GENERAL NOTES

A. BUILDING AND DESIGN CODES:

1. INTERNATIONAL BUILDING CODE 2012 WITH APPLICABLE MODIFICATIONS AND ORDINANCES.
2. ASCE 7-10 MINIMUM DESIGN LOADS BUILDING AND OTHER STRUCTURES.
3. FACTORY MUTUAL PUBLICATION 1-2 FOR EARTHQUAKE LOADS AND 1-7, 1-28 AND 1-28S FOR WIND LOADS.
4. AISC MANUAL OF STEEL CONSTRUCTION, 13TH EDITION.
5. ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, THE LATEST EDITION.
6. ACI 530 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES, THE LATEST EDITION.

B. DESIGN LOADS:

1. LIVE LOADS:	UNIFORM LOAD
a. FLAT ROOF	20 PSF
b. PITCHED ROOF	12 PSF

LIVE LOAD REDUCTION ON SUPPORTING ELEMENTS IN ACCORDANCE WITH 2012 IBC.

LOADING FOR MECHANICAL AND ELECTRICAL ROOMS ARE BASED ON THE WEIGHTS OF ASSUMED EQUIPMENT, AS INDICATED ON THE MECHANICAL DRAWINGS (INCLUDING THE WEIGHT OF CONCRETE PADS WHERE INDICATED ON THE MECHANICAL DRAWINGS). ANY CHANGES IN TYPE, SIZE, LOCATION, OR NUMBER OF PIECES OF EQUIPMENT SHOULD BE REPORTED TO THE ARCHITECT FOR VERIFICATION OF THE ADEQUACY OF SUPPORTING MEMBERS PRIOR TO PLACEMENT OF EQUIPMENT.

2. DEAD LOADS:	UNIFORM LOAD
a. FLAT ROOF	18 PSF
b. PITCHED ROOF	14 PSF

DESIGN DEAD LOADS INCLUDE THE WEIGHT OF STRUCTURAL COMPONENTS AND PERMANENT FIXTURES.

3. WIND LOADS:
   a. ULTIMATE DESIGN WIND SPEED (3 SECOND GUST) = 115 MPH
   b. RISK CATEGORY - II
   c. EXPOSURE - B
   d. INTERNAL PRESSURE COEFFICIENT - +/- 0.18
   e. COMPONENT AND CLADDING DESIGN PRESSURE - 20.2 PSF
   f. DESIGN UPLIFT - 12.0 PSF (NET)

4. SEISMIC LOADS
   a. SEISMIC OCCUPANCY CATEGORY - II
   b. SPECTRAL RESPONSE COEFF. - Sds = 0.101 Sd1 = .083
   c. SITE CLASS - D
   d. DESIGN CATEGORY - B
   e. BASIC FORCE RESISTING SYSTEM - BEARING WALL SYSTEM, LIGHT FRAMED WALLS WITH WOOD SHEAR PANELS
   f. RESPONSE MODIFICATION FACTOR - R = 6.5
   g. DESIGN BASE SHEAR - V = 0.018W
   h. ANALYSIS PROCEDURE - EQUIVALENT LATERAL FORCE, PROCEDURE

5. SNOW LOADS:
   a. GROUND SNOW LOAD - Pg = 5 PSF
   b. IMPORTANCE I - 1.00
   c. THERMAL FACTOR Ct - 1.0
   d. EXPOSURE Ce - 1.1
   e. RAIN ON SNOW - 5 PSF

C. GENERAL REQUIREMENTS:

1. SPECIFICATIONS ARE PART OF THE CONSTRUCTION DOCUMENTS AND MUST BE USED IN CONJUNCTION WITH THE DRAWINGS.
2. VERIFY EXISTING CONDITIONS AND DIMENSIONS PRIOR TO BEGINNING SHOP DRAWINGS OR FABRICATING MATERIALS. NOTIFY BRINKER CONSTRUCTION MANAGER OF DISCREPANCIES BEFORE PROCEEDING WITH ANY PHASE WORK. SUBMIT THE APPROPRIATE PLANS AND DETAILS REFLECTING THE FIELD VERIFIED EXISTING CONDITIONS FOR THE BRINKER CONSTRUCTION MANAGER'S USE.
3. VERIFY THE LOCATION OF CHASES, INSERTS, OPENINGS, SLEEVES, FINISHES, DEPRESSIONS, PADS, AND WALL OPENINGS.
4. DO NOT SCALE DRAWINGS FOR THE PURPOSE OF ESTABLISHING DIMENSIONS.
5. DETAILS LABELED "TYPICAL DETAILS" ON DRAWINGS APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. SUCH DETAILS APPLY WHETHER OR NOT DETAILS ARE REFERENCED AT EACH LOCATION. NOTIFY ENGINEER OF CONFLICTS REGARDING APPLICABILITY OF "TYPICAL DETAILS".
6. DO NOT LOAD THE SLAB ON GRADE OR SUPPORTED SLAB WITH ERECTION CRANES OR ERECTION EQUIPMENT. THE SLABS HAVE NOT BEEN DESIGNED FOR CRANE LOADS AND WILL REQUIRE AN INCREASE IN THICKNESS AND/OR REINFORCEMENT. OBTAIN BRINKER CONSTRUCTION MANAGER APPROVAL ON PROPOSED CRANE SUPPORT PLAN FOR SLABS PRIOR TO COMMENCING WORK.
7. DO NOT STORE OR STACK CONSTRUCTION MATERIALS ON POURED OR ERECTED FLOORS/ROOFS IN EXCESS OF 80 PERCENT OF LIVE LOAD. GENERAL CONTRACTOR WILL ENSURE THAT ALL SUB-CONTRACTORS ARE INFORMED OF LOADING RESTRICTIONS. AVOID IMPACT WHEN PLACING MATERIALS ON POURED OR ERECTED FLOORS OR ROOF.
8. THE CONTRACT STRUCTURAL DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION. PROVIDE ALL MEASURES REQUIRED TO PROTECT THE STRUCTURE, WORKMEN, AND OTHER PERSONS DURING CONSTRUCTION; INCLUDING BRACING, SHORING FOR CONSTRUCTION EQUIPMENT, SHORING FOR THE BUILDING, FORMS AND SCAFFOLDING. SHORING OF RETAINING WALLS AND OTHER TEMPORARY SUPPORTS AS REQUIRED.
9. PRINCIPAL OPENINGS THROUGH THE FRAMING ARE SHOWN ON DRAWINGS. EXAMINE THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR THE REQUIRED OPENINGS AND PROVIDE FOR REQUIRED OPENINGS WHETHER SHOWN ON THE STRUCTURAL DRAWINGS OR NOT, VERIFY SIZE AND LOCATION OF OPENINGS WITH THE MECHANICAL CONTRACTOR. DEVIATIONS FROM THE OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE APPROVED PRIOR TO IMPLEMENTING THE CHANGES.
10. THE STRUCTURAL ENGINEER'S REVIEW OF THE SHOP DRAWINGS SHALL NOT RELIEVE THE CONTRACTOR FROM RESPONSIBILITY FOR DEVIATIONS FROM THE CONTRACT DOCUMENTS, NOR FROM ANY ERRORS IN THE SHOP DRAWINGS.

SPECIAL INSPECTIONS NOTES

A. THE OWNER WILL EMPLOY THE SERVICES OF ONE OR MORE SPECIAL INSPECTORS TO PROVIDE SPECIAL INSPECTIONS DURING CONSTRUCTION FOR THE ITEMS IN THE SPECIAL INSPECTION TABLE BELOW IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE, 2012 EDITION. NOT APPLICABLE TO SLABS ON GROUND OR SITE CONCRETE FULLY SUPPORTED ON EARTH.

B. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.

C. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:

1. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS. THE INSPECTOR MAY NOT ALTER, MODIFY, ENLARGE OR WAIVE ANY OF THE REQUIREMENTS OF THE DOCUMENTS.
2. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE PROFESSIONAL OF RECORD, AND THE CONTRACTOR. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION.
3. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.

D. OBSERVATION BY STRUCTURAL ENGINEER OF RECORD IS NOT REQUIRED. IF STRUCTURAL OBSERVATION IS REQUIRED BY THE BUILDING AUTHORITY IT SHALL BE AS DIRECTED BY THE OWNER TO COMPLY WITH THE REQUIREMENTS OF THE GOVERNING BUILDING CODE.

E. THE FABRICATION OF TRUSSES AND OTHER ASSEMBLAGES CONSTRUCTED USING WOOD AND METAL MEMBERS, OR USING LIGHT METAL PLATE CONNECTORS, SHALL BE CONTINUOUSLY INSPECTED BY A QUALIFIED INSPECTOR APPROVED BY THE ENFORCEMENT AGENCY AND PAID FOR BY THE CONTRACTOR. THE INSPECTO SHALL FURNISH THE ARCHITECT, STRUCTURAL ENGINEER AND THE ENFORCEMENT AGENCY WITH A REPORT THAT THE LUMBER SPECIES, GRADES AND MOISTURE CONTENT, TYPE OF GLUE, TEMPERATURE AND GLUING PROCESS, TYPE OF METAL MEMBERS AND METAL CONNECTORS, AND THE WORKMANSHIP, CONFORM IN EVERY MATERIAL RESPECT WITH THE DULY APPROVED PLANS AND SPECIFICATIONS. EACH INSPECTED TRUSS SHALL BE STAMPED BY THE INSPECTOR WITH AN IDENTIFYING MARK.

F. INSPECTION OF FABRICATORS IS NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED BY THE BUILDING OFFICAL TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED PLANS AND SPECIFICATIONS TO THE BUILDING OFFICIAL AND TO THE ARCHITECT OF RECORD.

SPECIAL INSPECTION	FREQUENCY
SOILS:	
1. GRADING, EXCAVATION, AND FILLING: VERIFY CONFORMANCE TO RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEERING REPORT.	PERIODIC
CONCRETE FOUNDATIONS: (NOT APPLICABLE TO SLABS ON GROUND)	
1. PROVIDE INSPECTIONS DURING THE TAKING OF TEST SPECIMENS AND PLACING OF REINFORCED CONCRETE.	CONTINUOUS
2. BOLTS INSTALLED IN CONCRETE: PROVIDE INSPECTIONS DURING PLACEMENT OF CONCRETE AROUND BOLTS.	CONTINUOUS
3. EPOXY SET ANCHORS AND EXPANSION ANCHORS INSTALLED IN HARDENED CONCRETE.	CONTINUOUS
4. REINFORCING STEEL: VERIFY PLACEMENT AND CONFORMANCE TO CONSTRUCTION DOCUMENTS PRIOR TO THE CLOSING OF THE FORMS AND THE DELIVERY OF CONCRETE TO THE JOBSITE.	PERIODIC
5. WELDING OF REINFORCING STEEL (ASTM A 706)	
A. #6 BARS AND LARGER	CONTINUOUS
B. #5 BARS AND SMALLER, PROVIDED THE MATERIALS, QUALIFICATIONS OF WELDING PROCEDURES AND WELDERS AND VERIFIED PRIOR TO THE START OF THE WORK AND VISUAL INSPECTION OF ALL WELDS IS MADE PRIOR TO COMPLETION	PERIODIC
STEEL:	
1. HIGH-STRENGTH BOLTING (A325 AND A490 BOLTS)	
A. WHILE THE WORK IS IN PROGRESS, DETERMINE THAT THE REQUIREMENTS FOR BOLTS, NUTS, WASHERS, AND PAINT; BOLTED PARTS; AND INSTALLATION AND TIGHTENING ARE IN ACCORDANCE WITH APPROVED NATIONALLY RECOGNIZED STANDARDS	PERIODIC
B. MONITOR THE INSTALLATION OF BOLTS TO DETERMINE THAT ALL PILES OF CONNECTED MATERIALS HAVE BEEN DRAWN TOGETHER AND THAT THE SELECTED PROCEDURE IS PROPERLY USED TO TIGHTEN ALL BOLTS.	CONTINUOUS
STRUCTURAL WELDING:	
1. FOR WELDS WHERE THE PROVIDED MATERIALS, QUALIFICATIONS OF WELDING PROCEDURES AND WELDERS ARE VERIFIED IN ADVANCE; AND A VISUAL INSPECTION OF ALL WELDS IS MADE PRIOR TO COMPLETION	
A. SINGLE PASS FILLET WELDS NOT EXCEEDING 5/16 INCH IN SIZE	PERIODIC
B. WELDED SHEET STEEL FOR COLD-FORMED STEEL FRAMING MEMBERS SUCH AS STUDS AND JOISTS.	PERIODIC
REINFORCED MASONRY:	
1. AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:	
A. PROPORTIONS OF SITE-PREPARED MORTAR	PERIODIC
B. CONSTRUCTION OF MORTAR JOINTS	PERIODIC
C. LOCATION OF REINFORCEMENT	PERIODIC

FOUNDATION NOTES

A. REFER TO THE GEOTECHNICAL REPORT AND SPECIFICATIONS FOR GENERAL REQUIREMENTS OF EARTHWORK, OVEREXCAVATION, SUBGRADE PREPARATION, FILL AND COMPACTION, WATERPROOFING AND OTHER PERTINENT REQUIREMENTS AND INFORMATION.

B. FOUNDATION DESIGN IS BASED UPON SOILS EXPLORATION AND REPORT BY TERRACON CONSULTANTS, INC., REPORT ENTITLED "IHOP RESTAURANT ********", DATED DECEMBER 11, 2016 PROJECT NO. ********.

C. FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE NET BEARING PRESSURE OF 2,500 PSF, FOR INDIVIDUAL SPREAD FOOTINGS AND 2,000 PSF FOR CONTINUOUS FOOTINGS, FOUNDED AT A MINIMUM OF EIGHTEEN (18") INCHES BELOW FINAL GRADE.

D. PROTECT PIPES AND CONDUITS RUNNING THROUGH WALLS AND SLABS WITH 1/2 INCH EXPANSION MATERIAL. LOWER CONTINUOUS FOOTINGS AND GRADE BEAMS PERPENDICULAR TO PIPE RUNS TO ALLOW PIPES TO PASS ABOVE THE FOOTINGS OR THROUGH THE GRADE BEAMS. ALTERNATIVELY, PROVIDE A CONCRETE JACKET IF PIPES ARE LOW ENOUGH TO BE PLACED BELOW THE FOOTINGS AND GRADE BEAMS. LOWER FOOTINGS AND GRADE BEAMS PARALLEL TO PIPE RUNS TO AVOID SURCHARGE ONTO ADJACENT TRENCH EXCAVATIONS.

E. CONSTRUCTION MONITORING AND TESTING OF SUBGRADES FOR FILL, FOUNDATION, FLOOR SLAB AND PAVEMENT, FILL SELECTION, PLACEMENT AND COMPACTION SHALL BE PERFORMED BY AN EXPERIENCED SOILS ENGINEER AND/OR HIS REPRESENTATIVES.

F. ALL COMPACTION FILL, SUBGRADES, AND GRADES SHALL BE (A) UNDERLAIN BY SUITABLE BEARING MATERIAL, (B) FREE OF ALL ORGANIC, FROZEN, OR OTHER DELETERIOUS MATERIAL, AND (C) OBSERVED, TESTED AND APPROVED BY QUALIFIED ENGINEERING PERSONNEL REPRESENTING AN EXPERIENCED SOILS ENGINEER. PREPARATION OF SUBGRADES AFTER STRIPPING VEGETATION, ORGANIC, OR OTHER UNSUITABLE MATERIALS SHALL CONSIST OF (A) PROOFROLLING TO DETECT SOIL, WET, YIELDING SOILS OR OTHER UNSTABLE MATERIALS THAT MUST BE UNDERCUT, (B) SCARIFYING THE TOP EIGHT INCHES, (C) MOISTURE CONDITIONING THE SOILS AS REQUIRED, AND (D) RECOMPACTION TO SAME MINIMUM IN-SITU DENSITY REQUIRED FOR SIMILAR MATERIALS INDICATED UNDER ITEM H.

G. IN OVEREXCAVATION AND FILL AREAS, THE COMPACTED FILL MUST EXTEND (A) A MINIMUM OF FIVE FEET LATERAL DISTANCE BEYOND THE EXTERIOR EDGE OF THE FOUNDATION AT BEARING GRADE OR PAVEMENT AT SUBGRADE AND DOWN TO COMPACTED FILL SUBGRADE ON A MAXIMUM 0.5 (H): 1 (V) SLOPE, (B) ONE FOOT ABOVE GRADE BEAM GRADE OUTSIDE THE BUILDING, AND (C) TO FLOOR SUBGRADE INSIDE THE BUILDING. FILL SHALL BE PLACED AND COMPACTED ON A 3 (H):1 (V) SLOPE OR MUST BE STEPPED OR BENCHED AS REQUIRED TO FLATTEN IF NOT SPECIFICALLY APPROVED BY QUALIFIED PERSONNEL UNDER THE DIRECTION OF AN EXPERIENCED SOIL ENGINEER.

H. THE COMPACTED FILL MATERIALS SHALL BE FREE OF DELETERIOUS, ORGANIC, OR FROZEN MATTER, SHALL CONTAIN NO CHEMICALS THAT MAY RESULT IN THE MATERIAL BEING CLASSIFIED AS 'CONTAMINATED', AND SHALL BE LOW-EXPANSIVE WITH A MAXIMUM LIQUID-LIMIT (ASTM D-423) AND PLASTICITY INDEX (ASTM D-424) OF 35 AND 15, RESPECTIVELY. THE TOP 12 INCHES OF COMPACTED FILL SHOULD HAVE A MAXIMUM THREE-INCH-PARTICLE DIAMETER AND ALL UNDERLYING COMPACTED FILL A MAXIMUM SIX-INCH DIAMETER UNLESS SPECIFICALLY APPROVED UNDER THE DIRECTION OF AN EXPERIENCED SOILS ENGINEER PRIOR TO PLACEMENT.

I. THE DENSITY OF THE STRUCTURAL COMPACTED FILL AND SCARIFIED SUBGRADE AND GRADES SHALL NOT BE LESS THAN 95 PERCENT OF THE MAXIMUM DRY DENSITY AS DETERMINED BY STANDARD PROCTOR (ASTM D-698) WITH THE EXCEPTION OF THE TOP 12 INCHES OF PAVEMENT SUBGRADE WHICH SHALL HAVE A MINIMUM IN-SITU DENSITY OF 100 PERCENT OF MAXIMUM DRY DENSITY, OR FIVE PERCENT HIGHER THAN UNDERLYING FILL MATERIALS. THE MOISTURE CONTENT OF COHESIVE SOIL SHALL NOT VARY BY MORE THAN -2 TO +2 PERCENT OF THE OPTIMUM WHEN PLACED AND COMPACTED OR RECOMPACTED, UNLESS SPECIFICALLY RECOMMENDED/APPROVED BY THE SOILS ENGINEER MONITORING THE PLACEMENT AND COMPACTION. COHESIVE SOILS WITH MODERATE TO HIGH EXPANSIVE POTENTIALS (PI>15) SHOULD, HOWEVER, BE PLACED, COMPACTED AND MAINTAINED PRIOR TO CONSTRUCTION AT A MOISTURE CONTENT OF 3+/-2 PERCENT ABOVE OPTIMUM MOISTURE CONTENT TO LIMIT FUTURE HEAVE. THE FILL SHALL BE PLACED IN LAYERS WITH A MAXIMUM LOOSE THICKNESS OF EIGHT INCHES. THE COMPACTION EQUIPMENT SHOULD CONSIST OF SUITABLE MECHANICAL EQUIPMENT SPECIFICALLY DESIGNED FOR SOIL COMPACTION. BULLDOZERS OR SIMILAR TRACKED VEHICLES ARE TYPICALLY NOT SUITABLE FOR COMPACTION.

J. EXCAVATION, FILLING, SUBGRADE AND GRADE PREPARATION SHALL BE PERFORMED IN A MANNER AND SEQUENCE THAT WILL PROVIDE DRAINAGE AT ALL TIMES AND PROPER CONTROL OF EROSION. PRECIPITATION, SPRINGS, AND SEEPAGE WATER ENCOUNTERED SHALL BE PUMPED OR DRAINED TO PROVIDE A SUITABLE WORKING PLATFORM. SPRINGS OR WATER SEEPAGE ENCOUNTERED DURING GRADING/FOUNDATION CONSTRUCTION MUST BE CALLED TO THE SOIL ENGINEER'S ATTENTION IMMEDIATELY FOR POSSIBLE CONSTRUCTION PROCEDURE REVISION OR INCLUSION OF AN UNDERDRAIN SYSTEM.

K. WHEREVER, IN THE OPINION OF THE SOILS ENGINEER OR THE OWNER'S REPRESENTATIVES, AN UNSTABLE CONDITION IS BEING CREATED EITHER BY CUTTING OR FILLING, THE WORK SHALL NOT PROCEED INTO THAT AREA UNTIL AN APPROPRIATE GEOTECHNICAL EXPLORATION AND ANALYSIS HAS BEEN PERFORMED AND THE GRADING PLAN REVISED, IF FOUND NECESSARY.

SLAB-ON-GRADE NOTES

A. SLAB ON GRADE SHALL BE FOUR INCHES THICK, REINFORCED WITH #3 REBARS AT 12 INCHES ON CENTER EACH WAY UNLESS OTHERWISE NOTED ON THE DRAWINGS. REBAR SHALL BE PLACED AT MID-HEIGHT IN THE SLAB.

B. THE SLAB SHALL BE UNDERLAIN BY A MINIMUM TWENTY-FOUR (24") INCHES OF SELECT FILL OVER SUBGRADE. A MINIMUM 15 MIL STEGO WRAP (CLASS A) VAPOR BARRIER, MEETING THE REQUIREMENTS OF ASTM E 154 AND E 1745, SHALL BE PLACED BELOW THE FLOOR SLAB TO PROTECT MOISTURE-SENSITIVE FLOOR COVERINGS (I.E. TILE, OR CARPET, ETC.). THE SHEETS OF THE VAPOR RETARDER MATERIAL SHALL BE EVALUATED FOR HOLES AND/OR PUNCTURES PRIOR TO PLACEMENT AND THE EDGES OVERLAPPED AND SEALED WITH "STEGO WRAP" RED POLYETHYLENE TAPE OR "STEGO" MASTIC.

C. PROVIDE CONTROL JOINTS AND OPTIONAL CONSTRUCTION JOINTS ON MAJOR GRID LINES PER DETAIL 1/S102.

REINFORCING STEEL NOTES

A. PROVIDE DETAILING, FABRICATION, AND INSTALLATION OF REINFORCING AND ACCESSORIES IN ACCORDANCE WITH ACI 315 AND ACI 318.

B. PROVIDE NEW BILLET STEEL REINFORCING BARS IN ACCORDANCE WITH ASTM A 615, GRADE 60.

C. PROVIDE ANCHOR BOLTS CONFORMING TO THE STANDARDS OF ASTM F1554 OR 36 UNLESS OTHERWISE NOTED.

D. COORDINATE PLACEMENT OF CAST-IN-PLACE EMBEDMENTS AND ANCHOR BOLTS. SET ANCHOR BOLTS WITH A TEMPLATE. SECURELY ATTACH EMBEDDED ITEMS TO FORMWORK OR REINFORCING.

E. PROVIDE CLASS "B" REINFORCEMENT SPLICES (60 BAR DIAMETERS) FOR CONTINUOUS REINFORCEMENT. PROVIDE STANDARD 90-DEGREE HOOKS IN ACCORDANCE WITH ACI 318, UNLESS OTHERWISE NOTED. STAGGER SPLICES UNLESS SPECIFICALLY NOTED.

F. MAINTAIN THE FOLLOWING CONCRETE COVERAGE FOR REINFORCING STEEL UNLESS OTHERWISE NOTED:

1.	CONCRETE CAST AGAINST EARTH:	3"
2.	CONCRETE EXPOSED TO WEATHER:	
	a. NO. 6 AND LARGER	2"
	b. NO. 5 AND SMALLER	1 1/2"

G. DO NOT WELD OR BEND REINFORCEMENT IN THE FIELD UNLESS SPECIFICALLY SHOWN OR APPROVED BY STRUCTURAL ENGINEER.

H. WHEN SPECIFICALLY APPROVED, PROVIDE WELDED REINFORCEMENT ACCORDANCE WITH ASTM A 706 GRADE 60. USE LOW HYDROGEN ELECTRODES FOR WELDING OF REINFORCEMENT IN CONFORMANCE WITH "RECOMMENDED PRACTICES FOR WELDING REINFORCING STEEL", AMERICAN WELDING SOCIETY, AWS D12.1.

I. WHERE REQUIRED, PROVIDE DOWELS TO MATCH SIZE AND SPACING OF MAIN REINFORCING.

J. PROVIDE CONTINUOUS HORIZONTAL WALL/GRADE BEAM REINFORCEMENT WITH 90-DEGREE BENDS AND EXTENSIONS AT CORNERS AND INTERSECTIONS AS SHOWN ON TYPICAL BAR PLACING DETAILS.

K. PROVIDE BAR SUPPORT ACCESSORIES IN ACCORDANCE WITH THE LATEST ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES. SUPPORT BEAM REINFORCING ON BEAM BOLSTERS SPACED NOT MORE THAN 4 FEET ON CENTER.

L. PROVIDE BAR SUPPORTS WITH PLASTIC COATED LEGS OR HOT DIP GALVANIZED AFTER FABRICATION FOR CONCRETE EXPOSED TO VIEW. PROVIDE STAINLESS STEEL BAR SUPPORTS FOR CONCRETE TO RECEIVE A SANDBLAST FINISH.

CONCRETE NOTES

A. PROVIDE DIFFERENT CLASSES OF CONCRETE AS SHOWN BELOW. PROVIDE BATCH MIXING, TRANSPORTATION, PLACING AND CURING OF CONCRETE IN ACCORDANCE WITH RECOMMENDATIONS OF ACI 301 AND ACI 318. USE TYPE I PORTLAND CEMENT (ASTM C150) UNLESS OTHERWISE NOTED. PROVIDE ADMIXTURES AND SPECIAL REQUIREMENTS AS SPECIFIED.

1. NORMAL WEIGHT (150 PCF), F'c = 3,000 PSI CONCRETE AT 28 DAYS.
   a. ALL CONCRETE GRADE SUPPORTED SLABS AND GRADE BEAMS.
   b. FOOTINGS.

B. UNLESS SPECIFIED BELOW, CONCRETE MUST REACH THE FOLLOWING PERCENTAGES OF ITS 28 DAY COMPRESSIVE STRENGTH (F'c) BEFORE FORMS MAY BE REMOVED:

1. WALLS, COLUMNS AND BEAM SIDES. 40 PERCENT

C. PROVIDE CONCRETE MIXES DESIGNED BY A QUALIFIED TESTING LABORATORY FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER.

D. PROVIDE CONSTRUCTION AND CONTROL JOINTS AS INDICATED ON DRAWINGS. HORIZONTAL CONSTRUCTION JOINTS ARE NOT ALLOWED UNLESS SPECIFICALLY NOTED OR APPROVED BY STRUCTURAL ENGINEER. NOTIFY STRUCTURAL ENGINEER OF PROPOSED CONSTRUCTION JOINT OR CONTROL JOINT LOCATIONS WHICH ARE DIFFERENT OR IN ADDITION TO JOINTS INDICATED ON DRAWINGS. PROVIDE 6,000-SQUARE FOOT MAXIMUM AREA OF CONCRETE PLACEMENT IN THE SLAB BETWEEN CONSTRUCTION JOINTS. PROVIDE 75-FOOT MAXIMUM SPACING OF CONSTRUCTION JOINTS IN GRADE BEAMS. PROVIDE GRADE BEAM CONSTRUCTION JOINTS IN MIDDLE 1/3 OF THE SPAN WHEN A BEAM INTERSECTS A GIRDER WITHIN THE MIDDLE 1/3 OF THE GIRDERS SPAN, OFFSET THE JOINT IN THE GIRDER A DISTANCE EQUAL TO TWICE THE WIDTH OF THE BEAM.

E. CHAMFER EXPOSED EDGES 3/4 INCH UNLESS NOTED OTHERWISE.

F. WIRE BRUSH AND CLEAN CONSTRUCTION JOINTS PRIOR TO POURING NEW CONCRETE.

G. REFERENCE THE APPROPRIATE DISCIPLINE DRAWINGS FOR SUBSLAB PIPING, FLOOR DRAINS AND SLAB AND WALL PENETRATIONS.

H. FOR PIPE INSTALLED HORIZONTALLY WITHIN SLAB, PROVIDE MAXIMUM OUTSIDE DIAMETER OF 30 PERCENT OF THE SLAB THICKNESS. PLACE CONDUIT OR PIPE BETWEEN THE TOP AND BOTTOM LAYERS OF REINFORCEMENT WITHIN THE CENTER THIRD OF THE SLAB. DO NOT SPACE CONDUITS OR PIPE CLOSER THAN THREE DIAMETERS ON CENTER.

I. PLACE SPECIFIED CONTINUOUS WATERSTOPS IN ALL EXTERIOR BELOW GRADE CONSTRUCTION JOINTS AND ELSEWHERE AS SHOWN IN DRAWINGS.

STRUCTURAL STEEL NOTES

A. DESIGN, DETAIL AND ERECT STRUCTURAL STEEL ELEMENTS IN ACCORDANCE WITH THE LATEST VERSION OF THE FOLLOWING:

1. AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.
2. AISC MANUAL OF STEEL CONSTRUCTION, ALLOWABLE STRESS DESIGN.
3. AWS STRUCTURAL WELDING CODE, D1.1.

B. PROVIDE STRUCTURAL STEEL OF THE FOLLOWING ASTM DESIGNATIONS UNLESS NOTED OTHERWISE:

1. CHANNELS	ASTM A36
2. COLD FORMED STEEL TUBING (HSS)	ASTM A 500, GRADE B (46 KSI YIELD)
3. BASE PLATES, MISCELLANEOUS STEEL PLATES, AND RODS	ASTM A 36
4. HIGH STRENGTH BOLTS	ASTM A 325
5. HARDENED STEEL WASHERS	ASTM F 436

C. WELD MINIMUM SIZE AND STRENGTH

1. PROVIDE MINIMUM SIZE OF FILLET WELDS AS SPECIFIED IN TABLE J2.4 OF THE AISC MANUAL.
2. DEVELOP THE FULL TENSILE STRENGTH OF THE MEMBER ELEMENT JOINED, ON ALL SHOP AND FIELD WELDS, UNLESS NOTED OTHERWISE ON THE DRAWINGS.
3. WHERE CONNECTIONS ARE NOTED ON DRAWINGS AS MOMENT CONNECTIONS, PROVIDE WELDS TO DEVELOP FULL FLEXURAL CAPACITY OF THE LESSER MEMBER.
4. PROVIDE ELECTRODES FOR FIELD OR SHOP WELDING THAT CONFORM TO AWS D1.1 CLASS E70.

D. PROVIDE MINIMUM OF TWO BOLTS PER CONNECTION. MINIMUM BOLT DIAMETER TO BE 3/4-INCH.

E. PROVIDE BOLTS, NUTS AND WASHERS THAT ARE HOT DIP GALVANIZED ACCORDING TO ASTM A 153, CLASS C WHEN USED TO CONNECT STEEL ELEMENTS THAT ARE HOT DIP GALVANIZED AFTER FABRICATION.

F. STEEL FABRICATION

1. FABRICATE AND ASSEMBLE STRUCTURAL MEMBERS/ASSEMBLIES IN SHOP TO GREATEST EXTENT POSSIBLE.
2. SPLICING OF STRUCTURAL STEEL MEMBERS IS PROHIBITED WITHOUT PRIOR APPROVAL BY THE BRINKER CONSTRUCTION MANAGER.
3. FABRICATOR SHALL BE RESPONSIBLE FOR ALL ERRORS OF DETAILING ON THE SHOP DRAWINGS, ERRORS IN FABRICATION, AND THE CORRECT FITTING OF STRUCTURAL STEEL MEMBERS.

G. CONFORM TO THE AISC CODE OF STANDARD PRACTICE, FOR ERECTION TOLERANCES. FIELD MODIFICATION TO STRUCTURAL STEEL IS PROHIBITED WITHOUT PRIOR APPROVAL BY THE BRINKER CONSTRUCTION MANAGER.

H. CLEAN STEEL OF RUST, LOOSE MILL SCALE AND OTHER FOREIGN MATERIALS WHERE REQUIRED FOR FABRICATION, FITTING UP, OR WELDING.

I. DO NOT CUT STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT PRIOR REVIEW AND APPROVAL OF THE BRINKER CONSTRUCTION MANAGER.

J. HOT DIP GALVANIZE AFTER FABRICATION STRUCTURAL STEEL AND THEIR CONNECTIONS PERMANENTLY EXPOSED TO THE OUTSIDE. SUCH ITEMS INCLUDE BUT ARE NOT LIMITED TO:

1. CANOPY.
2. AWNINGS.

EXAMINE THE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR OTHER ITEMS THAT REQUIRE HOT DIPPED GALVANIZING.

K. PROVIDE GROUT FOR BASE PLATES THAT IS NON-SHRINK, NON-METALLIC GROUT WITH MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 5000 PSI.

L. FURNISH STEEL SHOP DRAWINGS FOR ARCHITECT'S AND STRUCTURAL ENGINEERS REVIEW PRIOR TO FABRICATION. INCLUDE WELDING PROCEDURES, TESTING PROGRAMS FOR WELDING AND HIGH STRENGTH BOLTING, COATING MATERIAL AND ERECTION SEQUENCE ON SHOP DRAWINGS.

M. MILL STEEL COLUMN ENDS TO FIT FLUSH WITH BASE PLATE, CAP PLATE AND END PLATES. FIELD ASSEMBLY OF THESE STEEL ELEMENTS TO THE COLUMNS IS PROHIBITED.

N. PROVIDE TEMPORARY SHORING OR BRACING DURING CONSTRUCTION PHASE, PRIOR TO COMPLETING CONNECTIONS.

O. ALL STEEL SHALL BE PAINTED WITH PRIMER.

STRUCTURAL WOOD NOTES

A. WOOD FRAMING:

1. ALL WOOD STRUCTURES HAVE BEEN DESIGNED IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
2. ALL DIMENSIONAL LUMBER SHALL BE #2 SOUTHERN YELLOW PINE, EXCEPT POSTS AND BEAMS WHICH SHALL BE #1 SOUTHERN YELLOW PINE STAMPED BY A MEMBER OF THE WWPA AND KILN DRIED TO 19% MAXIMUM MOISTURE CONTENT.
3. PRESSURE TREAT ALL WOOD PRODUCTS IN DIRECT CONTACT WITH THE FOUNDATION PER SPECIFICATION.
4. JOISTS, RAFTERS AND BEAMS SHALL NOT BE NOTCHED EXCEPT WHERE SHOWN IN DETAILS. OBTAIN STRUCTURAL ENGINEER'S APPROVAL FOR ANY HOLES THROUGH OR NOTCHES IN THE TOP OF HORIZONTAL MEMBERS.
5. SILL PLATES SHALL BE A MINIMUM 2 x (STUD WIDTH) TREATED S.P. SILL PLATES AT ALL SHEAR WALLS SHALL BE FASTENED TO THE FOUNDATION AS SCHEDULED. SILL PLATES AT EXTERIOR WALLS SHALL BE FASTENED TO THE FOUNDATION WITH 5/8" DIAMETER x 18" LONG ANCHOR BOLTS SPACED AT 48" ON CENTER. LOCATE ANCHOR BOLTS A MAXIMUM OF 9" FROM EACH END OF PLATE. FOR MISLOCATED BOLTS, 5/8" DIAMETER SIMPSON TITEN HD SCREW ANCHORS MAY BE USED AS A REPAIR ALTERNATE AT NON-SHEARWALLS ONLY. SILL PLATES AT INTERIOR LOAD BEARING WALLS SHALL BE FASTENED TO THE FOUNDATION WITH ONE 5/8" DIAMETER x 18" LONG ANCHOR BOLT AT 48" ON CENTER. SILL PLATES AT NON-LOAD BEARING INTERIOR WALLS SHALL BE FASTENED TO THE FOUNDATION WITH ONE SIMPSON PDPWL POWDER ACTUATED FASTENER WITH A MINIMUM EMBEDMENT OF 1 ¼" AT 16" ON CENTER.
6. ALL HARDWARE USED IN THE WORK SHALL BE SIMPSON STRONG-TIE, KC METALS, SILVER OR OTHER HARDWARE OF EQUAL CAPACITY AND HAVING ICBO APPROVAL.
7. A DOUBLE PLATE MADE OF TWO MEMBERS OF THE SAME WIDTH AS THE STUDS SHALL BE PLACED AT THE TOP OF EVERY BEARING PARTITION OR EXTERIOR WALL. SUCH DOUBLE PLATES SHALL BE LAPPED AT CORNER, AND JOINTS IN UPPER AND LOWER MEMBERS SHALL BE AT LEAST 4 FEET APART, EXCEPT AT CORNERS.
8. EXTERIOR WALLS AND BEARING WALLS SHALL BE FRAMED, UNLESS NOTES OTHERWISE, AS FOLLOWS:

MEMBER SIZE AND SPACING	LOADING
a. 2x6 STUDS AT 16" ON CENTER W/ BLOCKING 8'-0" O.C. MIN.	ROOF

9. ALL NAILING SHALL BE DONE IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF THE LOCAL BUILDING CODE, EXCEPT AS OTHERWISE SHOWN, AND SHALL BE DONE WITH COMMON WIRE NAILS UNLESS ANOTHER TYPE OF NAIL IS ALLOWED BY THE LOCAL BUILDING CODE. (RE: NAILING SCHEDULE)
10. BOLTS SHALL BE UNFINISHED BOLTS CONFORMING TO ASTM A307. BOLTS HOLES THROUGH THE WOOD SHALL BE DRILLED NO MORE THAN 1/16" GREATER THAN BOLT DIAMETER.
11. PROVIDE ALL LOAD BEARING HEADERS AS SHOWN ON THE FRAMING PLANS. IF NO HEADER IS MARKED, PROVIDE DOUBLE JOIST HEADERS ABOVE ENTRY OPENING IN ACCORDANCE WITH THE FOLLOWING SCHEDULE:

OPENING WIDTH	HEADER SIZE
UP TO 2'-6" WIDE	TRIPLE 2x6
2'-6 TO 3'-8" WIDE	TRIPLE 2x8
3'-8" TO 6'-0" WIDE	TRIPLE 2x10

HEADERS FOR LOAD BEARING OPENINGS LARGER THAN 6'-0" WIDE SHALL BE ENGINEERED

12. PROVIDE A MINIMUM (3) 2x STUDS AT EACH END OF LVL BEAMS. PROVIDE A MINIMUM OF (2) 2x STUDS MEMBERS UNDER ALL BEAMS AND HEADERS. PROVIDE A MINIMUM OF DOUBLE STUDS ON EACH SIDE OF ALL OPENINGS, BUILT-UP STUD COLUMNS SHALL BE CONTINUOUS TO THE FOUNDATION AND SHALL BE NAILED TOGETHER WITH 16d NAILS AT 18" O.C. FOR THE FULL STUD HEIGHT.

B. PLYWOOD:

1. PLYWOOD SHALL BE EXTERIOR TYPE 1 WITH EXTERIOR GLUE CONFORMING TO PS-1 OF GRADE AS SPECIFIED, LAID WITH FACE GRAIN PERPENDICULAR TO SUPPORTS. USE COMMON NAILS ONLY UNLESS OTHERWISE APPROVED BY THE STRUCTURAL ENGINEER AND LOCAL BUILDING OFFICIAL. ALL PLYWOOD SHALL BE 5 PLY MATERIAL.
   a. ROOF SHEATHING:
      i. ¾" CDX PLYWOOD SHEATHING (FLAT ROOF), P II 48/24 OR BETTER, WITH 10d NAILS (COMMON) AT 6" CENTER TO CENTER AT SUPPORT EDGES, AND AT 12" CENTER TO CENTER AT FIELD UNBLOCKED, UNLESS NOTED OTHERWISE.
   b. WALL SHEATHING:
      i. ½" (STRUCTURAL 1) "ZIP PANEL" SHEATHING (32/16 SPAN RATING) ATTACHED TO FRAMING WITH 8d NAILS (COMMON) AT 6" CENTER TO CENTER AT PANEL EDGES AND AT 12" CENTER TO CENTER AT FIELD.
   c. SHEAR WALLS:
      i. REFER TO SHEAR WALL SCHEDULE.
2. ALL PLYWOOD SHEATHING AT WALLS AND ROOF SHALL BE INSPECTED AND APPROVED BY BUILDING INSPECTOR PRIOR TO COVERING WITH FLOORING OR ROOFING MATERIAL.

C. REDBUILT LAMINATED VENEER LUMBER (REDLAM LVL):

1. ALL LAMINATED VENEER LUMBER SHALL BE DESIGNED AND MANUFACTURED TO THE STANDARDS SET FORTH IN THE NER-481 REPORT.
2. ALLOWABLE UNIT STRESSES REQUIRED FOR DRY CONDITIONS OF USE FOR LAMINATED VENEER LUMBER SHALL BE AS FOLLOWS:
   a. BENDING....................2,900 PSI
   b. COMPRESSION PARALLEL TO GRAIN..........2,635 PSI
   c. HORIZONTAL SHEAR..........285 PSI
   d. COMPRESSION PERPENDICULAR TO GRAIN..........750 PSI
   e. MODULUS OF ELASTICITY.......... 2,000,000 PSI
3. LAMINATED VENEER LUMBER MEMBER SIZES SHOWN ARE NET; OTHER MEMBER SIZES ARE NOMINAL.

MANUFACTURED TRUSSES: (DEFERRED SUBMITTAL)

A. MANUFACTURED OPEN-WEB PIN CONNECTED TRUSSES SHALL BE BY REDBUILT, LLC **(NO SUBSTITUTIONS ARE ALLOWED).**

B. TRUSSES SHALL BE DESIGNED FOR THE DEAD LOADS OF THE MATERIALS AND MECHANICAL EQUIPMENT SHOWN ON THE DRAWINGS PLUS A LIVE LOAD AS INDICATED IN THESE NOTES.

C. MANUFACTURER SHALL FURNISH SHOP DRAWINGS AND CALCULATIONS SIGNED BY A LICENSED ENGINEER IN THE STATE THAT THIS PROJECT WILL BE CONSTRUCTED IN AND SHALL BE RESPONSIBLE FOR ALL REQUIREMENTS OF THE BUILDING DEPARTMENT.

D. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND THE ENGINEER FOR APPROVAL PRIOR TO FABRICATION.

E. SHOP DRAWINGS INCLUDING LATERAL BRACING DETAILS SHALL BE AVAILABLE ON THE JOB SITE DURING TIMES OF INSPECTIONS, AND SHALL BEAR CLEAR INDICATION THAT THEY HAVE BEEN REVIEWED BY THE ENGINEER OF RECORD.

F. ALL HARDWARE REQUIRED FOR CONNECTIONS BETWEEN TRUSSES SHALL BE DESIGNED AND SPECIFIED BY THE REGISTERED TRUSS ENGINEER OF RECORD.

G. TRUSS SHOP DRAWINGS MUST BE APPROVED WITHIN 30 DAYS OF PERMIT ISSUANCE. NO INSPECTIONS WILL BE PERFORMED IF TRUSS DRAWINGS ARE NOT APPROVED AFTER 30 DAYS.

H. THIRTY-DAY TRUSS SUBMITTAL POLICY WILL REMAIN IN EFFECT UNTIL TRUSSES ARE APPROVED.

DESIGN CRITERIA:

1. LOADING:

MAIN ROOF TOP CHORD LIVE LOAD:	20 psf
MAIN ROOF TOP CHORD DEAD LOAD	10 psf
ROOF BOTTOM CHORD DEAD LOAD:	10 psf
NET WIND UPLIFT:	12.0 psf WITHIN 5'-0" OF BUILDING PERIMETER

2. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/240.
3. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.
4. DESIGN FOR PARAPET KICKER AND TOWER WALL LOADS PER DETAILS SHOWN ON SHEETS S202 AND S203.

TYPICAL LUMBER NAILING SCHEDULE

NAILING SHOWN IS TYPICAL EXCEPT AS NOTED ON PLANS. USE COMMON NAILS.

1. JOIST TO SILL OR GIRDER, TOENAILS	3-8d
2. BRIDGING TO JOIST, TOE NAIL EACH END	2-8d
3. 1"x6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	2-8d
4. WIDER THAN 1"x6" SUBFLOOR TO EACH JOIST, FACE NAIL	3-8d
5. 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	2-16d
6. SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL 16"O.C.	16d AT
7. TOP PLATE TO STUD, END NAIL	2-16d
8. STUD TO SOLE PLATE NAILS OR 4-8d TOE NAILS	2-16d END
9. DOUBLE STUDS, FACE NAIL 12"O.C.	16d AT
10. DOUBLED TOP PLATES, FACE NAIL 16"O.C.	16d AT
11. TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	2-16d
12. CONTINUOUS HEADER, TWO PIECES 16"O.C. ALONG EA. EDGE	16d AT
13. CEILING JOISTS TO PLATE, TOE NAIL	3-8d
14. CONTINUOUS HEADER TO STUD, TOE NAIL	4-8d
15. CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	3-16d
16. CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	3-16d
17. RAFTER TO PLATE, TOENAIL	3-8d
18. 1" BRACE TO EACH STUD AND PLATE, FACE NAIL	2-8d
19. 1"x6" SHEATHING OR LESS TO EACH BEARING, FACE NAIL	2-8d
20. WIDER THAN 1"x8" SHEATHING TO EACH BEARING, FACE NAIL	3-8d
21. BUILT-UP CORNER STUDS 24"O.C.	16d AT
22. BUILT-UP GIRDER AND BEAMS 32"O.C. AT TOP & BOTTOM	20d AT
ENDS AND AT EA. SPLICE	2-20d AT
23. 2" PLANKS EACH BEARING	2-16d AT

EPOXY ANCHORED DOWELS AND BOLTS

A. EPOXY ANCHORS USED FOR FIELD REPAIRS OR SUBSTITUTIONS SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE.

B. INSTALL BOLTS AND DOWELS IN ACCORDANCE WITH CURRENT ICBO REPORT FOR THE BOLT, AND RECOMMENDATIONS OF THE MANUFACTURER. FOLLOW MANUFACTURER'S INSTRUCTIONS EXPLICITLY.

C. INSTALL BOLTS AND DOWELS PERPENDICULAR TO THE FACE OF CONCRETE, UNLESS OTHERWISE INDICATED IN THE DRAWINGS.

D. DIAMETER OF DRILLED HOLES SHALL BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT REDIRECT HOLES ALONG LENGTH OF BOLT.

E. DRILL HOLES IN CONTINUOUS OPERATION. BLOW DUST FROM THE HOLE USING COMPRESSED AIR.

F. FILL ABANDONED HOLES WITH EPOXY GROUT.

G. INSTALLATION OF EPOXY ANCHORED DOWELS AND BOLTS SHALL BE CONTINUOUSLY INSPECTED BY THE TESTING LABORATORY TO ENSURE THAT HOLES ARE OF THE PROPER DIAMETER AND LENGTH, ARE PROPERLY CLEANED, AND THAT BOLTS ARE INSTALLED CORRECTLY.

STRUCTURAL MASONRY NOTES

A. REINFORCED MASONRY WORK AND MATERIALS TO BE IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES A.C.I. 530/ ASCE 5/ TMS 402.

B. REINFORCED MASONRY TO CONFORM TO THE SPECIFICATIONS FOR MASONRY STRUCTURES ACI 530.1/ ASCE 6/ TMS 602 (WITH THE EXCEPTIONS NOTED IN JOB SPECIFICATIONS).

C. PROVIDE CONCRETE MASONRY UNITS (CMU) OF NORMAL WEIGHT (125 PCF), GRADE N, TYPE 1, 1900 PSI COMPRESSIVE STRENGTH ON NET AREA, CONFORMING TO THE LATEST EDITION OF ASTM C 90. LAY UNITS IN RUNNING BOND UNLESS OTHERWISE NOTED.

D. PROVIDE MASONRY MORTAR MEETING THE REQUIREMENTS OF TYPE S PER ASTM C 270. PROVIDE AGGREGATES FOR MORTAR CONFORMING TO ASTM C 144. PROVIDE MORTAR WITH A MINIMUM COMPRESSIVE STRENGTH OF 1,800 PSI AT 28 DAYS.

E. PROVIDE MASONRY GROUT WITH MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,500 PSI TESTED IN ACCORDANCE WITH ASTM C 1019. GROUT TO MEET THE MINIMUM REQUIREMENTS SPECIFIED IN ASTM C 476.

F. PROVIDE MASONRY ASSEMBLAGES WITH MINIMUM PRISM STRENGTH (F'm) OF 1,500 PSI, TESTED IN ACCORDANCE WITH ASTM C 140.

G. PROVIDE CONCRETE MASONRY UNITS IN ACCORDANCE ASTM C 426 LIMITS FOR DRYING SHRINKAGE OF CONCRETE BLOCKS.

H. PROVIDE VERTICAL REINFORCEMENT IN CMU WALLS AS SHOWN IN DRAWINGS. FILL THE REINFORCED CELLS SOLID WITH GROUT. MAXIMUM HEIGHT OF GROUT POURS TO BE AS PER ACI 530.1 TABLE 7.

I. PROVIDE HORIZONTAL JOINT REINFORCING IN WALLS AT 16" ON CENTER VERTICAL SPACING. JOINT REINFORCING TO BE HOT DIP GALVANIZED. 3/16" SIDE RODS WITH 9 GAGE CROSS TIES, TRUSS TYPE, CONFORMING TO A.S.T.M. A 82. SPACE JOINT REINFORCEMENT AT 8" ON CENTER IN THE PARAPETS. LAP SPLICE ENDS A MINIMUM OF 12" AT SPLICES.

J. LAY HOLLOW UNITS WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS. PROVIDE FULL MORTAR COVERAGE FOR WEBS WHEN ADJACENT TO GROUTED CELLS.

K. ALIGN VERTICAL CELLS TO BE FILLED WITH GROUT TO PROVIDE CONTINUOUS UNOBSTRUCTED VERTICAL CELLS. REMOVE OVERHANGING MORTAR OR OTHER OBSTRUCTION AND DEBRIS FROM THE INSIDES OF SUCH CELL WALLS. PROVIDE GROUT WITH 8 INCH SLUMP AND CONSOLIDATE BY MEANS OF HAND TAMPING TO ENSURE COMPLETE FILLING OF CELLS.

L. PROVIDE 8-INCH HIGH BOND BEAM (MINIMUM DEPTH UNLESS OTHERWISE NOTED) AT TOP OF ALL CMU WALLS. REINFORCE BOND BEAM AS INDICATED. IF BOND BEAMS AT INTERSECTING WALLS MEET AT DIFFERENT ELEVATIONS, EXTEND BOND BEAMS AROUND INTERSECTING CORNER TO FIRST INTERIOR REINFORCED CELL, BUT NOT LESS THAN 4 FEET. CONSTRUCT BOND BEAMS WITH PORTLAND CEMENT GROUT, 2,500 PSI MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS AND A MAXIMUM AGGREGATE SIZE OF 3/8".

M. PROVIDE MINIMUM 1 #5 VERTICAL REINFORCING AT CORNERS OF CMU WALLS. PROVIDE 2 #5 VERTICAL REINFORCING AT ENDS OF WALLS, EACH SIDE OF CONTROL JOINT, AND EACH SIDE OF WALL OPENINGS. PROVIDE MINIMUM 3/8" THROUGH-WALL CONTROL JOINTS AT 40'-0" MAXIMUM SPACING OR AT LOCATIONS DETAILED ON THE ARCHITECTURAL DRAWINGS.

N. INSTALL ANCHORS, ACCESSORIES, AND OTHER ITEMS TO BE BUILT IN AS WORK PROGRESSES.

O. LAP SPLICE VERTICAL REINFORCING MINIMUM OF 40 BAR DIAMETERS OR 24 INCHES, WHICHEVER IS GREATER.

P. PERFORM CUTTING AND FITTING OF MASONRY WITH MASONRY SAWS PROVIDING CUT FINISHED UNITS.

Q. GROUT CELLS AT OR BELOW FINISHED GRADE ARE TO BE GROUTED SOLID.

R. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL CORE, DO NOT SLOPE DOWEL MORE THAN ONE HORIZONTAL TO SIX VERTICAL.

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