

GENERAL NOTES

GENERAL

- 1. ALL DESIGN AND CONSTRUCTION SHALL CONFORM TO THE BUILDING CODE REFERENCED IN THE DESIGN BASIS AND THAT BUILDING CODE'S REFERENCED CODES AND STANDARDS.
2. THE STRUCTURAL DRAWINGS SHALL BE UTILIZED IN CONJUNCTION WITH DRAWINGS FROM OTHER DESIGN TRADES, SUCH AS ARCHITECTURAL, MECHANICAL, ELECTRICAL, ETC. THE CONTRACTOR IS TO INFORM THE ENGINEER OF ANY CONFLICTS BETWEEN THE STRUCTURAL DRAWINGS AND ANY OTHER DRAWINGS.
3. THE STRUCTURAL GENERAL NOTES AND STRUCTURAL DRAWINGS ARE TO BE USED IN CONJUNCTION WITH THE PROJECT SPECIFICATIONS. IN CASE OF ANY CONFLICTS BETWEEN THE NOTES, DRAWINGS AND SPECIFICATION, THE STRICTEST REQUIREMENTS PREVAIL. THE CONTRACTOR IS TO INFORM THE ENGINEER OF ANY CONFLICTS.

DRAWINGS

- 1. APPLY DETAILS, SECTIONS AND NOTES ON THE DRAWINGS WHERE CONDITIONS ARE SIMILAR TO THOSE INDICATED. AT FEATURES THAT ARE NOT FULLY DETAILED OR SPECIFIED, THEIR CONSTRUCTION IS TO BE SIMILAR TO WHAT IS SHOWN OR SPECIFIED AT OTHER SIMILAR CONDITIONS.
2. DETAILS LABELED 'TYPICAL' AND NOTES INCLUDING THE WORD 'TYPICAL (OR TYP.)' APPLY AT ALL CONDITIONS THAT ARE SIMILAR IN NATURE, UNLESS NOTED OTHERWISE.
3. USE DIMENSIONS INDICATED ON DRAWINGS. DO NOT SCALE DRAWINGS.
4. CENTERLINES OF COLUMNS, BEAMS, GRADE BEAMS, WALLS, FOUNDATIONS, AND OTHER FRAMING MEMBERS COINCIDE WITH ESTABLISHED GRIDLINES, UNLESS NOTED OTHERWISE.
5. BEAMS, JOISTS, RAFTERS, ETC. ARE ASSUMED TO BE SPACED EQUALLY IF NOT INDICATED OTHERWISE.

EXISTING CONDITIONS

- 1. THE CONTRACTOR IS TO FIELD VERIFY ALL EXISTING CONDITIONS. THE CONTRACTOR IS TO INFORM THE ENGINEER OF ANY DISCREPANCIES BETWEEN THE EXISTING CONDITIONS AND THE STRUCTURAL DRAWINGS, AND OF ANY CONDITIONS THAT MAY NOT HAVE BEEN VISIBLE PRIOR TO CONSTRUCTION.

TEMPORARY CONDITIONS

- 1. THE STRUCTURE HAS BEEN DESIGNED SUCH THAT IT IS STABLE ONCE ALL ELEMENTS OF THE LATERAL LOAD-RESISTING SYSTEM ARE IN PLACE. THIS INCLUDES ELEMENTS SUCH AS FOUNDATIONS, COLUMNS, BEAMS, BRACES, DECKING AND WELDING. THE CONTRACTOR IS RESPONSIBLE FOR THE TEMPORARY STABILITY OF THE STRUCTURE.
2. THE STRUCTURE HAS NOT BEEN DESIGNED TO ACCOMMODATE ANY CONSTRUCTION LOADING THAT HAS NOT BEEN INDICATED IN THE DESIGN BASIS OR ON THE PLANS. THIS INCLUDES LOADS SUCH AS CONSTRUCTION VEHICLES, LIKE CRANES AND MANLIFTS, OR LOADS DUE TO THE STORAGE OF MATERIALS, LIKE PALLETS OF DRYWALL, PLYWOOD OR STONE. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING THE STRUCTURAL CAPACITY OF ANY ELEMENTS THEY INTEND TO LOAD BEYOND THE STATED LOADS. THE CONTRACTOR IS TO SUBMIT DESIGN CALCULATIONS, SIGNED AND SEALED BY A REGISTERED DESIGN PROFESSIONAL, INDICATING AS SUCH.

SUBMITTALS

- 1. THE CONTRACTOR IS TO PREPARE AND TRANSMIT ALL SUBMITTALS TO THE ENGINEER AND/OR ARCHITECT WITH ADEQUATE TIME TO REVIEW PRIOR TO CONSTRUCTION OR FABRICATION. THE CONTRACTOR IS ALSO TO PREPARE AND KEEP CURRENT A LIST OF ALL SUBMITTALS AND A SUBMITTAL SCHEDULE.
2. THE REUSE OR REPRODUCTION OF ANY PORTION OF THESE DOCUMENTS FOR USE AS SHOP DRAWINGS IS STRICTLY PROHIBITED WITHOUT THE WRITTEN PERMISSION OF MCFARLAND JOHNSON, INC.
3. SHOP DRAWINGS ARE TO BE PREPARED IN ACCORDANCE WITH THE STANDARDS GOVERNING THE TYPE OF WORK.
4. SUBMITTALS REQUIRING REVIEW BY THE STRUCTURAL ENGINEER INCLUDE:
A. SHOP DRAWINGS.
B. DESIGN CALCULATIONS.
C. MIX DESIGNS, AND MATERIAL CERTIFICATES.
D. PRODUCT DATA, REPORTS AND OTHER LITERATURE.

SUBMITTALS

- 5. SHOP DRAWINGS ARE TO BE PROVIDED FOR ALL STRUCTURAL ELEMENTS. SHOP DRAWINGS ARE TO INCLUDE ITEMS SUCH AS:
A. PLANS, ELEVATIONS AND SECTIONS.
B. LAYOUT OF BEAMS, COLUMNS, WALLS, DECKING, ANCHOR BOLTS, ETC.
C. LAYOUT OF EMBEDDED ITEMS.
D. LAYOUT OF SLAB/FLOOR/ROOF DECK AND WALL OPENINGS ALONG WITH BEAM PENETRATIONS.
E. FASTENING, ATTACHMENTS, SHOP WELDING AND FIELD WELDING.
F. LAYOUT AND MAGNITUDE OF ANY LOADING ON THE STRUCTURE.

SUBMITTALS

- 6. DEFERRED SUBMITTALS ARE THOSE WHERE THE DESIGN OF SPECIFIC ELEMENTS AND THEIR ATTACHMENTS HAS NOT BEEN COMPLETED AS PART OF THE CONSTRUCTION DOCUMENTS BUT ARE DELEGATED TO THE DESIGN BY A SPECIALTY OR SUB-CONTRACTED ENGINEER. DEFERRED SUBMITTALS ON THIS PROJECT INCLUDE:
A. COLD-FORMED STEEL FRAMING.
B. STRUCTURAL STEEL CONNECTIONS.
C. PRECAST CONCRETE JOISTS.
D. METAL FABRICATIONS.

- 7. THE OWNER, CONTRACTOR OR SUB-CONTRACTOR IS TO ENGAGE THE REGISTERED DESIGN PROFESSIONAL TO PROVIDE DESIGN OF ELEMENTS AS PART OF A DEFERRED SUBMITTAL. CALCULATIONS ARE TO BE SIGNED AND SEALED BY A REGISTERED DESIGN PROFESSIONAL.

- 8. DEFERRED SUBMITTALS ARE TO INCLUDE AT A MINIMUM BOTH SHOP DRAWINGS AND CALCULATIONS. THEY ARE TO BE SUBMITTED TO THE ENGINEER, ARCHITECT AND BUILDING OFFICIAL.

TESTING AND INSPECTIONS

- 1. THE OWNER IS TO ENGAGE AN INDEPENDENT TESTING LABORATORY TO PROVIDE INSPECTIONS AS REQUIRED PER THE STATEMENT OF SPECIAL INSPECTIONS. REFER TO THE SPECIFICATIONS.

CAST-IN-PLACE CONCRETE

CODES

- 1. ALL CONCRETE WORK SHALL COMPLY WITH THE FOLLOWING CODES AND STANDARDS AND ANY OTHER CODES, STANDARDS, OR OTHER DOCUMENTS REFERENCED THEREIN. VERSIONS OR EDITIONS SHALL BE AS REFERENCED BY THE BUILDING CODE INDICATED IN THE DESIGN BASIS.

- A. ACI (AMERICAN CONCRETE INSTITUTE) BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY - ACI 318
B. AWS (AMERICAN WELDING SOCIETY) STRUCTURAL WELDING CODE - REINFORCING STEEL - AWS D1.4

MATERIALS

- 1. ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE (145 PCF) AND MEET THE FOLLOWING REQUIREMENTS.

- A. MINIMUM COMPRESSIVE STRENGTH (FC) PER DAY:
1. ALL CONCRETE: 4,000 PSI
B. PORTLAND CEMENT IS TO BE PER ASTM C150, TYPE I OR II.
C. AGGREGATE IS TO BE PER ASTM C33. 1" MAXIMUM AGGREGATE SIZE.
D. WATER IS TO BE POTABLE WITH A MAXIMUM WATER/CEMENT RATIO OF 0.50.
E. USE AN AIR ENTRAINING ADMIXTURE CONFORMING TO ASTM C260. AIR CONTENT TO BE BETWEEN 4-6% ±1%.
F. CONTRACTOR TO SUBMIT PREFERRED PROPOSED ADMIXTURES WITH MIX DESIGN. INCLUDE APPLICABILITY OF ADMIXTURES WHEN USED TOGETHER.

CAST-IN-PLACE CONCRETE CONTINUED

REINFORCING STEEL SHALL MEET THE FOLLOWING REQUIREMENTS:

- A. DEFORMED BARS: ASTM A615, GRADE 60
B. WELDBLE DEFORMED BARS: ASTM A706, GRADE 60

CONCRETE PLACEMENT

- 1. THERE SHALL BE NO HORIZONTAL JOINTS IN ANY CONCRETE POURS UNLESS SPECIFICALLY NOTED.
2. THE CONCRETE CONTRACTOR SHALL COORDINATE AND INSTALL (OR GIVE OTHER TRADES AMPLIFIED OPPORTUNITY TO INSTALL) ALL EMBEDDED ITEMS AND LEAVE-OUTS SUCH AS ANCHORS, BOLTS, PLATES, NAILERS, SLOTS, CHASES, PIPE SLEEVES, ETC.
3. PROVIDE A 3/4" CHAMFER AT ALL EDGES OF EXPOSED BEAMS AND COLUMNS.
4. THE CONCRETE CONTRACTOR SHALL DESIGN, FURNISH AND INSTALL ANY TEMPORARY SHEETING, SHORING OR BRACING NECESSARY. DESIGN TO BE PROVIDED BY A REGISTERED DESIGN PROFESSIONAL.

CONCRETE CURING

- 1. HORIZONTAL SURFACES SHALL BE KEPT MOIST FOR A MINIMUM OF SEVEN DAYS.
2. CONCRETE TEMPERATURE DURING THE FIRST SEVEN DAYS SHALL BE MAINTAINED BETWEEN 50°F AND 90°F. PREVENT RAPID DRYING.
3. VERTICAL SURFACES SHALL RECEIVE TWO COATS (ONE AT TIME OF STRIPPING AND ANOTHER THREE DAYS LATER) OF AN APPROVED CURING COMPOUND.
4. CONTRACTOR SHALL REPAIR AT HIS EXPENSE ALL CONCRETE SLAB DEFECTS SUCH AS CURLING AND CRACKING. GRINDING, PATCHING, OR OTHER REPAIR PROCEDURES SHALL BE APPROVED BY THE ENGINEER PRIOR TO STARTING WORK.

REINFORCING PLACEMENT

- 1. CLEAR COVER OF REINFORCING BARS SHALL BE AS FOLLOWS:
A. BOTTOM OF FOOTINGS: 3"
B. FACES EXPOSED TO WEATHER - #5 BAR OR SMALLER: 1-1/2"
C. FACES EXPOSED TO WEATHER - #6 BAR OR LARGER: 2"
D. FACES NOT EXPOSED TO WEATHER: 3/4"
E. BEAM AND COLUMN STIRRUPS: 1-1/2"
2. LAP ALL BARS A MINIMUM OF 30 BAR DIAMETERS. IN BEAMS, SPLICE TOP BARS AT MIDSPAN AND BOTTOM BARS OVER SUPPORTS. SPLICE A MAXIMUM OF 50% OF THE TENSION BARS AT ANY ONE POINT. IN WALLS, SPLICE BARS IN A STAGGERED PATTERN, ALTERNATING EACH FACE.
3. BARS INDICATED AS CONTINUOUS ARE TO BE RUN CONTINUOUSLY AROUND CORNERS AND DOWELED INTO INTERSECTING WALLS. LAP BARS AS NECESSARY.
4. SPLICE WELDED WIRE MESH A MINIMUM OF EITHER ONE FULL MESH OR 6 INCHES.
5. REINFORCING SHALL BE SECURELY TIED IN ITS PROPER PLACE BEFORE AND DURING PLACEMENT OPERATIONS USING ONLY APPROVED CHAIRS AND SPACERS. USE PLASTIC TIPPED ACCESSORIES IN CONCRETE EXPOSED TO WEATHER, WATER OR VIEW.
6. NO BARS SHALL BE CUT OR OMITTED IN THE FIELD WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.

STRUCTURAL STEEL

CODES

- 1. ALL STRUCTURAL STEEL WORK SHALL COMPLY WITH THE FOLLOWING CODES AND STANDARDS AND ANY OTHER CODES, STANDARDS, OR OTHER DOCUMENTS REFERENCED THEREIN. VERSIONS OR EDITIONS SHALL BE AS REFERENCED BY THE BUILDING CODE INDICATED IN THE DESIGN BASIS.

- A. AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION) SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS - AISC 360
B. AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES - AISC 308
C. AISC SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS - AISC 341
D. RCSC (RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS) SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS
E. AWS (AMERICAN WELDING SOCIETY) STRUCTURAL WELDING CODE - STEEL - AWS D1.1

MATERIALS

- 1. STRUCTURAL STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING REQUIREMENTS, UNLESS NOTED OTHERWISE:
A. WIDE FLANGE AND WT SHAPES: ASTM A992 (FY = 50 KSI)
B. ANGLES, CHANNELS, PLATES: ASTM A36 (FY = 36 KSI)
C. HOLLOW STRUCTURAL SECTIONS: ASTM A500 GRADE B (FY = 42 KSI FOR ROUND AND 48 FOR RECTANGULAR/SQUARE)
D. PIPE: ASTM A53, GRADE B (FY = 35 KSI)
E. BOLTS: ASTM A325
F. NUTS: ASTM A663
G. WASHERS: ASTM A436
H. ANCHOR RODS: ASTM F1554, GRADE 55
I. LEADED SPODS: ASTM A108
J. ELDED ELECTRODES: E70XX

- 2. STEEL SECTIONS, PLATES, ANCHOR RODS, NUTS, BOLTS AND WASHERS ARE TO BE GALVANIZED. PROVIDE BOLT HOLES IN PIECES AS NECESSARY. LOCATE VENT HOLES IN ORDER TO PROVIDE POSITIVE DRAINAGE. DO NOT BRANCH CONNECTIONS OF PIECES TO BE FIELD WELDED. COAT FIELD WELDED LOCATIONS WITH RICH PRIMER AFTER WELDING OPERATIONS.

CONNECTIONS

- 1. CONNECTIONS SHALL BE EITHER BOLTED OR WELDED. FIELD CONNECTIONS ARE TO BE BOLTED UNLESS DETAILED OTHERWISE.
2. BOLTED CONNECTIONS ARE TO HAVE A MINIMUM OF TWO 3/4" Ø BOLTS. PROVIDE BOLT HOLES AT CONNECTIONS, COLUMN BASE PLATES, LEVELING PLATES, ETC. AS REQUIRED BY THE STEEL CONSTRUCTION MANUAL UNLESS DETAILED OTHERWISE.

COLD-FORMED STEEL FRAMING AND PRECAST CONCRETE

GENERAL

- 1. TO BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA.
2. SEE PLANS FOR CONCEPTUAL FRAMING AND SPECIFICATIONS FOR REQUIREMENTS.

MISCELLANEOUS

ROOFING AND CLADDING

- 1. USE ONLY PRODUCTS APPROVED FOR USE IN MIAMI-DADE COUNTIES.
2. SIZE GUTTER AND DOWNSPOUTS PER FLORIDA BUILDING CODE. PLUMBING TO MATCH EXISTING TO BEST EXTENT POSSIBLE.
3. MATCH WALL PANEL AND STANDING SEAM ROOF COLORS TO EXISTING BUILDING.

DESIGN BASIS

BUILDING CODE

- 1. THE CONSTRUCTION DOCUMENTS ARE BASED ON THE REQUIREMENTS OF THE 2017 FLORIDA BUILDING CODE.

BUILDING RISK CATEGORY (PER IBC)

- 1. PARKING GARAGE: II

DEAD LOADS

- 1. SUPERIMPOSED DEAD LOADS TO ACCOUNT FOR GENERAL ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND FIRE PROTECTION SYSTEMS HAVE BEEN APPLIED AS FOLLOWS:
A. SECOND FLOOR: 5 PSF
B. ROOF: 10 PSF

LIVE LOADS

- 1. DESIGN LIVE LOADS HAVE BEEN APPLIED AS FOLLOWS. CONCENTRATED LOADS HAVE BEEN APPLIED AS UNIFORMLY DISTRIBUTED ON AN AREA OF 2.5 SQUARE FEET, UNLESS NOTED OTHERWISE.
UNIFORM CONCENTRATED
A. PARKING GARAGE: 40 PSF 3,000 LBS ON 4.5 X 4.5 INCHES
B. HANDRAILS: 50 LBS/FT 200 LBS AT ANY POINT
C. ROOF: 20 PSF
D. STAIRWELLS/EXITS: 100 PSF 300 LBS ON 4 SQUARE INCHES

SNOW LOADS

- 1. SNOW LOADING HAS BEEN DETERMINED PER ASCE 7-10, CHAPTER 7. CRITERIA IS AS FOLLOWS:
A. GROUND SNOW LOAD, Pg: 0.0 PSF
B. FLAT ROOF SNOW LOAD, Pf: 0.0 PSF

WIND LOADS

- 1. BUILDING WIND LOADING HAS BEEN DETERMINED PER ASCE 7-10, CHAPTER 25 AND 28. CRITERIA IS AS FOLLOWS:
A. WIND EXPOSURE CATEGORY: B
B. ULTIMATE BASIC WIND SPEED, V: 115 MPH
C. INTERNAL PRESSURE COEFFICIENT Gcpi: ±0.3 (CONCRETE STRUCTURE) ±0.3 (ROOF PLENUM)

SEISMIC LOADS

- 1. BUILDING SEISMIC LOADING HAS BEEN DETERMINED PER ASCE 7-10, CHAPTERS 11 AND 12. CRITERIA IS AS FOLLOWS:
A. SHORT PERIOD SPECTRAL ACCELERATION, Ss: 0.356 g
B. ONE SECOND PERIOD SPECTRAL ACCELERATION, S1: 0.120 g
C. SITE CLASS: B
D. SHORT PERIOD SITE COEFFICIENT, Fa: 1.0
E. ONE SECOND PERIOD SITE COEFFICIENT, Fv: 1.0
F. SHORT PERIOD SPECTRAL RESPONSE ACCELERATION, Sds: 0.224 g
G. ONE SECOND PERIOD SPECTRAL ACCELERATION, Sd1: 0.080 g
H. SEISMIC IMPORTANCE FACTOR, Ie: 1.0
I. SEISMIC DESIGN CATEGORY, D: B
J. SEISMIC REDUNDANCY FACTOR, R: 1.0
K. SEISMIC FORCE-RESISTING SYSTEM: ORDINARY CONCRETE MOMENT FRAMES
L. RESPONSE MODIFICATION FACTOR, R: 3.0
M. OVERSTRENGTH FACTOR, Qs: 2.5
N. DEFLECTION AMPLIFICATION FACTOR, Cd: 2.5
O. ANALYTICAL PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE
P. SEISMIC BASE SHEAR: 207 KIPS

DESIGN BASIS CONTINUED

- 2. SEISMIC FORCES ON STRUCTURAL AND NON-STRUCTURAL COMPONENTS AND THEIR ATTACHMENTS HAVE BEEN DETERMINED PER ASCE 7-10, CHAPTERS 11, 12 AND 13.

- A. DESIGNERS OF ELEMENTS AS PART OF A DEFERRED SUBMITTAL ARE TO DETERMINE SEISMIC LOADING SPECIFIC TO THOSE ELEMENTS PER ASCE 7-10, CHAPTERS 11, 12 AND 13 OR ANOTHER RELEVANT SEISMIC LOADING PROCEDURE.

FOUNDATION LOADS AND SOIL CAPACITY

- 1. FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL REPORT PREPARED BY TIERRA SOUTH FLORIDA, INC. DATED AUGUST 13, 2019. PILE RECOMMENDATIONS ARE AS FOLLOWS:
A. PILE TYPE: 14"Ø AUGERCAST PILE
B. GROUT STRENGTH: 4,000 PSI
C. MINIMUM EMBEDMENT: 5 FT INTO LIMESTONE
D. ESTIMATED LENGTH: 10 FT BELOW GRADE
E. COMPRESSIVE CAPACITY: 35 TONS ± 100 TONS
F. TENSION CAPACITY: 20 TONS

FUTURE EXPANSION

- 1. THE BUILDING DESIGN AS PART OF THIS SET OF DOCUMENTS DOES NOT INCLUDE ANY ALLOWANCES FOR FUTURE BUILDING EXPANSION.

CODE INFORMATION

- 1. ZONING CLASSIFICATION: AIRPORT DISTRICT (A)

CONSTRUCTION

- 1. CONSTRUCTION METHOD: EXPANSION OF EXISTING PARKING GARAGE WITH PARTIAL ROOF COVERING

EXISTING BUILDING

- 1. OCCUPANCY: PARKING GARAGE
2. USE GROUP: S-2
3. CONSTRUCTION TYPE: 1B
4. FLOOR AREA: 35,463 FT²
5. SPRINKLERED: NO

NEW BUILDING

- 1. OCCUPANCY: PARKING GARAGE
A. BUILDING QUALIFIES AS AN OPEN PARKING GARAGE PER 406.5.2
2. USE GROUP: S-2
3. CONSTRUCTION TYPE: 2B (STEEL FRAME) ON 1B (CONCRETE)
4. FLOOR AREA: 8,768 FT²
5. ROOF AREA: 5,157 FT²
6. SPRINKLERED: NO

CODE INFORMATION

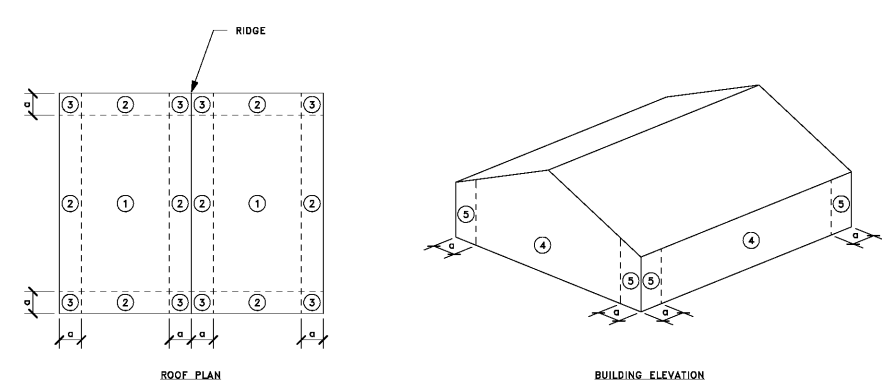
- 1. TOTAL FLOOR AREA: 44,321 FT²
2. ALLOWABLE FLOOR AREA: UNLIMITED (1B)
3. ALLOWABLE ROOF AREA: 50,000 FT² (2B)
4. BUILDING HEIGHT: 2 TIERS
5. ALLOWABLE HEIGHT: 12 TIERS

FIRE RATING

- ELEMENT CONSTRUCTION TYPE FIRE RATING (HOURS)
1. PRIMARY STRUCTURAL FRAME: 1B 2
2. FLOOR CONSTRUCTION: 1B 2
3. PRIMARY STRUCTURAL FRAME: 2B 0
4. ROOF CONSTRUCTION: 2B 0

EGRESS

- 1. EXISTING WEST EGRESS STAIR IS BEING REMOVED AND REPLACED.
2. OCCUPANT LOAD IS BASE ON FLOOR AREA OF ENTIRE PARKING GARAGE.
3. OCCUPANT LOAD: 222 OCCUPANTS
4. NUMBER OF EGRESS STAIRS: 2
5. OCCUPANT LOAD PER EGRESS STAIR: 111 OCCUPANTS
6. MAXIMUM EXIT ACCESS DISTANCE: 270 FT



CONTRACTOR NOTE

Table with 5 columns: AREA 1, AREA 2, AREA 3, AREA 4, AREA 5. Each column has sub-columns for PRESSURE and SUCTION. Rows represent wind area (FT²) categories: ≤ 10, 20, 50, ≥ 100.

- NOTES:
1. REFER TO PLAN AND ELEVATION FOR WIND AREA LOCATIONS.
2. PRESSURE AND SUCTION VALUES ARE ULTIMATE WIND PRESSURES GIVEN IN POUNDS PER SQUARE FOOT. TO FIND ALLOWABLE VALUES, MULTIPLY THE LISTED VALUE BY 0.6.
3. VALUES IN BETWEEN LISTED EFFECTIVE WIND AREAS MAY BE LINEARLY INTERPOLATED.
4. DISTANCE "a" IS 5'-3".



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PROJECT MILESTONE table with columns: NO., DATE, DESCRIPTION. Row 1: BID PLANS.

CLIENT: KEY WEST INTERNATIONAL AIRPORT
PROJECT: AIRPORT MAINTENANCE STORAGE FACILITY PROJECT
KEY WEST, FL

Table with columns: DRAWN, DESIGNED, CHECKED, SCALE, DATE, PROJECT. Values: OHG, OHG, CEP, MARCH 4, 2020, 1821.05.

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY, IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

DRAWING TITLE
GENERAL NOTES AND DESIGN BASIS

DRAWING NUMBER