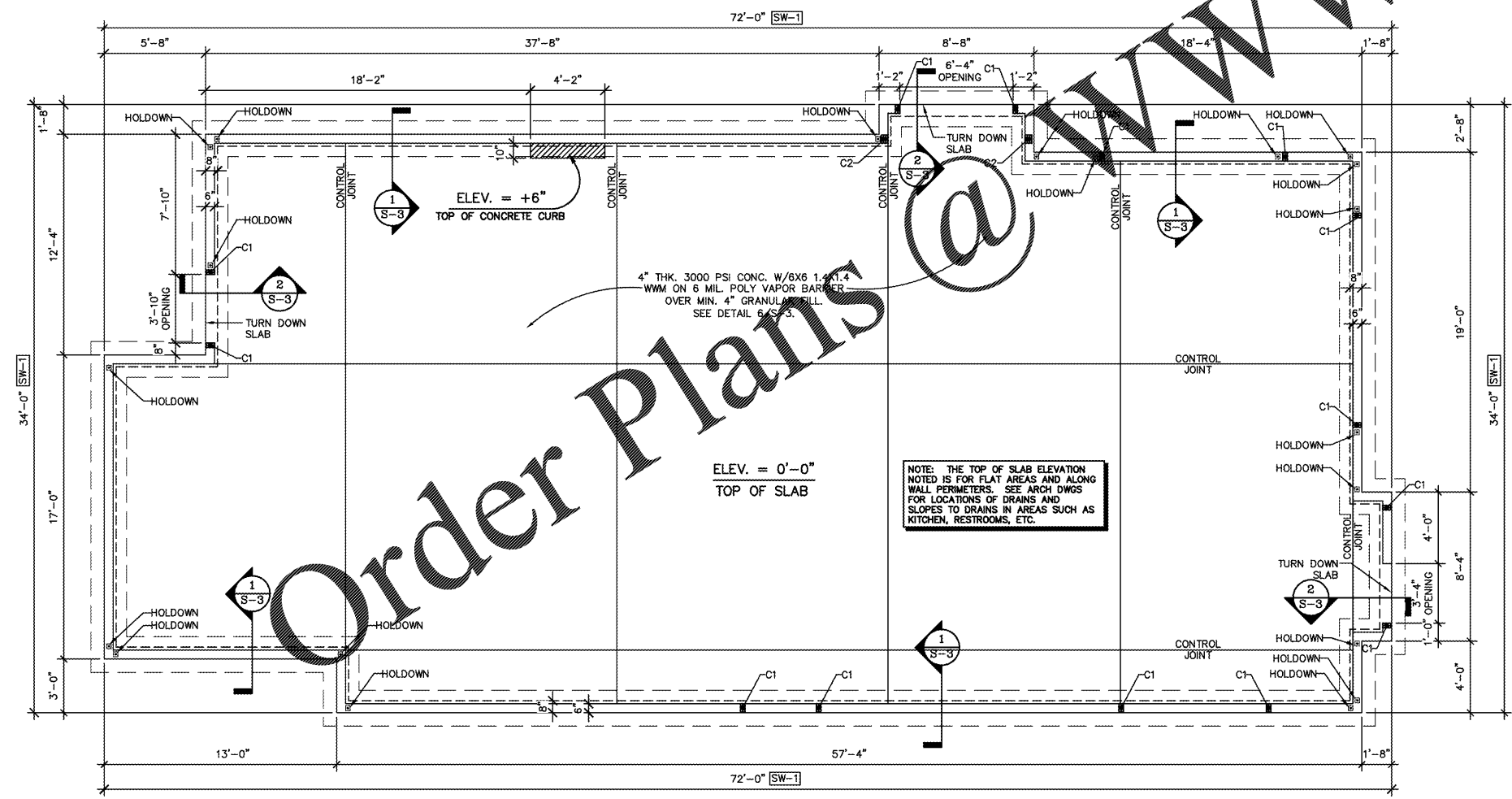
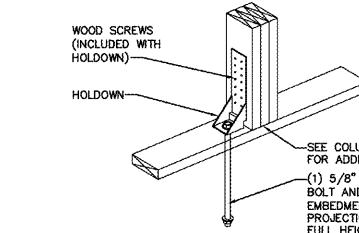
NOTES:

- SEE 3/S-2 FOR TYPICAL HEADER FRAMING
- PROVIDE (1) KING STUD EACH END FOR EVERY 4'-0" OF OPENING WIDTH

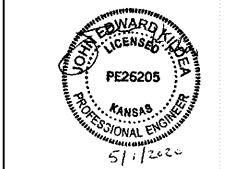
SHEAR WALL SCHEDULE						
MARK	SHEATHING	NAILS	SPACING	END POST	ANCHOR BOLT SPACING	HOLD-DOWNS
SW-1	15/32" STRUCTURAL PLYWOOD/OSB EXTERIOR GRADE	10d	6" 12"	(3) 2x6	5/8" @ 32" O.C. MIN 12" EMBEDMENT	HDU8-SDS2.5 HOLD-DOWN ANCHOR AS SHOWN ON PLAN

NOTES:

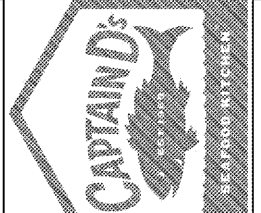
- FRAMING FOR SHEAR WALLS SHALL BE 2X6 STUDS SPACED AT 16" O.C. PROVIDE (3) 2X6 STUDS MINIMUM, BACK TO BACK, AT END OF EACH SHEAR WALL.
- WALLS NOT A PART OF THE SHEAR WALL CONSTRUCTION SHALL BE ANCHORED TO THE FOUNDATION AS INDICATED IN THE NOTES AND SCHEDULES.
- SEE 2/S-1 FOR ANCHOR HOLD-DOWN DETAIL.



FOUNDATION PLAN
 SCALE: 1/4" = 1'-0"



A NEW LOCATION FOR
CAPTAIN D'S
 7525 STATE AVENUE
 KANSAS CITY, KS 66112
CAPTAIN D'S, LLC
 624 GRASSMERE PARK DRIVE,
 SUITE 30
 NASHVILLE, TN 37211



Job Number:
 Revisions:

S-1
 FOUNDATION PLAN