1.0 BUILDING CODES USED FOR DESIGN

DESIGN IS IN COMPLIANCE WITH 2012 INTERNATIONAL BUILDING CODE AND GEORGIA STATE AMENDMENTS.

1.1 DESIGN LOADS

A. ROOF LIVE LOAD = 20 PSF
B. ROOF DEAD LOAD = 20 PSF

BASIC WIND SPEED = 115 MPH
WIND IMPORTANCE FACTOR, Iw, = 1.00
WIND EXPOSURE CATEGORY = B
OCCUPANCY CATEGORY = 2
INTERNAL PRESSURE COEFFICIENTS = +/- 0.18

COMPONENTS AND CLADDING NOT DESIGNED BY THE ENGINEER OF RECORD SHALL BE DESIGNED USING WIND PRESSURES IN ACCORDANCE WITH ASCE 7.

SEISMIC OCCUPANCY CATEGORY = 2
SEISMIC IMPORTANCE FACTOR = 1
SEISMIC SITE CLASS = C
MAPPED 0.2 SEC SPECTRAL RESPONSE ACCELERATION Ss = 0.20
MAPPED 1.0 SEC SPECTRAL RESPONSE ACCELERATION S1 = 0.09
DESIGN SPECTRAL RESPONSE ACCELERATION COEFFICIENT Sds = 0.21
DESIGN SPECTRAL RESPONSE ACCELERATION COEFFICIENT Sd1 = 0.15
SEISMIC DESIGN CATEGORY = C
BASIC SEISMIC FORCE RESISTING SYSTEM =
ORDINARY REINFORCED MASONRY SHEAR WALLS
RESPONSE MODIFICATION COEFFICIENT, R = 2 (TABLE 12.2-1, ASCE 7)
SEISMIC RESPONSE COEFFICIENT, Cs, = 0.11
SEISMIC DESIGN BASE SHEAR = 13,200 lb
SEISMIC ANALYSIS PROCEDURE - EQUIVALENT LATERAL FORCE PROCEDURE

1.2 NEW CONSTRUCTION

1. THE CONTRACTOR SHALL FOLLOW WRITTEN DIMENSIONS ONLY. DO NOT SCALE DRAWINGS.
2. ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY AT ANY SIMILAR CONDITION ELSEWHERE ON THE PROJECT, EXCEPT WHERE A DIFFERENT DETAIL OR SECTION IS SHOWN.
3. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. WHERE DISCREPANCIES OCCUR, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ARCHITECT OR ENGINEER PRIOR TO CONSTRUCTION.
4. THE STEEL FRAMING MEMBERS SHOWN RELY ON BUILDING COMPONENTS OTHER THAN STRUCTURAL STEEL FOR FINAL STRUCTURAL STABILITY (PREVIOUSLY REFERRED TO AS A "SELF-SUPPORTING STEEL FRAME BY THE AISC 'CODE OF STANDARD PRACTICE'). THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND PROVISION OF ANY AND ALL TEMPORARY BRACING AND SHORING AGAINST WIND, ERECTION AND ALL CONSTRUCTION LOADS UNTIL ALL ELEMENTS, MEMBERS AND CONNECTIONS (FLOORS, ROOFS, SHEAR WALLS, ETC.), AS SHOWN ON THE CONTRACT DOCUMENTS ARE COMPLETELY INSTALLED. THE STRUCTURAL MEMBERS SHOWN ON THE CONTRACT DOCUMENTS ARE DESIGNED FOR THE ANTICIPATED LOADS THAT THE STRUCTURE WILL BE SUBJECTED TO ONLY AFTER ALL STRUCTURAL ELEMENTS ARE IN PLACE AND FINAL CONNECTIONS ARE COMPLETE.
5. THE GENERAL CONTRACTOR SHALL VERIFY ALL OPENING SIZES, PAD SIZES AND LOCATIONS WITH THE RESPECTIVE SUB-CONTRACTORS.

1.4 MISCELLANEOUS

1. ALL ANCHOR BOLTS FOR MECHANICAL AND ELECTRICAL EQUIPMENT ARE FURNISHED AND LOCATED BY THE RESPECTIVE CONTRACTORS AND SET BY THE GENERAL CONTRACTOR EXCEPT WHERE OTHER CONTRACTORS FURNISH THEIR OWN CONCRETE PADS.
2. ALL PIPE SLEEVES ARE FURNISHED BY AND LOCATED BY THE MECHANICAL AND ELECTRICAL CONTRACTORS AND SET BY THE GENERAL CONTRACTOR.
3. THE GENERAL CONTRACTOR SHALL VERIFY ALL OPENING SIZES, PAD SIZES AND LOCATIONS WITH THE RESPECTIVE SUB-CONTRACTORS.
4. ALL CORE DRILLING SHALL BE DONE BY THE MECHANICAL AND ELECTRICAL CONTRACTORS FOR THEIR OWN WORK UNDER THE SUPERVISION OF THE GENERAL CONTRACTOR. NO REINFORCING SHALL BE CUT. VERIFY LOCATION OF REINFORCING BEFORE CORE DRILLING. THERE SHALL NOT BE ANY CORE DRILLING THROUGH BEAMS OR COLUMNS. MAXIMUM CORE HOLE THROUGH SLABS SHALL BE PIPE DIAMETER PLUS 1".

1.5 SHOP DRAWINGS

1. SHOP DRAWINGS, UNLESS OTHERWISE NOTED, SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.
2. PRIOR TO SUMITTAL, THE CONTRACTOR SHALL REVIEW THE SHOP DRAWINGS AND INDICATED ANY CORRECTIONS REQUIRED. THE CONTRACTOR SHALL STAMP AND SIGN THE DRAWINGS INDICATING THEY HAVE BEEN REVIEWED.
3. SHOP DRAWINGS SHALL BE FURNISHED FOR ALL STRUCTURAL COMPONENTS. ALL SUBMITTALS TO BE MINIMUM THREE (3) SETS OF PRINTS.
4. REVIEW OF SHOP DRAWINGS BY ENGINEER DOES NOT RELIEVE CONTRACTOR FROM COMPLIANCE WITH REQUIREMENTS OF THE DRAWINGS AND SPECIFICATIONS. REVIEW IS FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT OF THE PROJECT AND GENERAL COMPLIANCE WITH THE INFORMATION GIVEN IN THE CONTRACT DOCUMENTS. THE CONTRACTOR IS RESPONSIBLE FOR - CONFIRMING AND CORRELATING ALL QUANTITIES AND DIMENSIONS, FABRICATION PROCESSES AND TECHNIQUES OF CONSTRUCTION, COORDINATING HIS WORK WITH THAT OF OTHER TRADES, AND PERFORMING HIS WORK IN A SAFE AND SATISFACTORY MANNER.
5. SUBMIT COPIES OF THE GA PE STAMPED SHOP DRAWINGS, REVIEWED BY DPoR, TO GWINNETT COUNTY BUILDING PLAN REVIEW. NO FRAMING INSPECTIONS WILL BE PERFORMED UNTIL DRAWINGS LISTED ARE SUBMITTED.
   a. AWNINGS/CANOPIES

2.0 EXCAVATION AND EARTHWORK

1. THE SOILS AND FOUNDATION ENGINEERING REPORT IS FOR INFORMATIONAL PURPOSES ONLY AND SHALL NOT BE CONSIDERED PART OF THE CONTRACT DOCUMENTS.
2. THE FOUNDATION DESIGN IS BASED ON THE FOLLOWING NET ALLOWABLE BEARING PRESSURES -
   SPREAD FOOTINGS = 2,000 PSF
   WALL FOOTINGS = 2,000 PSF
3. ALL FOOTING EXCAVATIONS SHALL BE INSPECTED, PRIOR TO CONCRETE PLACEMENT, BY A SOILS ENGINEER TO VERIFY SUITABLE BEARING MATERIAL OF CAPACITY AS SPECIFIED.
4. NOTIFY THE OWNER'S REPRESENTATIVE WHEN ADDITIONAL EXCAVATION IS REQUIRED TO REACH SUITABLE BEARING MATERIAL.
5. THE SOILS ENGINEER SHALL CERTIFY THAT ALL FOUNDATIONS WERE PLACED ON MATERIAL WITH THE BEARING VALUE AS SPECIFIED.
6. WITHIN THE EXCAVATION AREA OF THE FOUNDATIONS, ALL VEGETATION, TOPSOIL, PREVIOUSLY PLACED FILL AND UNSUITABLE SOILS SHALL BE REMOVED. ALL FOOTINGS TO BEAR ON VIRGIN SOILS OR PROPERLY PLACED AND COMPACTED ENGINEERED FILL.

2.1 BACKFILLING

1. NO BACKFILLING AND COMPACTING OF EARTH SHALL BE PERMITTED AGAINST BASEMENT WALLS UNTIL SUPPORTING SLABS HAVE BEEN POURED AND HAVE REACHED 75% OF THEIR DESIGN STRENGTH OR UNLESS ADEQUATE BRACING SUBMITTED FOR REVIEW IS PROVIDED.
2. BOTH SIDES OF FOUNDATION WALLS FOR SLAB-ON-GRADE CONSTRUCTION SHALL BE BACKFILLED SIMULTANEOUSLY SO AS TO PREVENT OVER-TURNING OR LATERAL MOVEMENT OF WALLS.
3. ALL GRADE BEAMS SHALL BE ADEQUATELY BRACED TO PREVENT LATERAL MOVEMENT DURING BACKFILLING AND COMPACTION.
4. FOUNDATIONS SHALL BE BACKFILLED AS SOON AS POSSIBLE.

2.2 FOUNDATION / UNDERGROUND MECHANICAL COORDINATION

1. UNDERGROUND SEWER, WATER, GAS LINES, ETC., CROSSING CONTINUOUS WALL FOUNDATIONS SHALL NOT PASS THROUGH FOOTINGS. WHERE PIPE OCCURS ABOVE TOP OF FOOTING, SLEEVE THROUGH WALL. WHERE PIPE OCCURS IN FOOTING DEPTH, DROP TOP OF FOOTING AS REQUIRED SO THAT PIPE PASSES JUST ABOVE FOOTING. IF TOP OF PIPE IS LESS THAN 6" BELOW BOTTOM OF FOOTING, PROVIDE 1" COMPRESSIBLE FOAM INSULATION BELOW FOOTING FOR WIDTH OF PIPE.

3.0 CONCRETE

1. ALL CONCRETE WORK INCLUDING FORMING, REINFORCING, MIXING, PLACING AND CURING SHALL BE DONE IN ACCORDANCE WITH THE ACI MANUAL OF CONCRETE PRACTICE INCLUDING 'BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE', 318 AND 'SPECIFICATIONS FOR STRUCTURAL CONCRETE', ACI 301, LATEST EDITIONS.
2. ALL CONCRETE SHALL ATTAIN A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF f'c = 3,500 PSI
3. MIX DESIGNS SHALL BE IN ACCORDANCE WITH METHOD 1 OR METHOD 2 OF ACI 301-05.
4. ALL CONCRETE EXPOSED TO FREEZING / THAWING SHALL HAVE A MINIMUM ENTRAINMENT OF 4.5% TO 7.5%.
5. ALL CONCRETE SHALL HAVE A MAXIMUM WATER-CEMENT RATIO OF 0.50.
6. CONCRETE SHALL ARRIVE AT THE JOBSITE WITH A SLUMP OF 3" TO 5".
7. DEPOSIT CONCRETE AS NEAR AS PRACTICAL TO FINAL POSITION TO AVOID SEGREGATION DUE TO RE-HANDLING OR FLOWING. CONCRETE SHALL BE VIBRATED INTO FORMS WHILE PLACING, WITHOUT OVER-VIBRATING. REINFORCING SHALL HAVE MINIMUM CONCRETE COVER OF -
   CONCRETE EXPOSED TO SOIL = 3"
   CONCRETE EXPOSED TO AIR = 2"
9. SLAB-ON-GRADE SHALL BE PLACED IN CONTINUOUS STRIPS PER ACI RECOMMENDATIONS.
10. COORDINATE CONCRETE WORK WITH THAT OF OTHER TRADES TO ALLOW FOR SETTING OF SLEEVES, ACCESSORIES, ETC.
11. ALL ANCHOR BOLTS SHALL BE IN PLACE PRIOR TO POURING OF CONCRETE.
12. CONCRETE TEST REPORTS SHALL BE MADE AVAILABLE AT JOB SITE FOR REVIEW BY INSPECTOR.
13. DESIGN OF CONCRETE STRUCTURAL ELEMENTS INCLUDING WALLS, FORMED SLABS, BEAMS AND COLUMNS IS IN ACCORDANCE WITH ACI 318-05.

3.1 JOINTS IN CONCRETE

1. CONSTRUCTION AND/OR CONTROL JOINTS SHALL BE MADE AS DETAILED ON THE DRAWINGS. SAW-CUT CONTROL JOINTS SHALL BE INSTALLED WITHIN 12 HOURS OF CONCRETE PLACEMENT.
2. CONSTRUCTION AND/OR CONTROL JOINTS FOR SLAB-ON-GRADE CONSTRUCTION SHALL BE LOCATED ON COLUMN LINES.
3. MAXIMUM SPACING OF CONSTRUCTION AND/OR CONTROL JOINTS IN SLAB-ON-GRADE CONSTRUCTION SHALL BE AS SHOWN ON DRAWINGS.
4. CONSTRUCTION OR CONTRACTION JOINTS IN CONCRETE FOUNDATION WALLS SHALL BE SPACED NO FURTHER THAN 20 FEET APART.
5. CONSTRUCTION JOINTS FOR ELEVATED SLABS SHALL BE LOCATED AT THE CENTER OF THE SPAN. REINFORCEMENT SHALL BE CONTINUOUS ACROSS ALL CONSTRUCTION JOINTS.

3.2 CONCRETE REINFORCEMENT

1. THE REINFORCING STEEL CONTRACTOR SHALL FABRICATE ALL REINFORCEMENT AND FURNISH ACCESSORIES, CHAIRS, SPACER BARS AND SUPPORTS NECESSARY TO SECURE THE REINFORCEMENT UNLESS SHOWN OTHERWISE ON THE DRAWINGS.
2. REINFORCING STEEL SHALL BE ASTM A615, GRADE 60.
3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
4. CONCRETE REINFORCEMENT SHALL BE PLACED ACCORDING TO THE CRSI 'RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS'.
5. ALL REINFORCEMENT SPLICES SHALL BE LAPPED PER ACI REQUIREMENTS MINIMUM UNLESS NOTED OTHERWISE. PROVIDE CORNER BARS FOR ALL HORIZONTAL REINFORCEMENT AT CORNERS AND INTERSECTIONS.
6. TOP BARS SHALL BE HOOKED AT END SPANS.
7. REINFORCEMENT TO BE WELDED SHALL CONFORM TO THE REQUIREMENTS OF ASTM A706 AND THE WELDING SHALL BE IN ACCORDANCE WITH AWS D1.4, STRUCTURAL WELDING CODE - REINFORCING STEEL, BY THE AMERICAN WELDING SOCIETY.

4.0 MASONRY

1. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-90. COMPRESSIVE STRENGTH OF MASONRY SHALL BE DETERMINED BY THE UNIT STRENGTH METHOD. NET AREA COMPRESSIVE STRENGTH, f'm, SHALL BE 1,500 PSI.
2. MORTAR AND ITS INGREDIENTS AS DELIVERED TO THE MASON SHALL CONFORM TO ASTM C 270, TYPE S.
3. MASONRY CEMENT SHALL CONFORM TO ASTM C 91, TYPE S.
4. GROUT FOR REINFORCED AND NON-REINFORCED MASONRY SHALL CONFORM TO ASTM C 476. MINIMUM GROUT COMPRESSIVE STRENGTH SHALL BE 2,500 PSI.
5. MASONRY CONSTRUCTION SHALL CONFORM TO THE LATEST STANDARDS OF THE MASONRY STANDARDS JOINT COMMITTEE (ACI 530-11/ASCE 5-11/ TMS 402-11 AND ACI 530.1/ASCE 6-11/TMS 602-11.)
6. MASONRY BONDING SHALL BE RUNNING BOND UNLESS NOTED OTHERWISE.

5.0 STRUCTURAL STEEL

1. FABRICATION AND ERECTION OF STRUCTURAL STEEL MEMBERS IS TO BE IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES' AS INDICATED IN THE NINTH EDITION OF THE 'MANUAL OF STEEL CONSTRUCTION' FOR ALLOWABLE STRESS DESIGN UNLESS NOTED OTHERWISE.
2. ALL CONNECTIONS SHALL BE BOLTED OR WELDED AND SHALL BE DESIGNED FOR THE END REACTIONS INDICATED ON PLANS. IF REACTIONS ARE NOT INDICATED, CONNECTIONS SHALL BE DESIGNED TO DEVELOP 60% OF THE ALLOWABLE UNIFORM LOAD TABULATED IN THE NINTH EDITION OF THE AISC 'MANUAL OF STEEL CONSTRUCTION' FOR ALLOWABLE STRESS DESIGN UNLESS NOTED OTHERWISE. NUMBER OF BOLTS MUST SATISFY MINIMUM REQUIREMENTS AS FOLLOWS -
   (2) BOLTS PER CONNECTION FOR 8" AND 10" DEEP MEMBERS
   (3) BOLTS PER CONNECTION FOR 12" AND 14" DEEP MEMBERS
   (4) BOLTS PER CONNECTION FOR 16" AND 18" DEEP MEMBERS
   (5) BOLTS PER CONNECTION FOR 21" AND 24" DEEP MEMBERS
   (6) BOLTS PER CONNECTION FOR 27" AND DEEPER MEMBERS
3. ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS -
   WIDE FLANGE SHAPES, CHANNELS - ASTM A992
   ANGLES AND PLATES - ASTM A36
   PIPE SHAPES - ASTM A53, GRADE B OR ASTM A501
   TUBE SHAPES - ASTM A500, GRADE B
4. ALL FILLER METAL USED IN WELDING SHALL BE 70 KSI YIELD, LOW-HYDROGEN.
5. ALL WELDING SHALL BE BY CERTIFIED WELDERS AND SHALL CONFORM TO THE AWS AND MEET AISC MINIMUM REQUIREMENTS FOR WELD SIZE. ALL WELDED JOINTS SHALL CONFORM TO THE PROVISIONS OF AWS D1.1-10 STRUCTURAL WELDING CODE BY AMERICAN WELDING SOCIETY. ALL WORK SHALL BE PERFORMED BY FULLY QUALIFIED WELDERS IN THE TYPE OF CONSTRUCTION INVOLVED. WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.
6. BOLTS SHALL BE HIGH-STRENGTH A-325 BOLTS. CONNECTIONS SHALL CONFORM TO RCSC-2009, 'THE SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS'. CONNECTIONS ARE BEARING TYPE. BOLTS SHALL BE TIGHTENED TO 'SNUG-TIGHT' CONDITION.
7. DESIGN OF SPECIAL CONNECTIONS BETWEEN STEEL FRAMING COMPONENTS BY OTHER THAN THE PROJECT STRUCTURAL ENGINEER OF RECORD SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA INCLUDING, BY NOT LIMITED TO, BRACE END CONNECTIONS, MOMENT RESISTING CONNECTIONS, MODIFIED BEAM SEAT CONNECTIONS, AND MEMBER SPLICE CONNECTIONS.
8. STEEL COLUMN BASE PLATES SHALL BE SIZED AS SHOWN ON PLAN WITH 3/4" DIAMETER ANCHOR BOLTS (U.N.O.) AND NON-METALLIC, NON-SHRINK GROUT FOR UNIFORM BEARING.
9. UNLESS NOTED OTHERWISE, STRUCTURAL STEEL SUPPLIER IS TO FURNISH 4 3" X 3" X 1/4" SHOP WELDED ANGLE FRAMES AT ALL ROOF OPENINGS. VERIFY SIZE AND LOCATION WITH CONTRACTOR.
10. ALL PLATES USED IN BOLTED CONNECTIONS SHALL HAVE ROLLED OR GAS CUT EDGES.
11. ALL STRUCTURAL STEEL AND MISCELLANEOUS METALS SHALL BE PRIME PAINTED WITH ONE COAT OF FABRICATOR'S STANDARD RUST-INHIBITIVE PRIMER UNLESS INDICATED IN THE PROJECT SPECIFICATIONS. TOUCH UP ALL DISTURBED AREAS AFTER ERECTION. STEEL TO BE FIRE-PROOFED SHALL RECEIVE PAINT/FINISH PROCESS COMPATIBLE WITH FIRE-PROOFING.
12. ADJUSTABLE MASONRY TIES SHALL BE FURNISHED AT 16" O.C. VERTICALLY AND 24" HORIZONTALLY ON ALL STEEL MEMBERS ENCASED IN OR ADJACENT TO MASONRY WALLS, WHETHER OR NOT SUCH ANCHORS ARE SHOWN ON THE DRAWINGS. TIES SHALL BE CAPABLE OF TRANSMITTING FORCES PERPENDICULAR TO THE PLANE OF THE WALL.
13. SPLICES MEMBERS ONLY WHERE INDICATED AND ACCEPTED ON SHOP DRAWINGS.
14. DO NOT CORRECT FABRICATION ERRORS BY GAS-CUTTING WITHOUT PERMISSION OF ENGINEER. DO NOT FLAME CUT HOLES OR ENLARGE HOLES BY BURNING.

5.1 STEEL BAR JOIST AND GIRDERS

1. ALL STEEL JOIST AND GIRDERS SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS AND RECOMMENDED CODE OF STANDARD PRACTICE FOR STEEL JOISTS AND JOIST GIRDERS. JOIST FABRICATOR SHALL BE A MEMBER OF THE SJI.
2. BRIDGING FOR 'K' SERIES STEEL JOISTS SHALL BE CONTINUOUS STEEL ANGLE TOP AND BOTTOM OF JOISTS IN LENGTHS TO PERMIT LAPPING AT JOIST PANEL POINTS FOR WELDING. WELD BRIDGING TO CHORDS. SPACING AND SIZE OF BRIDGING SHALL BE IN ACCORDANCE WITH SJI RECOMMENDED CODE OF STANDARD PRACTICE. INSTALL BRIDGING IMMEDIATELY AFTER JOIST ERECTION, BEFORE ANY CONSTRUCTION LOADS ARE APPLIED AND BEFORE ANY WORKMEN ARE ALLOWED ON THE JOISTS.
3. PROVIDE BOTTOM CHORD CEILING EXTENSIONS AS SHOWN ON ARCHITECTURAL DRAWINGS OR AS NOTED OTHERWISE.
4. HEADER ANGLES FOR STEEL JOISTS SHALL BE DESIGNED AND FURNISHED BY THE JOIST FABRICATOR AS NOTED ON THE DRAWINGS.
5. NET UPLIFT ON JOISTS SHALL BE 10 PSF UNLESS NOTED OTHERWISE ON DRAWINGS.

5.2 STEEL ROOF DECK

1. STEEL ROOF DECK SHALL BE PAINTED OR GALVANIZED AS INDICATED ON DRAWINGS.
2. END JOINTS OF ROOF DECK SHALL HAVE 2" MINIMUM END LAPS.
3. ROOF DECK SHALL BE 1-1/2" X 22 GAUGE WIDE RIB DECK, TYPE B, UNLESS NOTED OTHERWISE ON DRAWINGS.
4. ROOF DECK IS TO BE ATTACHED AT 12" O.C. OVER ALL INTERIOR SUPPORTS AND 6" O.C. OVER ALL EXTERIOR SUPPORTS, UNLESS NOTED OTHERWISE ON DRAWINGS.
5. ROOF DECK IS TO BE ATTACHED TO SUPPORTS USING DRILL POINT, SELF-TAPPING SCREWS AS INDICATED ON DRAWINGS.
6. PROVIDE SIDELAP SCREWS AS NOTED ON DRAWINGS, OR AS REQUIRED FOR MINIMUM SIDELAP ATTACHMENT BY THE STEEL DECK INSTITUTE.
7. MAKE JOINTS OVER SUPPORTING MEMBERS ONLY AND USE MINIMUM TRIPLE SPANS.
8. ROOF DECK CAN BE ATTACHED BY WELDING WITH THE APPROVAL OF THE ENGINEER. PUDDLE WELDS TO HAVE A NOMINAL DIAMETER OF 5/8".

5.4 COLD FORMED METAL FRAMING

1. ALL COLD FORMED MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH AMERICAN IRON AND STEEL INSTITUTE (AISI) 'SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS'.
2. ALL COLD FORMED METAL FRAMING MEMBERS SHALL BE OF THE TYPE, SIZE AND GAUGE AS SHOWN ON THE DRAWINGS.
3. ALL STUDS, JOISTS AND ACCESSORIES SHALL BE PRIMED WITH RUST INHIBITIVE PAINT MEETING THE PERFORMANCE REQUIREMENTS OF TT-P-636C, OR SHALL BE FORMED FROM STEEL HAVING A G-60 GALVANIZED COATING CONFORMING TO ASTM A924.
4. ALL PAINTED 12, 14 AND 16 GAUGE MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A570 WITH A MINIMUM YIELD STRENGTH OF 50 KSI. ALL GALVANIZED 12, 14 AND 16 GAUGE MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A653 WITH A MINIMUM YIELD STRENGTH OF 50 KSI.
5. ALL PAINTED 18 AND 20 GAUGE MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A611 WITH A MINIMUM YIELD STRENGTH OF 40 KSI. ALL GALVANIZED 18 AND 20 GAUGE MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A653 WITH A MINIMUM YIELD STRENGTH OF 33 KSI.
6. FASTENING OF COMPONENTS SHALL BE WITH SELF-DRILLING SCREWS OR WELDING. SCREWS OR WELDS SHALL BE OF SUFFICIENT SIZE TO INSURE THE STRENGTH OF THE CONNECTION. ALL WELDING IS TO BE DONE PER MANUFACTURER'S RECOMMENDATIONS ON ROD TYPE AND AMPERAGE. WIRE TYING OF COMPONENTS SHALL NOT BE PERMITTED.
7. ALL COLD FORMED METAL STUDS AND JOISTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS REGARDING MINIMUM INSTALLATION STANDARDS FOR BEARING, BRIDGING AND BRACING.
8. PROVIDE ALL ACCESSORIES INCLUDING, BUT NOT LIMITED TO, TRACKS, CLIPS, WEB STIFFENERS, ANCHORS, FASTENING DEVICES, RESILIENT CLIPS AND OTHER ACCESSORIES REQUIRED FOR A COMPLETE AND PROPER INSTALLATION, AND AS RECOMMENDED BY THE MANUFACTURER FOR THE STEEL MEMBERS USED.
9. PRIOR TO FABRICATION OF FRAMINGS, THE CONTRACTOR SHALL SUBMIT FABRICATION AND ERECTION DRAWINGS TO OBTAIN APPROVAL.
10. WHERE BRICK TIES ARE REQUIRED, THERE SHALL BE A MINIMUM OF (1) BRICK TIE FOR EVERY 2-2/3 S.F. OF WALL AREA. THESE SHALL BE SPACED AT A MAXIMUM OF 24" ON CENTER. TIES SHALL BE A MINIMUM OF 9 GAUGE CORROSION RESISTANT WIRE AND SHALL BE OF AN ADJUSTABLE TYPE SUCH AS 'DUR-A-WAL' ADJUSTABLE DA201 TYPE OR EQUAL. CORRUGATED, GALVANIZED SHEET TIES ARE NOT ACCEPTABLE. ALL TIES MUST BE ATTACHED THROUGH SHEATHING TO THE STUDS PER MANUFACTURER'S RECOMMENDATIONS. STRUCTURAL BACKING TO WHICH MASONRY VENEER ANCHORS ARE ATTACHED SHALL BE CORROSION RESISTANT AND HAVE A BASE METAL THICKNESS OF 0.043 MIN.

SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT		APPLICABLE TO THIS PROJECT			
MATERIAL / ACTIVITY	SERVICE	Y/N	EXTENT	AGENT*	DATE COMPLETED
1704.2 Inspection of Fabricators					
Verify fabrication/quality control procedures.	In-plant review	N			
Structural steel fabricator	Submit Certificate of Compliance	Y	Continuous	CMT	
1704.3 Steel Construction					
Material verification of high-strength bolts, nuts, and washers	Review material marking and certificates of compliance	Y	Periodic	CMT	
Inspection of high-strength bolting:	Field inspection				
a. Snug-tight joints		Y	Periodic	CMT	
Material verification of structural steel:					
a. Identification markings	Field inspection	Y	Periodic	CMT	
b. Certified mill tests	Review submittals	Y	Each submittal	DPoR	
Welding materials	Review certificate of compliance and field verification	Y	Periodic	CMT	
Structural steel welding:	Shop and field inspection				
a. Complete and partial penetration groove welds		N	Continuous		
b. Multi-pass fillet welds		N	Continuous		
c. Single-pass fillet welds > 5/16"		N	Continuous		
d. Single-pass fillet welds < 5/16"		Y	Periodic	CMT	
1704.4 Concrete Construction					
Inspection of reinforcing steel installation.	Field inspection	Y	Periodic	CMT	
Verify use of approved design mix	Field review	Y	Periodic	DPoR	
Fresh concrete sampling.	Field testing	Y	Continuous	CMT	
Inspection of anchors and reinforcing steel installed in hardened concrete: verify anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torqu	Field inspection	Y	Periodic	CMT	
Inspection of concrete and shotcrete placement for proper application techniques	Field inspection	Y	Continuous	CMT	
Concrete and shotcrete curing operations.	Field inspection	Y	Periodic	CMT	
Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports	Y	Periodic	CMT	
Inspection of formwork for shape, lines, location and dimensions	Field inspection	Y	Periodic	CMT	
1704.5 Masonry Construction					
Verify proportions of site prepared mortar, grout and prestressing grout for bonded tendons.	Field and submittal review	Y	Periodic	CMT	
Verify construction of mortar joints.	Field inspection	Y	Periodic	CMT	
Verify location of reinforcement and connectors, and placement of prestressing tendons and anchorages.	Field inspection	Y	Periodic	CMT	
Verify size and location of structural masonry elements.	Field and submittal review	Y	Periodic	CMT	
Verify type, size, and location of anchors including details of anchorage of masonry to structural members, frames, or other construction.	Field inspection	Y	Level 1 - Periodic Level 2 - Continuous	CMT	
Verify size, grade, and type of reinforcement.	Field inspection	Y	Periodic	CMT	
Verify welding of reinforcing bars.	Field inspection	N	Continuous	CMT	
Verify protection of masonry during hot/cold weather.	Field inspection	Y	Periodic	CMT	
Verify grout space is clean prior to grouting	Field inspection	Y	Level 1 - Periodic Level 2 - Continuous	CMT	
Verify grout placement complies with code and construction document provisions	Field inspection	Y	Continuous	CMT	
Observe preparation of prisms required by construction documents	Field inspection	Y	Continuous	CMT	
Verify compliance with required testing and inspection provisions of construction documents and the approved submittals.	Field testing and inspection	Y	Periodic	CMT	
1704.7 Soils					
Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	Field inspection	Y	Periodic	CMT	
Verify excavations are extended to proper depth and have reached proper material.	Field inspection	Y	Periodic	CMT	
Perform classification and testing of controlled fill materials.	Field inspection	Y	Periodic	CMT	
Verify use of proper materials, densities, and lift thicknesses during placement and compaction of controlled fill	Field inspection	Y	Continuous	CMT	
Prior to placement of controlled fill, observe subgrade and verify that site has been prepared properly	Field inspection	Y	Periodic	CMT	

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PROFESSIONAL SEAL:

GEORGIA REGISTERED No. 18701 PROFESSIONAL ENGINEER JACK LANCE BELL

PROJECT TITLE:

BUTLER CREEK RETAIL BUILDING

ACWORTH, GEORGIA

PROJECT INFO:

3330 COBB PARKWAY NW
ACWORTH, GA 30144

PRINT RECORD

Description	Date
ISSUED FOR PERMIT	01.17.20

REVISION

No.	Description	Date

STRUCTURAL NOTES

Project number	D19-127
Date	12/18/2019
Drawn by	JLB
Checked by	RM

S-100

Scale	As indicated

ISSUED FOR CONSTRUCTION

11/27/2019 1:19:04 AM