

**GENERAL LOADING NOTES**

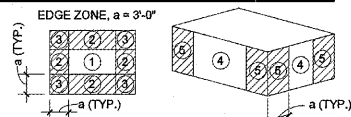
- Building Code: 2018 International Building Code
- Gravity Loads:
 

Location	Live Load Uniform	Dead Load Uniform
Slab-on-Grade	100 psf	—
Roof	20 psf	20 psf
Ground Snow Load, Pg	10 psf	—
Flat Roof Snow Load, Pf	12 psf	—

\*Uniform dead load includes actual structural weights.
- Wind Load:
  - Ultimate Design Wind Speed, Vult (3-second gust) 126 mph
  - Nominal Design Wind Speed, Vnd (3-second gust) 98.4 mph
  - Risk Category II
  - Wind Exposure C
  - Internal Pressure Coefficient (GCP) = ±0.18

\*THE SALIENT CORNER SHALL BE DEFINED AS THE VERTICAL SURFACE LOCATED WITHIN A DISTANCE OF 1/10TH THE LEAST WIDTH OF THE STRUCTURE BUT NOT MORE THAN 10'-0" FROM A PROMINENT SALIENT CORNER.

ZONE	Vasd = 98.4 MPH, EXP C RISK CATEGORY II, GCPI = ±0.18		
	10	50	100
1	+10.0 / -21.1	+10.0 / -19.8	+10.0 / -19.3
2	+19.3 / -35.4	+17.3 / -26.6	+16.4 / -22.9
3	+19.3 / -35.4	+17.3 / -26.6	+16.4 / -22.9
4	+19.3 / -20.9	+17.3 / -18.1	+16.4 / -17.2
5	+19.3 / -25.7	+17.3 / -20.1	+16.4 / -18.3



**Horizontal Design Pressures for Parapets:**

Area	Case A	Case B	Case A	Case B
10 sf	+50.7 psf	-35.5 psf	+50.7 psf	-40.6 psf
50 sf	+39.4 psf	-31.3 psf	+39.4 psf	-34.3 psf
100 sf	+34.6 psf	-29.5 psf	+34.6 psf	-31.6 psf

Note that Linear Interpolation is allowed between the areas listed above. See section above for Wind Speed, Exposure Factor and Risk Category. These wind pressure shall be used for the design of exterior component and cladding materials not specifically designed and detailed by the Structural Engineer of Record.

- Seismic:
  - Risk Category II
  - Seismic Importance Factor, (Ie) = 1.0
  - Site Coefficient
  - Ss = 0.257
  - Si = 0.099
  - Soe = 0.273
  - Sor = 0.159
  - Site Class D
  - Seismic Design Category C
- Geotechnical Report-
  - A. Geotechnical Investigation Performed By: Terracon, Inc. 73065101 & Report No. 16232 December 20, 2008
  - B. Project Number
  - C. Date of Report

**GENERAL SOIL PREPARATION AND FOUNDATION NOTES**

- Foundations have been designed in accordance with the Geotechnical Report prepared for this project. A copy of this report is available for inspection in the specifications.
- All excavation, fill, compaction and grading of the site shall be in accordance with the specifications and the recommendations contained in the geotechnical report(s).
- At the locations where utility trenches cross the building line, 5" of each trench centered on the building line shall be backfilled with a compacted, low-permeability clay.
- Continuous and spread footings foundations -
  - A. Design Bearing Pressure is 2,500 psf (net) for foundations bearing on suitable soils per Geotech Report. Bearing materials shall be verified by a licensed geotechnical engineer.
  - B. All foundations are designed and detailed with formed sides. If the contractor elects to use earth formed sides, one inch shall be added to each side to provide adequate cover over the reinforcing at the contractors expense.
  - C. Moisture content in footing excavations shall be maintained until footing is placed. Footings shall be pured as soon as practical after excavations are completed.
- Do not backfill foundation walls until the restraining slabs or adequate bracing are in place. All backfill shall be placed and compacted in accordance with the specification.
- Exterior slabs shall slope away from the structure a minimum of 1/4" per foot unless otherwise noted.
- Bottom of footings shall bear a minimum of 24" below adjacent finished grade.

**CAST-IN-PLACE CONCRETE NOTES**

- The concrete requirements are-
  - A. 28 Day concrete compressive strengths-
 

Mix Location	Min. FC(PSI)	Max. Slump (in)	Max. W/C Ratio	Max. Aggregate ASTM C33	Air Content
Foundations	3000	5	0.55	1" No. 57	0%
Slab-on-Grade	4000	4	0.45	3/4" No. 87	0%

\*All Mixes exposed to freeze/thaw shall be air entrained, re: to specifications.
  - Concrete clear cover over reinforcing shall be in accordance with the current edition of ACI 308 listed below unless otherwise noted -
 

Location	Clear Cover (inches)
Cast against earth	3"
Exposed to earth or weather #6 and larger	3"
Exposed to earth or weather #5 and smaller	1 1/2"
Slabs not exposed to weather	1 1/2"
Slabs-on-grade (cover from top of slab)	1 1/2"
- Concrete reinforcing shall meet the following -
  - A. Reinforcing bars shall conform to the requirements of ASTM A615 Grade 60. Reinforcing bars required to be welded shall conform to the requirements of ASTM A706 Grade 60. Welding of reinforcing other than specified is prohibited.
  - B. All reinforcing shall be continuous. Continuous bars shall lap in accordance with table "A" on sheet S0.0, unless otherwise noted.
  - C. Provide corner bars in outside face of all foundations and walls equal in size and spacing to main horizontal reinforcing. Extend inside face reinforcing of all foundations and walls to outside face and bend to a standard 90 degree hook.

**CAST-IN-PLACE CONCRETE NOTES CONT'D**

- Concrete reinforcing shall meet the following-
  - D. Provide 2-#5 each side of each opening thru walls or slabs unless noted otherwise. Bars to extend 2'-0" past opening.
  - E. Shop Drawings shall be submitted with reinforcing steel detailed in accordance with the current addition of ACI 318.
- Forming and embedment shall meet the following -
  - A. All foundations are designed and detailed with formed sides. If the contractor elects to use earth formed sides, one inch shall be added to each side to provide adequate cover over the reinforcing at the contractors expense.
  - B. All exposed edges of concrete shall be chamfered 3/4" inside forms or tolled to 3/4" radius on slabs unless otherwise noted.
  - C. Slabs-on-Grade shall have construction joints and control joints (sawed joints) located as shown on the drawings. Contractor shall locate slab joints on shop drawings.
  - D. At the contractors option, either the sawed or keyed construction joint may be used. The keyed joint shall be used to terminate any placement.
  - E. Where necessary, vertical construction joints shall be located within the center one-third of the span. All joints shall be thoroughly cleaned and purposely roughened to 1/4" prior to placing adjacent concrete. Joints in exposed concrete shall be used with a maximum spacing of 50'.
  - F. The contractor shall be responsible for the design of all forming, temporary bracing and shoring.
  - G. No aluminum shall be embedded in concrete. Conduit and piping embedded in concrete shall be spaced a minimum of four diameters and the outside diameter shall be less than 30% of the member thickness placed between layers of reinforcing.
- Curing for concrete surfaces not in contact with forms: One of the following procedures shall be applied immediately after completion of placement and finishing -
  - A. Ponding or continuous sprinkling.
  - B. Application of absorptive mats or fabric kept continuously wet.
  - C. Application of waterproof sheet materials, conforming to specifications for waterproof sheet materials for curing concrete (ASTM C171).
  - D. Application of a curing compound conforming to specifications for liquid membrane - forming compounds for curing concrete (ASTM C308). The compound shall be applied in accordance with the recommendations of the manufacturer immediately after any water sheen which may develop after finishing has disappeared from the concrete surface. It shall not be used on any surface against which additional concrete is to be bonded unless it is proven that the curing compound will not prevent bond, or unless positive measures are taken to remove it completely from areas to receive bonded applications. Curing shall be continued at least 7 days.
  - E. Perform adequate slab moisture emission tests per ASTM F1869-04 to confirm that emission levels meet the covering manufacturer's specification before placing the covering. These tests should not be conducted until the buildings closed in and the HVAC equipment run sufficiently to create temperature/humidity environment that is representative of the typical conditions the covering will experience.
- All vapor retarder's shall be Stego Wrap 15-Mil Class A vapor retarder or approved equal unless more stringent requirements are shown in specifications. 6 mil visqueen is not approved equal.

**COLD FORMED METAL FRAMING NOTES (CFMF)**

- All exterior or load bearing walls shall be constructed of steel "C" studs of the size shown in the plans and shall conform to AISI specifications. Minimum section properties shall meet steel stud manufacturers association's published section properties.
- Minimum width of the studs shall be 1 5/8", and the lip of the "C" portion shall be a minimum of 1/2".
- Stud track shall be 16 ga. minimum or as indicated in details and shall be anchored as follows:
  - To Concrete
  - Red Head (Ramset) large diameter tapcon (LDT) anchors 3/8" dia. with 2 1/2" embedment @ 32" o.c. (or approved equal)
- Steel studs shall be installed in accordance with the manufacturer's recommendations. Horizontal bridging shall be placed at 4'-0" o.c. or as per manufacturer's recommendation if less than 4'-0".
- A minimum yield strength of 33KSI (33,000 psi) with the exception that 16 ga. and heavier studs shall have a minimum yield strength of 50KSI (50,000 psi).

MINIMUM DESIGN THICKNESS (MILS)	DESIGN THICKNESS (IN.)	INSIDE CORNER RADIUS (IN.)	GAGE NO. (REFERENCE ONLY)
18	0.0188	0.0843	25
27	0.0283	0.0796	22
30	0.0312	0.0781	20-DRY-WALL
33	0.036	0.0764	20-STRUCTURAL
43	0.0451	0.0712	18
54	0.0566	0.0849	16
68	0.0713	0.1069	14
97	0.1017	0.1525	12

NOTE: Minimum Thickness represents 95% of the thickness and is the minimum acceptable thickness delivered to the job site based on Section A3.4 of the AISI Specification.

- Strap x-bracing shall have a minimum yield strength of 50 ksi. See detail 9/S2.2 for strap bracing connection to stud wall and slab.
- Strap x-bracing locations shown on the plans are approximate. A just strap bracing locations miss wall openings.
- When a strap is spliced together, weld the two pieces together with 1/8" x 4" long fillet weld (N.S. & F.W.).
- Install strap bracing along the outside face of the steel joist unless noted otherwise. Typical exterior stud attachment to top & bottom runner tracks shall be (2) #16 HWH self-curing screws minimum.
- Strap x-bracing shall be pulled tight, eliminating any sagging in the strap. Attach the strap to each intermediate stud with one #9 screw.
- Typical light gauge metal framing connections established by the American Iron and Steel Institute (AISI) is as follows:
 

MEMBER DEPTH	MEMBER SHAPE	RETURN LIP DIMENSION	MINIMUM DESIGN THICKNESS (MILS)
36" x 1.625"	S = STUD/JOIST	125 = 1.25"	(SEE SCHEDULE ABOVE)
600 = 8.00"	T = TRACK	150 = 1.50"	
800 = 8.00"	U = U-CHANNEL	162 = 1.625"	
1000 = 10.00"	F = FURRING CHANNEL	200 = 2.00"	

**MISCELLANEOUS NOTES**

- Reproduction of structural contract documents as shop drawings, erection plans, fabrication plans or details if not authorized and, if submitted, will be rejected without checking. A license to use any portion or all the CAD files for the limited purpose of assisting contractor's preparation of shop drawings for submittal under the construction contract may be purchased from HFA AE under a standard form of agreement for \$1,000. Files were created in Revit 2018.
- Contractor shall supply all items for attaching mechanical and electrical equipment to the building structure to resist all loads including seismic forces. Attachment shall be made so as not to overstress structural members. Coordinate the attachments and locations of the equipment with the structural shop drawings. Re: to mechanical and electrical drawings for additional requirements.
- Substitution of expansion anchors for embedded anchors as shown on the drawings will not be permitted unless approved by the engineer in advance.
- The contractor shall provide the following additional services -
  - A. Verification of all dimensions, elevations, opening sizes, and mechanical equipment weights prior to starting work.
  - B. Provide temporary bracing and shoring as required for stability during construction.
  - C. Verification of all floor depressions and offsets with architectural drawings.
  - D. Remove all abandoned foundations, utilities, pipelines, etc. that interfere with new construction.
  - E. Review and approve all shop drawings prior to submittal noting changes made which do not comply with design drawings.
- Plans, sections, and details are not to be scaled for determination of quantities, lengths, or fit of materials.

**MISCELLANEOUS NOTES CONT'D.**

- See architectural, mechanical and electrical drawings for size and location of all openings, sleeves, curbs, pads, inserts, etc. not shown on structural drawings. Before fabrication of materials, coordinate with mechanical and electrical requirements.
  - Construction documents consist of these drawings and a separate book of specifications. The drawings and specifications are complimentary, neither is meant to stand alone for any portion of the work described herein. Any conflict between drawings and specifications shall be reported immediately to the architect.
  - Epoxy anchors shall be Hilti HAS Rods embed with Hilti HIT-HY 200 adhesive. Anchor rod size and embedment shall be as indicated in the details.
- CONCRETE COLD WEATHER REQUIREMENTS**
- In cold weather, all heating equipment and protective enclosures shall be on the job ready for use when concrete is placed. Such equipment shall be adequate for the purpose of heating the concrete materials and protecting the concrete in accordance with the following requirements:
- Whenever the temperature of the surrounding atmosphere is 40°F or lower, or there is a probability of it falling below 40°F, all aggregates and water shall be pre-heated and all forms, fill and ground with which the concrete is in contact or is calculated to come in contact with, shall be de-frosted. Steel reinforcement and aggregates shall be protected by adequate means to prevent formation of an ice film.
  - All concrete placed in the forms shall have a temperature between 60°F and 80°F and adequate means shall be provided by enclosures and heating equipment for maintaining a temperature of at least 72°F for the first seven days and at least 50°F for as much more time as may be necessary, as determined by the engineer, to ensure the proper curing of the concrete.
  - Canvas or other protective covering shall be kept clear of all concrete in order to permit free circulation of air around all walls, columns, and over tops of all slabs.
  - The contractor is to protect the soil from freezing during the construction period, this includes footings that have been cast.
  - Whenever the temperature of the surrounding atmosphere is 40°F to 50°F, no concrete work shall be done unless approved by the structural engineer.

**STRUCTURAL STEEL NOTES**

- Structural steel shall meet the following requirements unless otherwise noted on the drawings -
 

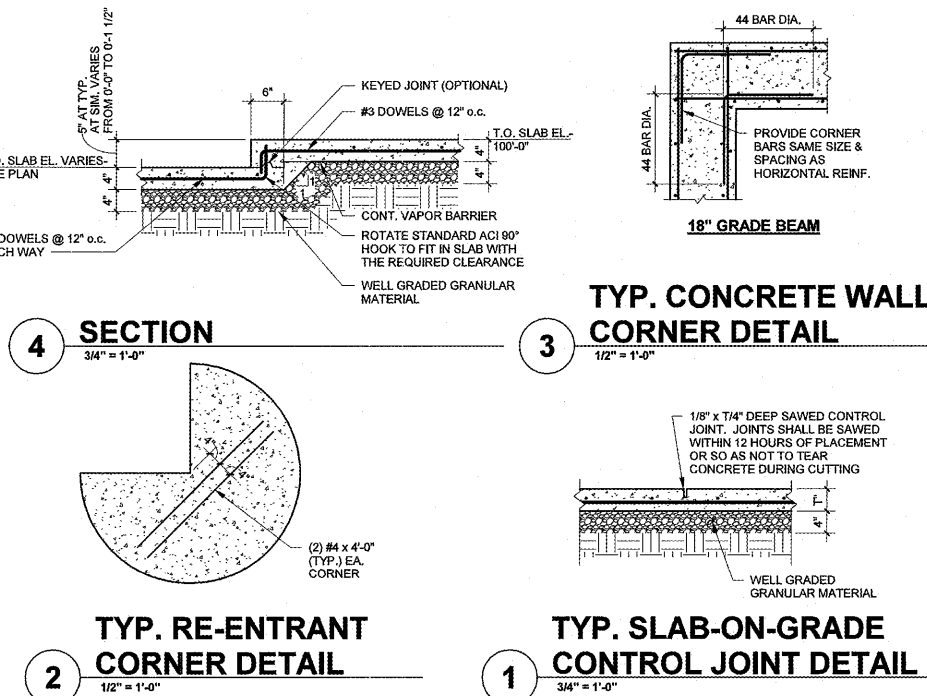
Type	ASTM	Grade	Fy(Min) (psi)
Structural shapes (except angles)	A992	50	50,000
Steel Angles, Plates, & Rods	A36	—	36,000
Structural Tubing	A500	B	46,000
Anchor Bolts	F1554	36	36,000
Structural Bolts	A325	—	—
- Structural steel connections -
  - A. Connection bolts shown in drawings are 3/4" diameter A325 bearing type "N" unless noted otherwise
  - B. All bolted connections requiring friction type pretensioned bolts will be indicated as (F.T.). Direct tension indicator (DTI) washers shall be used to insure proper tensioning.
  - C. All welding shall be in accordance with latest AWS code. All welds shall use E70XX electrodes.
- Steel framing connections shall be lightened and columns shall be plumbed and grouted below the baseplate in place before decking is attached to frame.
- No holes shall be cut thru steel beams in field unless approved in writing.

**REQUIRED SPECIAL INSPECTIONS (BY TESTING AGENCY)**

IN ADDITION TO THE REGULAR INSPECTIONS REQUIRED BY SECTION 110, THE FOLLOWING ITEMS REQUIRE SPECIAL INSPECTION IN ACCORDANCE WITH SECTION 110.1706 & 110.1709 OF THE 2018 IBC.

ITEM	SECTION
Inspection of Fabrication - Verify fabrication/quality control procedures	1704.2.1
Steel Construction - Field Welding	Table 1705.2.2
- Details	Table 1705.2.2
- High-Strength Bolts	Table 1705.2
Concrete Construction - Reinforcing Steel Installation	Table 1705.3
- Cast-in-place Anchor Bolts	Table 1705.3
- Verify Design of Formwork	Table 1705.3
- Fresh Concrete Sampling	Table 1705.3
- Concrete Placement	Table 1705.3
- Concrete Curing Operations	Table 1705.3
- Evaluation of Concrete Strength	Table 1705.3
Soils - Verify materials below are adequate to achieve design bearing capacity	Table 1705.6
- Verify excavations are extended to proper depth and have reached proper bearing material	Table 1705.6
- Perform classification and testing of controlled fill materials	Table 1705.6
- Verify site preparation with soils report	Table 1705.6
- Verify use of proper materials, densities and lift thickness during placement and compaction of controlled fill	Table 1705.6

- REQUIRED SPECIAL INSPECTION NOTES:**
- Refer to project specification for additional quality control/quality assurance requirements.
  - General Contractor shall coordinate any additional special inspection requirements with Owner and applicable building authorities.
  - Special Inspections are the responsibility of the Owner.
  - The names of persons or firms who are to perform the Special Inspections shall be forwarded to the Building Official for approval.
  - The Special Inspector(s) shall complete and submit all forms required by the Authority Having Jurisdiction.



**TABLE "A" REINFORCEMENT TENSION LAPS, EMBEDMENT LENGTHS AND HOOK LENGTHS fy=60000 PSI**

BAR SIZE	LAP CLASS	SPLICE		HOOK LENGTH	BAR SIZE	LAP CLASS	SPLICE		HOOK LENGTH
		TOP	OTHER				TOP	OTHER	
#3	A	20"	16"	6"	#3	A	18"	14"	7"
	B	27"	21"			B	23"	18"	
#4	A	28"	22"	11"	#4	A	25"	19"	9"
	B	36"	28"			B	32"	25"	
#5	A	36"	27"	13"	#5	A	31"	24"	12"
	B	46"	36"			B	40"	31"	
#6	A	43"	33"	16"	#6	A	37"	28"	14"
	B	56"	43"			B	48"	37"	
#7	A	62"	48"	18"	#7	A	54"	42"	16"
	B	81"	62"			B	70"	54"	

- NOTES:**
- LENGTHS SHOWN CONFORM WITH NON-SEISMIC PROVISIONS OF THE CURRENT EDITION OF ACI 318 FOR UNCOATED BARS NOT ENCLOSED BY CLOSELY SPACED SPALLS OR REINFORCEMENT DEVELOPMENT OF REINFORCEMENT NOT COVERED BY THE TABLES CONFORM WITH THE CURRENT EDITION OF ACI 318.
  - BAR CLEAR SPACING IS THE CENTER TO CENTER BAR SPACING MINUS TWO BAR DIAMETERS WHEN ALL BARS ARE LAPPED AT THE SAME LOCATION. BAR CLEAR SPACING IS TWICE THE CENTER TO CENTER BAR SPACING MINUS TWO BAR DIAMETERS WHEN ALL BARS ARE EMBEDDED AT THE SAME LOCATION. THE BAR CLEAR SPACING IS THE CENTER TO CENTER BAR SPACING MINUS ONE BAR DIAMETER.
  - CLASS A LAP LENGTHS APPLY WHEN BARS ARE STAGGERED TO LAP HALF THE BARS AT THE SAME LOCATION OR WHEN BARS ARE LAPPED AT A LOCATION OF MINIMUM STRESS IN THE BARS.
  - LAP AND EMBEDMENT LENGTHS SHOWN APPLY WHEN BAR MINIMUM CONCRETE COVER OVER BARS CONFORMS WITH VALUES GIVEN IN THE TABLE FOR "CONCRETE COVER". THESE COVER VALUES CONFORM WITH THE CURRENT EDITION OF ACI 318.
  - CLASS A LAP AND EMBEDMENT LENGTH HAVE THE SAME VALUE.
  - CLASS B LAP LENGTHS APPLY WHEN ALL BARS ARE SPLICED AT A LOCATION OF MAXIMUM STRESS IN THE BARS.
  - HOOK LENGTH GIVEN IS THE STRAIGHT LINE DISTANCE FROM THE LOCATION OF MAXIMUM STRESS IN THE BAR TO THE OUTSIDE END OF THE HOOK. MULTIPLY LENGTHS GIVEN BY 0.7 FOR HOOKS WITH SIDE COVER NORMAL TO THE HOOK NOT LESS THAN 2 1/2 INCHES AND FOR 90 DEGREE HOOKS COVER ON BAR EXTENSION BEYOND HOOK NOT LESS THAN 2 INCHES.
  - TOP BARS ARE HORIZONTAL REINFORCEMENT PLACED SO THAT MORE THAN 12 INCHES OF CONCRETE IS CAST BELOW THE REINFORCEMENT.
  - MULTIPLY LAP AND EMBEDMENT LENGTHS BY 2.0 FOR BARS WITH CLEAR SPACING OF TWO BAR DIAMETERS OR LESS OR CONCRETE COVER OF ONE BAR DIAMETER OR LESS.
  - MINIMUM CONCRETE COVER FROM FACE OF MEMBER TO EDGE BAR SHALL NOT BE LESS THAN TWO AND ONE HALF BAR DIAMETERS.

**TYPICAL STRUCTURAL ABBREVIATIONS**

ACI	AMERICAN CONCRETE INSTITUTE	LBS.	POUNDS
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	LG.	LONG
A.B.	ANCHOR BOLT	LH	LONG LEG HORIZONTAL
A.E.S.	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL	LV	LONG LEG VERTICAL
A.F.F.	ABOVE FINISH FLOOR	LONG.	LONGITUDINAL
ARCH.	ARCHITECT	MAX.	MAXIMUM
ASTM	AMERICAN SOCIETY OF TESTING MATERIALS	MBM	METAL BUILDING MANUFACTURER
BAL.	BALANCE	MECH.	MECHANICAL
B.F.F.	BELOW FINISHED FLOOR	MFR.	MANUFACTURER
B.L.	BLOCK LINTEL	MIN.	MINIMUM
BLDG.	BUILDING	MISC.	MISCELLANEOUS
BM.	BEAM	MTL.	METAL
B.O.D.	BOTTOM OF DECK	N.I.C.	NOT IN CONTRACT
BOTT.	BOTTOM	N.S.	NEAR SIDE
BRG.	BEARING	N.T.S.	NOT TO SCALE
CL or CL	CENTERLINE	No.	NUMBER
C.J.	CONTRACTION JOINT	O.C.	ON CENTER
CLR.	CLEAR	O.D.	OUTSIDE DIAMETER
CMU	CONCRETE MASONRY UNIT	O.F.	OUTSIDE FACE
COL.	COLUMN	O.H.	OPPOSITE HAND
CONC.	CONCRETE	OPNG.	OPENING
CONN.	CONNECTION	PAF.	POWDERED ACTUATED FASTENER
CONST.	CONSTRUCTION	P.M.E.J.	PREMOLDED EXPANSION JOINT
CONT.	CONTINUOUS	PCF	POUNDS PER CUBIC FOOT
CTRD.	CENTERED	PED	PEDESTAL
D.B.A.	DEFORMED BAR ANCHOR	PL	PLATE
DEG. or °	DEGREE	PLF	POUNDS PER LINEAR FOOT
DIA. or Ø	DIAMETER	PROJ.	PROJECTION
DM.	DIMENSION	PSF	POUNDS PER SQUARE FOOT
DN	DOWN	PSI	POUNDS PER SQUARE INCH
DO	DITTO	QTY.	QUANTITY
DTL.	DETAIL	R.O.	ROUGH OPENING
DVG.	DRAWING	REF.	REFER
DWV.	DOWEL	R.	RADIUS
EA.	EACH	REINF.	REINFORCEMENT
E.F.	EACH FACE	RECD	REQUIRED
E.J.	EXPANSION JOINT	RTU	ROOF TOP UNIT
E.O.D.	EDGE OF DECK	S.D.S.	SELF-DRILLING SCREWS
E.O.S.	EDGE OF SLAB	SCHED.	SCHEDULE
EL.	ELEVATION	SECT.	SECTION
EQ.	EQUAL	SM.	SIMILAR
EW	EACH WAY	S.I.J.	STEEL JOIST INSTITUTE
EXIST.	EXISTING	S.O.G.	SLAB-ON-GRADE
EXP.	EXPANSION	SPA.	SPACING
F.F.E.	FINISH FLOOR ELEVATION	SPECS	SPECIFICATIONS
FLR.	FLOOR	STD.	STANDARD
FDN.	FOUNDATION	STIFF.	STIFFENER
FTG.	FOOTING	STL.	STEEL
F.S.	FAR SIDE	T&B	TOP AND BOTTOM
G.B.	GRADE BEAM	T.O.	TOP OF
GA.	GAUGE	T.O.P.	TOP OF PIER
GALV.	GALVANIZED	TOC	TOP OF CONCRETE
HK.	HOOK	TOF	TOP OF FOOTING
H.K.	HORIZONTAL	TOW	TOP OF WALL
H.S.A.	HEADED STUD ANCHOR	TRANS.	TRANSVERSE
I.F.	INSIDE FACE	TYP.	TYPICAL
IBC	INTERNATIONAL BUILDING CODE	U.N.O.	UNLESS NOTED OTHERWISE
INFO.	INFORMATION	VERT.	VERTICAL
JBE	JOIST BEARING ELEVATION	W	WITH
JST.	JOIST	WT.	WEIGHT
JT.	JOINT	W.P.	WORK POINT
KSI	KIPS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

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