

STRUCTURAL GENERAL NOTES :

STRUCTURAL STEEL:

- STRUCTURAL STEEL SHALL CONFORM TO "THE SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" AND THE "MANUAL OF STEEL CONSTRUCTION" BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC.
- MATERIAL REQUIREMENTS, UNO:
 - WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, CHANNELS, PLATES AND OTHER MISC. SHAPES SHALL CONFORM TO ASTM A36.
 - STRUCTURAL TUBING (HSS) SHALL CONFORM TO ASTM A500, GRADE C, F_y = 50 ksi. STRUCTURAL PIPE SHALL CONFORM TO ASTM A-53 GRADE B, TYPE E OR S, F_y = 35 ksi.
 - BOLTED CONNECTIONS SHALL CONSIST OF A MINIMUM 3/4" DIAMETER ASTM F3125 A325 HIGH STRENGTH BOLTS.
 - WELDING ELECTRODES SHALL CONFORM TO AWS E.1 OR A5.5 E-70XX (LOW-HYDROGEN FOR SMAW WELDING).
 - ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 36 THREADED RODS, UNO.
 - GROUT BELOW BASE PLATES SHALL BE HIGH-STRENGTH, NON-SHRINK, NONSTEELEIC GROUT, WITH A 28 DAY MINIMUM COMPRESSIVE STRENGTH OF 7,000 PSI.
 - HEADED ANCHORS FOR OTHER THAN COMPOSITE FLOOR SYSTEM SHALL BE 3/4" DIAMETER WITH AN AFTER WELD LENGTH OF 5" AND SHALL CONFORM TO ASTM A108, UNLESS NOTED OTHERWISE.
- SHEAR CONNECTORS: PROVIDE AWS D1.1, TYPE B, 3/4" DIAMETER, SOLID FLUXED HEADED SHEAR CONNECTOR STUDS AUTOMATICALLY END WELDED THROUGH THE STEEL DECK AS SHOWN ON THE DRAWINGS AND IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER.
- DEFORMED BAR ANCHORS (DBAS): FLUX FILLED BARS AUTOMATICALLY WELDED TO STRUCTURAL STEEL IN ACCORDANCE WITH THE RECOMMENDATION OF THE MANUFACTURER. PROVIDE MATERIAL WITH MINIMUM YIELD STRENGTH OF 60 KSI.
- BEAM CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR FOR THE REACTIONS SHOWN ON THE PLANS. IF REACTIONS ARE NOT SHOWN, THE FABRICATOR SHALL DESIGN THE BEAM CONNECTIONS TO SUPPORT AN END REACTION OF W/2 KIPS (1.5W/2 KIPS FOR COMPOSITE BEAMS) FROM THE TABLES IN PART 3 "MAXIMUM TOTAL UNIFORM LOAD" OF THE MANUAL OF STEEL CONSTRUCTION (13th EDITION), BUT CONNECTIONS SHALL NOT HAVE LESS THAN 2 ROWS OF BOLTS NOR BE DESIGNED FOR LESS THAN 10 KIPS (LRFD). CONNECTIONS SHALL BE DESIGNED AS BEARING-TYPE CONNECTIONS WITH THREADS IN THE SHEAR PLANE. SUBMIT CALCULATIONS, SIGNED AND SEALED, BY AN ENGINEER LICENSED AND INSURED IN THE STATE OF GEORGIA.
- ALL BRACING CONNECTIONS SHALL DEVELOP THE FORCE NOTED ON THE DRAWINGS. IF FORCE IS NOT NOTED ON DRAWINGS, THE BRACING CONNECTION SHALL DEVELOP THE ALLOWABLE TENSION FORCE IN THE MEMBER. BRACING CONNECTIONS SHALL BE DESIGNED AND DETAILED SO THAT ALL FORCE COMPONENTS WILL BE TRANSMITTED DIRECTLY TO THE CENTER OF GRAVITY OF INTERSECTING MEMBERS. WHERE THIS IS NOT POSSIBLE, CONNECTIONS SHALL BE DESIGNED FOR ALL RESULTING ECCENTRICITIES. BOLTED BRACING CONNECTIONS SHALL BE CONNECTED WITH A MINIMUM OF TWO BOLTS. GUSSET PLATES SHALL BE DESIGNED BY THE FABRICATOR. SUBMIT CALCULATIONS, SIGNED AND SEALED, BY AN ENGINEER LICENSED AND INSURED IN THE STATE OF GEORGIA.
- ALL BEAM CONNECTIONS SHALL BE STANDARD FRAMED, SEATED END, OR SINGLE-PLATE SHEAR CONNECTIONS AS SHOWN IN PART 10 OF THE AISC MANUAL OF STEEL CONSTRUCTION, 13th EDITION.
- IN GENERAL, CONNECTIONS SHALL BE FIELD BOLTED. ALL BOLTS DESIGNATED "SLIP CRITICAL" OR "FULLY TIGHTENED" SHALL BE TIGHTENED TO THE MINIMUM PRETENSION VALUE SHOWN IN TABLE J3.1 OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS. IN ADDITION, CONNECTIONS DESIGNATED "SLIP CRITICAL" SHALL HAVE PROPERLY PREPARED FAYING SURFACES TO MEET CLASS A SURFACE CONDITION, U.N.O. "SLIP CRITICAL" CONNECTIONS SHALL INCLUDE ALL BOLTS IN MOMENT CONNECTIONS. "FULLY TIGHTENED" CONNECTIONS SHALL INCLUDE ALL BOLTS LOADED IN DIRECT TENSION (SUCH AS HANGERS), BRACED FRAME CONNECTIONS, GIRT CONNECTIONS & MEMBERS THAT ARE PART OF THE MAIN LATERAL RESISTING SYSTEM. DIRECT TENSION INDICATOR (DTI) WASHERS OR TENSION CONTROL BOLTS (TCB'S) SHALL BE USED AT THESE CONDITIONS. ALL OTHER BOLTS SHALL BE, AT MINIMUM, SNUG TIGHT.
- WELDING SHALL BE PERFORMED BY WELDERS WITH CURRENT CERTIFICATION USING ASTM E70 SERIES LOW HYDROGEN ELECTRODES. WELDS SHOWN ON STRUCTURAL DRAWINGS ARE MINIMUM DESIGN REQUIREMENTS. FABRICATION SHOP DRAWING SHALL REFLECT WELDS IN ACCORDANCE WITH AWS REQUIREMENTS. SHOP DRAWINGS SHALL DETAIL ALL SHOP AND FIELD WELDS. SHOP AND FIELD WELD SHOWN ON DRAWINGS FOR CONCEPT, GENERAL CONTRACTOR SHALL COORDINATE WELDING SEQUENCE REQUIREMENTS.
- PROVIDE TEMPORARY BRACING OF STRUCTURAL FRAMING UNTIL ALL PERMANENT BRACING, MOMENT CONNECTIONS AND FLOOR AND ROOF DECKS (DIAPHRAGMS) ARE COMPLETELY INSTALLED. THE STRUCTURAL ELEMENTS ARE UNSTABLE UNTIL THE STRUCTURE IS COMPLETED IN ACCORDANCE WITH THE PLANS.
- STEEL SHALL RECEIVE ONE SHOP COAT AND ONE FIELD TOUCH UP COAT OF APPROVED PAINT, EXCEPT WHERE STEEL IS IN CONTACT WITH FRESH CONCRETE. STEEL IS TO RECEIVE FIREPROOFING OR STEEL IS TO BE GALVANIZED. UNLESS NOTED OTHERWISE, ALL EXPOSED STRUCTURAL AND MISCELLANEOUS STEEL, PLATES, BOLTS, AND ANCHORS SHALL BE GALVANIZED OR PAINTED WITH APPROVED RUST INHIBITING PRIMER. CLEAN AREAS WHERE GALVANIZING IS DAMAGED OR MISSING AND REPAIR GALVANIZING TO COMPLY WITH ASTM A 780.
- ALL MEMBERS NOTED AS "AESS" SHALL COMPLY WITH THE CLASSIFICATION NOTED IN ACCORDANCE WITH AISC DEFINITIONS OR THE ARCHITECTURAL SPECIFICATIONS, WHICHEVER IS MORE STRINGENT. SEE ARCH. FOR ALL PAINTING AND PREPARATION REQUIREMENTS AND MATERIAL SPECIFICATIONS.

LIGHT GAUGE STEEL FRAMING:

- LIGHT GAUGE STEEL FRAMING SYSTEM SHALL BE DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRINCIPLES AND GOVERNING CODES. THE DESIGN SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA. SHOP DRAWINGS SHALL BE SUBMITTED WHICH BEAR THE SIGNATURE, DATE, AND SEAL OF THE ENGINEER. SHOP DRAWINGS SHALL CLEARLY INDICATE CONNECTIONS AND MATERIALS USED. SECTIONS AND DETAILS SHOWN ON THE DRAWINGS ARE FOR CONCEPT ONLY.
- LIGHT GAUGE STEEL FRAMING SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL PRIOR TO FABRICATION SHOWING WALL SECTIONS COORDINATED WITH DRAWINGS SHOWING FRAMING, ACCESSORIES, ANCHORAGE AND CONNECTION DETAILS.
- MATERIAL SPECIFICATIONS FOR LIGHT-GAUGE STEEL:
 - 16 GA. OR HEAVIER: ASTM A-446, F_y = 50 KSI MIN.
 - 18 GA. OR LIGHTER: ASTM A-446, F_y = 33 KSI MIN.
- GALVANIZING: MINIMUM G-60 COATING
- ALL STUDS AND JOIST MEMBERS SHALL BE STRUCTURAL (14 TO 20 GAUGE), AND HAVE STIFFENED FLANGES
- CONNECTION MATERIAL GAUGE MATCH STUD GAUGE. U.N.O. CLIP ANGLES SHALL BE 14 GA. MINIMUM.
- BUILT-UP MEMBERS FASTEN TOGETHER WITH 1" LONG STITCH WELDS OR #12 SCREWS AT 12" O.C. MAXIMUM, EACH FLANGE, AND EACH TRACK.
- PROVIDE BRIDGING AT 5' MAXIMUM VERTICAL SPACING IN WALLS.
- STUDS SHALL BE INSTALLED TO SEAT SQUARELY (WITHIN 1/16") AGAINST THE WEB PORTION OF THE TOP AND BOTTOM TRACKS. TRACKS SHALL REST ON A CONTINUOUS, UNIFORM BEARING SURFACE. ALL STUD MEMBERS SHALL BE ATTACHED TO TRACKS WITH #8-18 LOW PROFILE HEAD SCREWS AT EACH STUD FLANGE, EACH END. MEMBERS 18 GAUGE OR HEAVIER MAY BE WELDED.
- TEMPORARY BRACING SHALL BE PROVIDED AND LEFT IN PLACE UNTIL WORK IS PERMANENTLY STABILIZED.
- SPLICING OF MEMBERS SPANNING BETWEEN SUPPORTS SHALL NOT BE PERMITTED.
- VERTICAL ALIGNMENT (PLUMBNESS) OF STUDS SHALL BE WITHIN 1/960TH (1/8" IN 10'-0") OF THE SPAN.
- HORIZONTAL ALIGNMENT (LEVELNESS) OF WALLS SHALL BE WITHIN 1/960TH (1/8" IN 10'-0") OF THEIR RESPECTIVE LENGTHS.
- SPACING OF STUDS SHALL NOT BE MORE THAN ± 1/8" FROM THE DESIGNED SPACING PROVIDING THAT THE CUMULATIVE ERROR DOES NOT EXCEED THE REQUIREMENTS OF THE FINISHED MATERIALS.
- PROVIDE DEEP TRACK ASSEMBLY AT TOPS OF ALL NON-LOAD BEARING STUD WALLS TO ALLOW FOR MOVEMENT OF STRUCTURE. ARCHITECT SHALL REVIEW IN PLACE STEEL STUD CONSTRUCTION PRIOR TO THE INSTALLATION OF GYPSUM WALL BOARD OR SHEATHING.
- DESIGN COLD FORMED STEEL FRAMING SYSTEMS TO WITHSTAND THE INDICATED DESIGN LOADS WITHOUT EXCEEDING THE FOLLOWING DEFLECTION LIMITS:

MEMBERS SUPPORTING MASONRY:	L/600 OR 1/2" MAXIMUM
MEMBERS SUPPORTING OTHER MATERIALS:	L/360 OR 1" MAXIMUM

- STEEL TRUSS MANUFACTURER SHALL DESIGN FOR THE FOLLOWING SUPERIMPOSED GRAVITY LOADS:

TOP CHORD DEAD LOAD	15 PSF
BOTTOM CHORD DEAD LOAD	0 PSF
TOP CHORD LIVE LOAD	20 PSF
BOTTOM CHORD LIVE LOAD, U.N.O.	0 PSF
- DESIGN ROOF TRUSSES TO RESIST WIND UPLIFT PRESSURES IN ACCORDANCE WITH THE BUILDING CODE NOTED ABOVE. USE ROOF DEAD LOAD (AVAILABLE TO RESIST UPLIFT) LISTED IN THE DESIGN LOAD SECTION IN DETERMINING NET UPLIFT PRESSURES.

STEEL DECK:

- UNLESS NOTED OTHERWISE, STEEL DECK SHALL BE GALVANIZED (MINIMUM 60), CONFORMING TO STEEL DECK INSTITUTE (SDI) STANDARDS. FLOOR DECK SHALL BE GRADE 50 MINIMUM. ROOF DECK SHALL BE GRADE 50 MINIMUM.
- STEEL FLOOR DECK SHALL BE FASTENED TO STEEL FRAMING WITH 3/8" PUDDLE WELDS AT 12" O.C. WHEN DECK IS PERP. TO FRAMING, AND 24" O.C. WHEN DECK IS PARALLEL TO FRAMING. FASTEN DECK EDGES AT 12" O.C. SIDELAP SCREWS SHALL BE #10 SCREWS AT 24" O.C. MAX.
- STEEL ROOF DECK SHALL BE ATTACHED PER PLANS BUT NOT LESS THAN A 3/4" PATTERN USING 3/8" PUDDLE WELDS AND #10 SIDELAP SCREWS AT 24" O.C. MAX FASTEN DECK EDGES AND TO PARALLEL FRAMING AT 12" O.C.

STEEL STAIRS:

- ENGINEERED STEEL STAIR SYSTEM AND CONNECTIONS SHALL BE DESIGNED TO THIS STRUCTURE SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF GEORGIA. SUBMIT SHOP DRAWINGS BEARING THE SEAL AND SIGNATURE OF THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. THE CONFIGURATION OF THE STEEL STAIR SYSTEM SHALL BE AS SHOWN ON THE ARCHITECTURAL DRAWINGS. STEEL STAIR SYSTEM AND CONNECTIONS SHALL BE DESIGNED FOR APPLICABLE LOADS AS INDICATED ON THE PLANS AND IN THE BUILDING CODE. LOADS SHALL BE CLEARLY INDICATED ON SHOP DRAWINGS. SHOP DRAWINGS SHALL SHOW AND SPECIFY CONNECTIONS UTILIZED WITHIN THE STEEL STAIR SYSTEM AS WELL AS CONNECTIONS TO AND LOADS IMPROSED UPON THE STRUCTURAL SYSTEM SHOWN ON THESE PLANS.

SPECIAL INSPECTIONS:

- DURING CONSTRUCTION, SPECIAL STRUCTURAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 1705 OF THE 2018 IBC. AN APPROVED SPECIAL INSPECTOR WITH QUALIFICATIONS SATISFACTORY TO THE BUILDING OFFICIAL SHALL PERFORM SPECIAL INSPECTIONS. ALL SPECIAL STRUCTURAL INSPECTION REPORTS SHALL BE PREPARED BY AND BEAR THE SEAL OF THE SPECIAL INSPECTOR, AND ALL REPORTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL, ARCHITECT, AND TO THE STRUCTURAL ENGINEER.
- SPECIAL INSPECTOR SHALL PREPARE THE REQUIRED QUALITY ASSURANCE PLANS & SUBMIT PLAN TO BUILDING OFFICIAL, ARCHITECT, AND THE STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION.
- THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK FOR CONFORMANCE WITH THE PERMITTED CONSTRUCTION DOCUMENTS. THE SPECIAL INSPECTOR SHALL FURNISH PERIODIC INSPECTION REPORTS TO THE BUILDING OFFICIAL AND THE REGISTERED DESIGN PROFESSIONALS OF RECORD. THE FREQUENCY OF REPORTS SHALL BE AS AGREED UPON BY THE BUILDING OFFICIAL. ALL NONCONFORMING ITEMS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF CORRECTED, THE BUILDING OFFICIAL, ARCHITECT, AND THE STRUCTURAL ENGINEER.
- THE SPECIAL INSPECTOR, UPON COMPLETION OF THE WORK AND PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY, SHALL SUBMIT A SIGNED & SEALED FINAL REPORT DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE PRIOR REPORTS.
- ALL STRUCTURAL ELEMENTS OF THE BUILDING FRAME SHALL BE INSPECTED FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS AND REQUIREMENTS OF SECTION 1705 OF THE 2018 IBC, INCLUDING, BUT NOT BE LIMITED TO THE SECTIONS LISTED ON THIS DRAWING.
- SPECIAL INSPECTIONS FOR WIND RESISTANCE ARE NOT REQUIRED PER IBC 1705.10.
- SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE ARE REQUIRED PER IBC 1705.11.

SCHEDULE OF SPECIAL INSPECTIONS

MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT	
		Y/N	EXTENT
1704.2 INSPECTION OF FABRICATIONS			
VERIFY FABRICATION/QUALITY CONTROL PROCEDURES	IN-PLANT REVIEW	Y	PERIODIC
1705.2 STEEL CONSTRUCTION			
MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS, AND WASHERS.	REVIEW MATERIAL MARKINGS AND CERTIFICATES OF COMPLIANCE	Y	PERIODIC
INSPECTION OF HIGH-STRENGTH BOLTING:			
A. SNUG TIGHT JOINTS		Y	PERIODIC
B. PRE-TENSIONED OR SLIP CRITICAL			
1) TURN-OFF-NUT WITH MATCHING MARKINGS		Y	PERIODIC
2) DIRECT TENSION INDICATOR		Y	PERIODIC
3) TWIST-OFF BOLT		Y	PERIODIC
4) TURN-OF-NUT WITHOUT MATCHING MARKINGS		Y	CONTINUOUS
5) CALIBRATED WRENCH		Y	CONTINUOUS
MATERIAL VERIFICATION OF STRUCTURAL STEEL:			
A. IDENTIFICATION MARKINGS	FIELD INSPECTION	Y	PERIODIC
B. CERTIFIED MILL TESTS	REVIEW SUBMITTALS	Y	PERIODIC
WELD FILLER MATERIALS:			
	REVIEW CERTIFICATE OF COMPLIANCE AND FIELD VERIFICATION	Y	PERIODIC AND EACH SUBMITTAL
STRUCTURAL STEEL WELDING:			
A. COMPLETE AND PARTIAL PENETRATION GROOVE WELDS			CONTINUOUS
B. MULTI-PASS FILLET WELDS		Y	CONTINUOUS
C. SINGLE-PASS FILLET WELDS > 1/4"		N	CONTINUOUS
D. SINGLE-PASS FILLET WELDS < 1/4"		Y	PERIODIC
INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS			
A. DETAILS SUCH AS BRACING AND STIFFENERS		Y	PERIODIC
B. MEMBER LOCATIONS		Y	PERIODIC
C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION		Y	PERIODIC
1705.2.1 STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL			
VERIFICATION OF COLD-FORMED STEEL DECK:			
A. IDENTIFICATION OF MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS		Y	PERIODIC
B. MANUFACTURER'S CERTIFIED TEST REPORTS		Y	PERIODIC
INSPECTION OF WELDING			
A. COLD-FORMED STEEL DECK:			
1. FLOOR AND ROOF DECK WELDS		Y	PERIODIC

SCHEDULE OF SPECIAL INSPECTIONS

MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT	
		Y/N	EXTENT
1705.3 CONCRETE CONSTRUCTION			
INSPECTION OF REINFORCING STEEL INSTALLATION.	FIELD INSPECTION		PERIODIC
INSPECTION OF CAST-IN-PLACE BOLTS PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED PER THE BUILDING CODE	FIELD INSPECTION		PERIODIC
INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS PER APPROVED RESEARCH REPORT IN ACCORDANCE WITH ACI 308.2	FIELD INSPECTION	Y	CONTINUOUS
VERIFY USE OF APPROVED DESIGN MIX	FIELD REVIEW	Y	PERIODIC
FRESH CONCRETE SAMPLING, BEATM STAMP AND AIR CONTENT TESTS AND DETERMINE TEMPERATURE OF CONCRETE	FIELD INSPECTION	Y	CONTINUOUS
INSPECTION OF CONCRETE AND PHOTOGRAPHIC RECORD FOR PROPER APPLICATION TECHNIQUES	FIELD INSPECTION	Y	CONTINUOUS
INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		Y	PERIODIC
ERECTION OF PRECAST CONCRETE MEMBERS	FIELD INSPECTION	N	PERIODIC
EVALUATION OF CONCRETE STRENGTH	FIELD INSPECTION	Y	PERIODIC
VERIFICATION OF IN-SITU CONCRETE STRENGTH PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS	FIELD INSPECTION	Y	PERIODIC
INSPECTION OF FORMWORK FOR SHAPE, LINES, LOCATION AND DIMENSIONS	FIELD INSPECTION	Y	PERIODIC
TESTING OF CONCRETE FLOOR FLATNESS AS REQUIRED PER CONSTRUCTION DOCUMENTS.	FIELD INSPECTION	Y	PERIODIC
SOILS			
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	FIELD INSPECTION	Y	PERIODIC
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	FIELD INSPECTION	Y	PERIODIC
PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.	FIELD INSPECTION	Y	PERIODIC
VERIFY SITE PREPARATION COMPLIES WITH APPROVED SOILS REPORT.	FIELD INSPECTION	Y	CONTINUOUS
VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL	FIELD INSPECTION	Y	CONTINUOUS
PRIOR TO PLACEMENT OF CONTROLLED FILL OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	FIELD INSPECTION	Y	PERIODIC
VERIFY DRY-DENSITY OF COMPACTED FILL COMPLIES WITH APPROVED SOILS REPORT.	REVIEW FIELD TESTING	Y	PERIODIC

OPEN WEB STEEL JOISTS:

- STEEL JOIST CONSTRUCTION SHALL CONFORM TO STEEL JOIST INSTITUTE LOAD TABLES, "STANDARD SPECIFICATIONS", AND "CODE OF STANDARD PRACTICE".
- IN ADDITION TO DEAD AND LIVE LOADS, STEEL ROOF JOISTS SHALL BE DESIGNED AND FABRICATED TO RESIST A NET WIND UPLIFT OF 15 PSF (SERVICE LOAD), UNLESS NOTED OTHERWISE.
- ALL JOISTS SHALL BE DESIGNED FOR A CONCENTRATED LIVE BEND LOAD OF 200 POUNDS LOCATED AT ANY POSITION ALONG THE TOP OR BOTTOM CHORD CONCURRENTLY WITH ALL OTHER DESIGN LOADS.
- SUBMIT SHOP DRAWINGS COMPLETELY DETAILING THE JOISTS CONNECTION. INCLUDE BRIDGING AND CONNECTIONS. CALCULATIONS SHALL BE SIGNED AND DATED BY A STRUCTURAL ENGINEER REGISTERED IN THE PROJECT STATE.
- ROOF JOISTS AT PERIMETER BEAMS SHALL HAVE JOISTS DESIGNED FOR 2,250# ROLLOVER FORCE (WIND/SERVICE)

STEEL BAR GRATE:

- STEEL BAR GRATE SHALL BE GENERAL FLOORING GRATING, TYPE 19-W-4, CONFORMING TO ASTM A36. BEARING BARS SHALL BE 1" x 3/4" SPACED AT 1 1/2". CROSS BARS SHALL BE STEEL RODS SPACED AT 4". FASTEN BAR GRATE TO ALL SUPPORT STEEL (ENDS, INTERMEDIATE AND SIDES) WITH 3/8" FILLET WELDS x 3/4" LONG SPACED AT 12" O.C. MAX. PROVIDE A MINIMUM OF TWO WELDS AT EACH SIDE AND EACH SUPPORT MEMBER.



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REVISIONS

NO.	DESCRIPTION

RENEWED MEDICAL BUILDING
CORE & SHELL ONLY
 SOUTH FULTON, GEORGIA
 PROJECT #19-9883



03/27/2020
SEAL
MARCH 27, 2020
PERMIT SET
RELEASED FOR CONSTRUCTION

GENERAL NOTES

DRAWING TITLE

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19-9883

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