

GENERAL

- 1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, DESIGN PROFESSIONAL, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THAT SET FORTH IN THE CONTRACT DOCUMENTS NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE DESIGN PROFESSIONAL OF RECORD OR ANY OF THE DESIGN PROFESSIONAL OF RECORD'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.

CODE/DESIGN CRITERIA

- 1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE FOLLOWING:
INTERNATIONAL BUILDING CODE, 2012 EDITION WITH GEORGIA AMENDMENTS.
2. GRAVITY LOADS
2.1 UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):
PRESS BOX STAIRS 100 PSF
2.2 UNIFORM ROOF LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):
ROOF 20 PSF
GROUND SNOW LOAD, Ps 5 PSF
2.3 DEAD LOADS (IN ADDITION TO STRUCTURE SELF-WEIGHT):
FLOOR:
MISCELLANEOUS 3 PSF
CEILING/MEP 8 PSF
ROOF:
MISCELLANEOUS 3 PSF
ROOFING/INSULATION 9 PSF
CEILING/MEP 6 PSF
3. WIND LOADS:
ULTIMATE DESIGN WIND SPEED, Vu1 = 115 MPH
NOMINAL DESIGN WIND SPEED, Vd10 = 90 MPH
RISK CATEGORY: II
EXPOSURE C
INTERNAL PRESSURE COEFFICIENT = +/-0.18
SEE COMPONENT AND CLADDING DESIGN WIND PRESSURE DIAGRAM
4. EARTHQUAKE LOADS:
RISK CATEGORY: II
SEISMIC IMPORTANCE FACTOR: I = 1.0
SHORT PERIOD MAPPED SPECTRAL RESPONSE ACCELERATION COEFFICIENT, Sds = 0.233
1 SECOND PERIOD MAPPED SPECTRAL RESPONSE ACCELERATION COEFFICIENT, SD1 = 0.128
ASSUMED SITE CLASS D
SHORT PERIOD DESIGN SPECTRAL RESPONSE ACCELERATION COEFFICIENT, SDS = 0.238
1 SECOND PERIOD DESIGN SPECTRAL RESPONSE ACCELERATION COEFFICIENT, SD1 = 0.128
SEISMIC DESIGN CATEGORY: C
ANALYSIS PROCEDURE: VALENT LATERAL FORCE PROCEDURE
6. THE EXISTING STRUCTURE, WITH PROPOSED MODIFICATIONS, HAS BEEN ANALYZED FOR GRAVITY AND LATERAL LOADS AND FOUND TO BE IN CONFORMANCE WITH IBC SECTION 5403 AND 5404 FOR ADDITIONS AND ALTERATIONS TO EXISTING STRUCTURE.
7. UNLESS NOTED OTHERWISE CALCULATED INDIVIDUAL MEMBER DEFLECTIONS (IN INCHES) DO NOT EXCEED THE FOLLOWING:
DEAD LOAD LIVE LOAD DEAD + LIVE LOAD
ROOF MEMBERS: L/360 L/360 L/240
FLOOR MEMBERS: L/360 L/360 L/240
WHERE: L = SPAN LENGTH (IN INCHES) BETWEEN SUPPORTS. (FOR CANTILEVERS, L IS TWICE THE LENGTH OF THE CANTILEVER) NOTE THAT THE TOTAL MAXIMUM CALCULATED FLOOR SYSTEM DEFLECTION WILL BE THE SUM OF THE DEFLECTIONS OF THE SUPPORTED ELEMENTS IN A BAY.
THE CALCULATED DEFLECTION FOR INDIVIDUAL MEMBERS SUPPORTING MASONRY DO NOT EXCEED L/600 FOR DESIGN LOADS APPLIED AFTER THE INSTALLATION OF THE MASONRY.

- 8. SPECIAL INSPECTIONS:
8.1 THE STRUCTURAL TESTING/INSPECTION AGENCY, SEE SPECIFICATION SECTION 014625, WILL PERFORM SPECIAL INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE. MATERIALS AND WORK TO BE INSPECTED INCLUDE CONCRETE, MASONRY, STEEL, AND COMPOSITE CONSTRUCTION. SEE SCHEDULE OF SPECIAL INSPECTIONS FOR A COMPLETE LIST OF WORK REQUIRING SPECIAL INSPECTIONS.
8.2 SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE ARE REQUIRED FOR STRUCTURAL COMPONENTS AND ASSEMBLIES WHICH ARE NOT FABRICATED AT THE CONSTRUCTION JOB SITE INCLUDING BUT NOT LIMITED TO FLOOR AND ROOF JOISTS OF STEEL MATERIALS, AND STRUCTURAL STEEL FRAMING.
8.3 SPECIAL INSPECTION AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE MAY BE WAIVED FOR ITEMS WHICH ARE PRODUCED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED UPON REVIEW OF THE FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND BY PERIODIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE WHICH STATES THAT THE FABRICATION WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.
8.4 THE PROJECT OWNER WILL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PERFORM INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE DURING CONSTRUCTION OF THE PROJECT. DOCUMENTATION THAT SUMMARIZES THE QUALIFICATION AND CREDENTIALS OF EACH SPECIAL INSPECTOR AND DEMONSTRATES COMPETENCE FOR INSPECTION OF EACH PARTICULAR TYPE OF CONSTRUCTION REQUIRING SPECIAL INSPECTION SHALL BE SUBMITTED TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.
8.5 APPROVED SPECIAL INSPECTORS SHALL FURNISH INSPECTION REPORTS TO THE CHIEF COMMERCIAL BUILDING INSPECTOR OR HIS DESIGNEE WHICH INDICATE THAT THE WORK INSPECTED WAS DONE IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. A FINAL REPORT WHICH DOCUMENTS THE RESULTS OF THE SPECIAL INSPECTIONS PERFORMED, INCLUDING CORRECTION OF ANY DISCREPANCIES IDENTIFIED DURING INSPECTION SHALL BE SUBMITTED PERIODICALLY AT A FREQUENCY APPROVED BY CHIEF COMMERCIAL BUILDING INSPECTOR PRIOR TO CONSTRUCTION.
9. NO PROVISIONS HAVE BEEN MADE FOR FUTURE HORIZONTAL OR VERTICAL EXPANSION.

FOUNDATION

- 1. EXISTING FOUNDATIONS HAVE BEEN EVALUATED BASED UPON AN ALLOWABLE BEARING CAPACITY OF 2500 PSF. EXISTING SOIL CONDITIONS SHALL BE REVIEWED BY A REGISTERED PROFESSIONAL GEOTECHNICAL ENGINEER. FOUNDATION CONDITIONS SHALL BE REVIEWED AND TAKE INTO CONSIDERATION THE TYPE OF BUILDING AND FOUNDATION LOADS INVOLVED AS WELL AS THE REQUIREMENTS OF THESE DOCUMENTS. WHERE THE ALLOWABLE BEARING CAPACITY IS LESS THAN ASSUMED FOR DESIGN THE GEOTECHNICAL ENGINEER SHALL NOTIFY THE DESIGN PROFESSIONAL, SO THAT THE DESIGN PROFESSIONAL CAN PROVIDE ADDITIONAL FOOTING RECOMMENDATIONS. DESIGN PROFESSIONAL IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT TO THOSE ASSUMED FOR DESIGN.
2. STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM.

REINFORCEMENT

- 1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND HAVE MINIMUM SIDE AND END LAP OF 8".
3. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE.
4. SPLICES SHALL BE CLASS B IN ACCORDANCE WITH ACI318 UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE DESIGN PROFESSIONAL.
5. PROVIDE DOVELLS FROM FOUNDATIONS THE SAME SIZE AND NUMBER AS THE VERTICAL WALL OR COLUMN REINFORCING, UNLESS NOTED OTHERWISE.
6. PLACE REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:
6.1 CONCRETE REINFORCEMENT COVER
NOT EXPOSED TO EARTH OR WEATHER:
ELEVATED SLABS 3/4" CLEAR
6.2 MASONRY REINFORCING STEEL SHALL BE PLACED IN THE CENTER OF THE WALL UNLESS NOTED OTHERWISE.
7. REINFORCING STEEL DESIGNATED CONTINUOUS SHALL BE LAPPED AS FOLLOWS:
CONCRETE REINFORCEMENT: CLASS B TENSION LAP
MASONRY REINFORCEMENT: 4# BAR DIAMETERS
8. ADHESIVE FOR REINFORCING DOVELLS IN EXISTING CONCRETE SHALL CONFORM TO ASTM C881-02, TYPE IV, GRADE 3, CLASS A, B, & C EXCEPT GEL TIMES AND EPOXY CONTENT. ADHESIVE SHALL CONSIST OF A TWO COMPONENT ADHESIVE SYSTEM CONTAINED IN SIDE BY SIDE PACKAGING CONNECTED TO A MIXING NOZZLE WHICH THOROUGHLY MIXES THE COMPONENTS AS IT IS INJECTED INTO THE HOLE. ADHESIVE SHALL HAVE PASSED ICC EVALUATION SERVICES, INC. (ICC-ES) ACCEPTANCE CRITERIA 308 FOR LONG TERM CREEP. REINFORCING INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 308 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT LENGTH SHALL BE 12 BAR DIAMETERS, UNLESS NOTED OTHERWISE.
10. ALL DOVELLS AND TERMINATING BARS SHALL HAVE A STANDARD 90 DEGREE HOOK.
11. ALL HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL AND/OR CONSTRUCTION JOINTS AND AROUND CORNERS.

CAST-IN-PLACE CONCRETE

- 1. CONCRETE WORK SHALL CONFORM TO ACI318 AND CRSI STANDARDS.
2. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH:
2.1 LIGHT WEIGHT STRUCTURAL CONCRETE (110-120 PCF FRESH UNIT WEIGHT/107-116 PCF AIR-DRIED UNIT WEIGHT):
SLABS ON COMPOSITE DECK 3500 PSI
3. PIPES OR DUCTS SHALL NOT EXCEED ONE THIRD THE SLAB OR WALL THICKNESS INCLUDING CROSSING UNLESS SPECIFICALLY DETAILED IN THE STRUCTURAL DOCUMENTS. ALL PIPES AND DUCTS SHALL BE PLACED IN THE MIDDLE THIRD OF THE SLAB OR WALL THICKNESS UNLESS SPECIFICALLY DETAILED OTHERWISE IN THE STRUCTURAL DOCUMENTS. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF PIPES, ETC.
4. REFER TO ARCHITECTURAL DRAWINGS FOR DOOR SILL FINISHES, CLIPS OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND/OR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.
5. CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE DESIGN PROFESSIONAL. NO HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL DOCUMENTS.
6. DEFECTIVE AREAS IN CONCRETE INCLUDING BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTH EXCEEDING 0.016 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE DESIGN PROFESSIONAL.

CONCRETE MASONRY

- 1. ALL CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 WITH A DENSITY LESS THAN 105 PCF (10.5 LB/FT3). MINIMUM 28-DAY COMPRESSIVE STRENGTH OF CONCRETE MASONRY SHALL BE FM CLASS 1500 PSI.
2. MASONRY SHALL COMPLY WITH THE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY AND SHALL BE OF THE FOLLOWING TYPE:
WALLS BELOW GRADE TYPE M
BEARING WALLS TYPE M OR S
3. CONCRETE MASONRY UNITS SHALL BE GROUTED WITH 2500 PSI COARSE GROUT AS SHOWN IN THE STRUCTURAL DOCUMENTS. GROUT FOR REINFORCED AND NONREINFORCED MASONRY SHALL CONFORM TO ASTM C476.
4. PROVIDE HORIZONTAL JOINT REINFORCEMENT WITH NO. 5 GAGE LONGITUDINAL WIRES AT 16" O/C VERTICALLY UNLESS NOTED OTHERWISE. PROVIDE SPECIAL ACCESSORIES FOR CORNERS, INTERSECTIONS, ETC.
5. PROVIDE OPEN BOTTOM BEAM BLOCK UNITS WITH 4" DEEP MINIMUM WEB OPENINGS AT HORIZONTAL REINFORCEMENT LOCATIONS. A MINIMUM CLEAR SPACE OF ONE BAR DIAMETER SHALL BE PROVIDED BETWEEN THE REINFORCING BARS AND THE FACE OF MASONRY UNITS.
6. PROVIDE CONTROL JOINTS IN ALL CONCRETE MASONRY WALLS AT LOCATIONS APPROVED BY THE DESIGN PROFESSIONAL AT A MAXIMUM SPACING OF 3 TIMES THE WALL HEIGHT OR 40'-0", WHICHEVER IS LESS.
7. PROVIDE DOVETAIL ANCHORS AT 16" O/C, UNLESS NOTED OTHERWISE, WHERE MASONRY WALLS ABUT CONCRETE SURFACES.
8. SUBMIT WRITTEN CONSTRUCTION PROCEDURES PRIOR TO THE START OF MASONRY CONSTRUCTION.
9. MINIMUM VERTICAL WALL REINFORCEMENT SHALL BE #5@32" O/C UNLESS NOTED OTHERWISE.
10. SUBMIT SHOP DRAWINGS FOR MASONRY REINFORCEMENT IN ACCORDANCE WITH SPECIFICATION SECTION 03200.

STRUCTURAL STEEL

- 1. STRUCTURAL STEEL SHALL CONFORM TO ASTM A992, UNLESS NOTED OTHERWISE.
STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM A500, GRADE B.
STRUCTURAL CHANNELS, MISCELLANEOUS PLATES AND CONNECTION MATERIAL SHALL CONFORM TO ASTM A36, UNLESS NOTED OTHERWISE.
2. BOLTS AND ANCHORS:
2.1 BOLTED CONNECTIONS SHALL BE TYPE N (BEARING TYPE WITH THREADS INCLUDED IN SHEAR PLANE) WITH MINIMUM 3/4" DIAMETER A325 BOLTS. SUBMIT PROPOSED BOLT TIGHTENING PROCEDURE FOR REVIEW.
2.2 ANCHOR BOLTS SHALL BE HEADED BOLTS CONFORMING TO ASTM F1554 AND SHALL BE HEADED BOLTS OR THREADED RODS WITH HEAVY HEXAGONAL NUT WELDED TO THE BOTTOM OF THE THREADED ROD, GRADE A36, UNLESS NOTED OTHERWISE.
2.3 EXPANSION ANCHORS SHALL HAVE BEEN EVALUATED BY THE ICC EVALUATION SERVICES, INC. (ICC-ES) WITH A PUBLISHED EVALUATION REPORT. ANCHORS INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 100 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE EQUAL TO 4.5 TIMES THE ANCHOR DIAMETER, UNLESS NOTED OTHERWISE.
2.4 ADHESIVE ANCHORS SHALL CONSIST OF AN ALL-THREAD STEEL ANCHOR WITH ADHESIVE CONFORMING TO ASTM C881-02, TYPE IV, GRADE 3, CLASS A, B, & C EXCEPT GEL TIMES AND EPOXY CONTENT. ADHESIVE SHALL CONSIST OF A TWO COMPONENT ADHESIVE SYSTEM CONTAINED IN SIDE BY SIDE PACKAGING CONNECTED TO A MIXING NOZZLE WHICH THOROUGHLY MIXES THE COMPONENTS AS IT IS INJECTED INTO THE HOLE. ADHESIVE SHALL HAVE PASSED ICC EVALUATION SERVICES, INC. (ICC-ES) ACCEPTANCE CRITERIA 308 FOR LONG TERM CREEP. ANCHORS INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 308 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE EQUAL TO 4.5 TIMES THE ANCHOR DIAMETER, UNLESS NOTED OTHERWISE.
3. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO BOTH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
4. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. CONNECTIONS SHALL BE DETAILED BASED ON THE DESIGN INFORMATION PROVIDED IN THE CONTRACT DOCUMENTS. CONNECTIONS SHALL BE DESIGNED FOR THE SERVICE LOAD REACTION VALUES SHOWN ON THE STRUCTURAL DRAWINGS. FOR STEEL MEMBERS WHOSE REACTIONS ARE NOT SHOWN, THE DESIGN REACTION SHALL BE OBTAINED FROM THE TABLES ENTITLED "MAXIMUM TOTAL UNIFORM LOAD" IN PART 3 OF THE AISC "MANUAL OF STEEL CONSTRUCTION", FOURTEENTH (14TH) EDITION. THE DESIGN REACTION SHALL BE THE TOTAL REACTION VALUE FOR NONCOMPOSITE BEAMS AND EQUAL TO THE TABULATED VALUE FOR COMPOSITE BEAMS. DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT WRITTEN PERMISSION FROM THE DESIGN PROFESSIONAL. DESIGN PROFESSIONAL SHALL BE COMPENSATED BY THE CONTRACTOR FOR THE COST INVOLVED IN THE REDESIGN OF CONNECTIONS FOR THE CONVENIENCE OF THE CONTRACTOR. STEEL CONNECTIONS NOT COMPLETELY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR. THIS DESIGN SERVICE SHALL BE INCLUDED IN THE CONTRACTOR'S SCOPE OF SERVICES. SHOP DRAWINGS AND CALCULATIONS FOR SUCH CONNECTIONS SHALL BE SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE. REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH CONNECTIONS. FOR CONNECTION DETAILS DEPENDING ARRANGEMENT CONCEPTS OF THE CONNECTION, WITHOUT COMPLETE DETAILS, THE CONNECTION DESIGN ENGINEER SHALL FOLLOW THAT ARRANGEMENT CONCEPT IN THE DESIGN. SINGLE ANGLE CONNECTIONS ARE NOT ACCEPTABLE.
5. USE PRE-QUALIFIED WELDED JOINTS IN ACCORDANCE WITH AISC AND THE STRUCTURAL WELDING INSTITUTE. "NON-PRE-QUALIFIED JOINTS" SHALL BE QUALIFIED PRIOR TO FABRICATION.
6. STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE GALVANIZED.

STEEL JOISTS

- 1. STEEL JOISTS, BRIDGING, AND THEIR CONNECTIONS SHALL BE DESIGNED, FABRICATED, AND ERECTED ACCORDING TO THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI).
2. STEEL ROOF JOISTS AND BRIDGING SHALL BE DESIGNED FOR A NET UNIFORM UPLIFT LOAD OF 17 PSF (ASD).
3. BRIDGING SHALL BE DESIGNED TO FULLY BRACE TOP CHORD OF JOISTS UNDER SERVICE LOADS FOR ROOF JOISTS NOT BRACED BY STEEL ROOF DECK.
4. DESIGN OF STEEL JOISTS, BRIDGING, AND THEIR CONNECTIONS SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND CONNECTIONS. CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONNECTIONS SHALL NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR THE DESIGN OF STEEL JOISTS, BRIDGING AND THEIR CONNECTIONS.
5. CONTRACTOR SHALL COORDINATE THE CONSTRUCTION AND ERECTION OF WALLS, BEAM FRAMING, MANUFACTURING, ETC. REINFORCEMENT SHALL BE INSTALLED TO RESIST THE DESIGN MOMENT AND CAMBER OF STEEL JOISTS.

METAL DECK

- 1. DECK DESIGN IS BASED ON THE STEEL DECK IN ACCORDANCE WITH DESIGN MANUAL FOR COMPOSITE DECKS, FORM, FIT AND FUNCTION, 2ND EDITION.
2. PROVIDE ROOF DECK WITH THE FOLLOWING MINIMUM PROPERTIES:
DEPTH 1 1/2"
THICKNESS .186 IN
SECTION MODULUS 175 IN^3
MOMENT OF INERTIA 4000 IN^4
YIELD STRESS 40,000 PSI
3. PROVIDE COMPOSITE FLOOR DECK WITH THE FOLLOWING MINIMUM PROPERTIES:
DEPTH 4"
THICKNESS .342 IN
SECTION MODULUS 418 IN^3
MOMENT OF INERTIA 40,000 IN^4
YIELD STRESS
4. DECK IS SPECIFIED BASED ON A THREE SPAN CONDITION. FURNISH HANGER GAGE DECK IF REQUIRED FOR ONE OR TWO SPAN CONDITIONS.
5. FASTEN ROOF DECK TO RESIST A NET UPLIFT OF 20 PSF (ASD) OR AS INDICATED ON THE DRAWINGS.
6. FASTEN ROOF DECK TO RESIST A DIAPHRAGM SHEAR FORCE OF 340 POUNDS PER LINEAR FOOT (ASD).

COMPOSITE FLOOR CONSTRUCTION

- 1. COMPOSITE FLOOR MEMBERS ARE DESIGNED TO BE UNSHORED UNLESS NOTED OTHERWISE.
2. COMPOSITE FLOOR SLABS ARE TO BE FINISHED LEVEL. THE WEIGHT OF THE WET CONCRETE WILL CAUSE DEFLECTIONS OF THE STEEL FRAMING AND DECKING. THUS, CONCRETE OVERRUNS ARE TO BE ANTICIPATED AND INCLUDED IN THE CONTRACTOR'S BASE BID. THE FLOOR SYSTEM HAS BEEN DESIGNED FOR A MAXIMUM DEFLECTION CONCRETE ALLOWANCE OF 3/4 INCH. DO NOT EXCEED SPECIFIED ALLOWANCE WITHOUT APPROVAL FROM THE STRUCTURAL ENGINEER.
3. COORDINATE EMBEDDED ITEMS REQUIRED FOR ARCHITECTURAL, STRUCTURAL, AND MECHANICAL ELEMENTS. OBTAIN WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER FOR PLACEMENT OF EMBEDDED ITEMS, RECESSES OR DEPRESSIONS IN THE SLAB NOT SHOWN ON THE STRUCTURAL DOCUMENTS.
4. CONDUIT, PIPE, OR DUCT CAST WITHIN COMPOSITE SLAB SHALL NOT EXCEED 1 INCH IN OUTSIDE DIAMETER NOR ONE THIRD THE CONCRETE COVER THICKNESS. SHALL BE PLACED WITHOUT CROSSOVERS, SHALL HAVE AT LEAST 3/4 INCH OF CONCRETE COVER AND SHALL BE PLACED AT LEAST 19 INCHES APART UNLESS SPECIFICALLY DETAILED AND SHOWN IN STRUCTURAL DOCUMENTS. THE STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR THE EFFECT OF CONDUIT, PIPE, OR DUCT NOT SPECIFICALLY SHOWN IN THE STRUCTURAL DOCUMENTS ON THE FULFILLMENT OF GOVERNING CODE REQUIREMENTS AND SPECIFIED FIRE RATINGS BY THE COMPOSITE SLAB SYSTEM.
5. PLACEMENT OF CONTROL JOINTS IN THE COMPOSITE FLOOR SLAB IS PROHIBITED.



PRESS BOX EXPANSION
RIVERSIDE MILITARY ACADEMY
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Table with 3 columns: No., DATE, DESCRIPTION. Row 1: 12/20/2019, BID & PERMIT SET.

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GENERAL NOTES

Sheet No. S-0.01
RELEASED FOR CONSTRUCTION