GENERAL

1. THE CONTRACTOR SHALL VERIFY ALL EXISTING SITE CONDITIONS BEFORE STARTING WORK. PROMPTLY NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BETWEEN ACTUAL SITE CONDITIONS AND THE CONTRACT DOCUMENTS.
2. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. THE ERECTION PROCEDURE AND SEQUENCE INCLUDING THE DESIGN ADEQUACY AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC., IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
3. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY FOR JOBSITE SAFETY AND FOR COMPLYING WITH ALL HEALTH AND SAFETY PRECAUTIONS AS REQUIRED BY ANY REGULATORY AGENCY. IN PERFORMING CONSTRUCTION OBSERVATION VISITS TO THE JOBSITE, THE STRUCTURAL ENGINEER WILL HAVE NO CONTROL OVER, OR RESPONSIBILITY FOR, THE CONTRACTOR'S MEANS, METHODS, SEQUENCES, TECHNIQUES, OR PROCEDURES IN PERFORMING THE WORK.
4. THE CONTRACTOR SHALL COORDINATE THE ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING, AND ELECTRICAL REQUIREMENTS WITH THE STRUCTURAL DRAWINGS, INCLUDING THE LOCATION OF MISCELLANEOUS ITEMS AFFECTING THE STRUCTURAL WORK SUCH AS OPENINGS, BENT PLATES, INSERTS, ETC. PROMPTLY NOTIFY THE ARCHITECT OF ANY DISCREPANCIES OR OMISSIONS.
5. THE CONTRACTOR SHALL VERIFY AND COORDINATE ALL FLOOR AND ROOF MOUNTED MECHANICAL EQUIPMENT DIMENSIONS AND WEIGHTS, AND VERIFY ALL FLOOR AND ROOF OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS AND APPROVED SHOP DRAWINGS. PROMPTLY NOTIFY THE ARCHITECT IF ACTUAL EQUIPMENT WEIGHTS AND/OR LOCATIONS VARY FROM THOSE INDICATED IN THE CONTRACT DOCUMENTS.
6. SHOP DRAWINGS WILL NOT BE REVIEWED BY THE DESIGNER UNTIL AFTER THE CONTRACTOR AND SUB-CONTRACTORS HAVE THOROUGHLY REVIEWED, VERIFIED EXISTING CONDITIONS, COORDINATED THE SHOP DRAWINGS WITH AFFECTED TRADES, AND APPROVED SHOP DRAWINGS PRIOR TO SUBMITTING TO THE ARCHITECT. SUBMIT ONE COPY OF REVIEWED DRAWINGS ELECTRONICALLY. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
7. SHOULD ANY OF THE INSTRUCTIONS SHOWN ON THE DRAWINGS CONFLICT WITH THE STRUCTURAL GENERAL NOTES, THE SPECIFICATIONS, OR WITH EACH OTHER, PROMPTLY NOTIFY THE ARCHITECT FOR CLARIFICATION PRIOR TO PROCEEDING WITH THE AFFECTED WORK.

DELEGATED DESIGN OF BUILDING COMPONENTS

1. DESIGN OF CERTAIN BUILDING COMPONENTS THAT ARE NOT CONSIDERED PART OF THE PRIMARY STRUCTURAL SYSTEM IS DELEGATED TO BE COMPLETED BY A SPECIALTY STRUCTURAL ENGINEER RETAINED BY THE CONTRACTOR. THESE COMPONENTS MAY INCLUDE BUT ARE NOT LIMITED TO NON-LOAD-BEARING COLD-FORMED METAL CURTAIN WALL FRAMING, STAIRS, AND DESIGNATED EQUIPMENT SUPPORTS. THE DESIGN SHALL INCLUDE THE CONNECTIONS TO THE PRIMARY BUILDING FRAME WHERE APPLICABLE.
2. THE SPECIALTY STRUCTURAL ENGINEER SHALL DESIGN THE COMPONENT(S) AND ASSOCIATED CONNECTIONS, FOR THE LOADS AND DEFLECTION REQUIREMENTS INDICATED. THE REVIEW OF THE SHOP DRAWING SUBMITTAL BY THE STRUCTURAL ENGINEER SHALL ONLY BE TO VERIFY COMPLIANCE WITH DESIGN INTENT. APPLICATION OF LOADS SPECIFIED, AND REVIEW OF THE PRIMARY BUILDING FRAME TO RESIST THE LOADS IMPOSED BY THE COMPONENT CONNECTIONS.
3. EXTERIOR CURTAIN WALL SYSTEMS, DEFLECTION CLIPS AT COLD-FORMED METAL FRAMING, AND HEAD JOINTS AT CURTAIN WALL AND STOREFRONT SYSTEMS SHALL ACCOMMODATE 1/2" OF VERTICAL STRUCTURAL DEFLECTION AND STORY DRIFT OF H/300 IN ADDITION TO THERMAL MOVEMENT OF THE SYSTEM.

DESIGN AND CODE INFORMATION

1. ALL CONSTRUCTION SHALL BE IN CONFORMANCE WITH THE 2018 KENTUCKY BUILDING CODE (BASED ON THE 2015 INTERNATIONAL BUILDING CODE).
2. BUILDING RISK CATEGORY — IV
3. WIND DESIGN DATA

A.	BASIC WIND SPEED, V_{ULT} (V_{ASD})	120 MPH (104 MPH)
B.	EXPOSURE CATEGORY	C
C.	INTERNAL PRESSURE COEFF. (ENCLOSED BUILDING)	+/- 0.18
D.	TOPOGRAPHIC FACTOR, Kz	1.0

4. SEISMIC DESIGN DATA

A.	SEISMIC DESIGN CATEGORY	A
B.	SEISMIC IMPORTANCE FACTOR	1.5
C.	SITE CLASS	B
D.	SPECTRAL RESPONSE ACCELERATION (S_s)	0.198
E.	SPECTRAL RESPONSE ACCELERATION (S_1)	0.080
F.	SPECTRAL RESPONSE COEFFICIENT (S_{ds})	0.132
G.	SPECTRAL RESPONSE COEFFICIENT (S_{d1})	0.080
H.	BASIC SEISMIC FORCE-RESISTING SYSTEM: STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE	
I.	RESPONSE MODIFICATION FACTOR (R)	3.0
J.	DEFLECTION AMPLIFICATION FACTOR (C_d)	3.0
K.	ANALYSIS PROCEDURE	EQUIV. LATERAL FORCE
L.	SEISMIC RESPONSE COEFFICIENT C_s	0.03
M.	SEISMIC BASE SHEAR	1.0 KIPS

5. SNOW DESIGN DATA

A.	GROUND SNOW LOAD (P_g)	15 PSF
B.	FLAT ROOF SNOW LOAD (P_f)	18 PSF (PLUS RAIN ON SNOW)
C.	SNOW IMPORTANCE FACTOR	1.2
D.	SNOW EXPOSURE FACTOR (C_e)	1.0
E.	SNOW THERMAL FACTOR (C_t)	1.0

6. LIVE LOADS DESIGN DATA (WITH LIVE LOAD REDUCTION WHERE APPLICABLE)

A.	FIRST FLOOR	100 PSF
B.	ROOF	30 PSF

SPECIAL INSPECTION AND TESTING

1. THE OWNER SHALL EMPLOY AN INDEPENDENT TESTING COMPANY TO PERFORM SITE INSPECTIONS AND TESTING IN ACCORDANCE WITH THE STATEMENT OF STRUCTURAL SPECIAL INSPECTIONS AND QC TESTING ON SHEET S0.2.

EXISTING BUILDINGS

1. INFORMATION REGARDING STRUCTURAL MEMBERS INDICATED TO BE EXISTING HAS BEEN OBTAINED FROM EXISTING DRAWINGS AND/OR LIMITED VISUAL OBSERVATIONS, THEREFORE ACTUAL CONDITIONS MAY DIFFER FROM THOSE INDICATED. IF THE CONTRACTOR SHALL IDENTIFY THESE DISCREPANCIES AND STRUCTURAL ENGINEER IN ORDER THAT THEY MAY BE CORRECTLY RESOLVED.
2. THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO THE CONSTRUCTION AND FABRICATION OF ANY NEW STRUCTURAL MEMBERS.

SUSPENDED MECHANICAL, ELECTRICAL, AND OTHER LOADS

1. THE TOTAL OF LOADS SUPPORTED DIRECTLY FROM ELEVATED SLABS ON METAL DECK SHALL NOT EXCEED 1,000 POUNDS IN ANY 10 FT. BY 10 FT. AREA. SEE MECHANICAL, PLUMBING, ELECTRICAL, AND ARCHITECTURAL DRAWINGS FOR HANGER AND BRACING REQUIREMENTS, ETC.
2. SUSPENDED CEILINGS AND TYPICAL LIGHT FIXTURES MAY BE SUPPORTED FROM THE ROOF DECK. ADDITIONAL LOADS FOR SMALL DUCTWORK, CONDUITS, AND MISCELLANEOUS LOADS MAY BE HUNG FROM THE ROOF DECK BUT SHALL NOT EXCEED 15 PSF, WITH NO INDIVIDUAL CONCENTRATED LOAD EXCEEDING 75 LBS. PLUMBING, FIRE PROTECTION, OR OTHER PIPING IS NOT PERMITTED TO BE HUNG FROM THE ROOF DECK. PROPOSED FASTENERS FOR ATTACHING TIE WIRES TO ROOF DECK MUST BE SUBMITTED FOR REVIEW PRIOR TO CONSTRUCTION. MECHANICAL, PLUMBING, AND ELECTRICAL CONTRACTOR(S) SHALL BE RESPONSIBLE FOR DESIGNING, PROVIDING, AND INSTALLING ADDITIONAL FRAMING SUPPORTS REQUIRED TO TRANSFER LOADS TO BEAMS.

FOUNDATION / EARTHWORK FOR STRUCTURES

1. FOUNDATION DESIGN IS BASED ON RECOMMENDATIONS PROVIDED IN A REPORT PREPARED BY TERRACON CONSULTANTS, INC. DATED AUGUST 31, 2010 (REPORT NO. 57105241).
2. FOUNDATION SYSTEM CONSISTS OF CONVENTIONAL SHALLOW SPREAD FOOTINGS AND CONTINUOUS WALL FOOTINGS DESIGNED TO BEAR ON SOIL CAPABLE OF SUPPORTING 2,500 PSF UNDER STATIC (GRAVITY) LOADS AND 3,300 PSF UNDER TRANSIENT (WIND AND SEISMIC) LOADS. WHERE SOFT SOILS ARE ENCOUNTERED, THE FOOTING SUBGRADE SHOULD BE UNDERCUT TO A FIRM STRATUM AND REPLACED WITH ENGINEERED FILL OR LEAN CONCRETE. THE BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE 2'-6" MINIMUM BELOW FINISHED GRADE FOR FROST PROTECTION.
3. NO FOOTINGS SHALL BEAR ON ROCK. IF ROCK IS ENCOUNTERED WITHIN 2'-0" FROM THE BOTTOM OF THE FOOTING, IT SHALL BE UNDERCUT TO A MINIMUM OF 2'-0" BELOW THE BOTTOM OF THE FOOTING AND REPLACED WITH COMPACTED FILL IN CONFORMANCE WITH THE GEOTECHNICAL REPORT AND CIVIL/SITE SPECIFICATIONS.
4. ALL SITE PREPARATION ACTIVITIES WITHIN THE BUILDING LIMITS AND EXTENDING 10 FEET BEYOND SHALL BE MONITORED AND VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER LICENSED IN THE PROJECT STATE. THE GEOTECHNICAL ENGINEER SHALL VERIFY THAT ACTUAL SITE CONDITIONS ARE CONSISTENT WITH THE GEOTECHNICAL REPORT, THAT FILL PLACEMENT AND COMPACTION, UNDERCUTTING, PROOFROLLING, ETC., IS IN GENERAL CONFORMANCE WITH THE GEOTECHNICAL REPORT AND CIVIL/SITE SPECIFICATIONS, AND THAT THE DESIGN CAPACITIES ARE OBTAINED.
5. THE DESIGN ADEQUACY AND SAFETY OF TEMPORARY RETAINING STRUCTURES, SHORING OF TRENCHES, ETC., REQUIRED TO CONSTRUCT THE FOUNDATION SYSTEM IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
6. DRAINAGE SHALL BE MAINTAINED AROUND THE FOUNDATION AT ALL TIMES, BOTH DURING AND AFTER CONSTRUCTION. SURFACE WATER SHALL FLOW RAPIDLY AWAY FROM THE FOUNDATION AT ALL TIMES. REFER TO CIVIL DRAWINGS FOR FINAL GRADING.
7. THE CRUSHED STONE BASE BELOW SLABS-ON-GRADE SHALL CONFORM TO THE KENTUCKY TRANSPORTATION CABINET (KTC) "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION", 2012 EDITION, FOR DENSE GRADED AGGREGATE OR CRUSHER RUN AGGREGATE.
8. THE ZONE IMMEDIATELY BEHIND RETAINING WALLS, FOUNDATION WALLS, AND ELEVATOR PIT WALLS SHALL CONSIST OF AN OPEN-GRADED STONE SUCH AS NO. 57, PER ASTM D448, EXTENDING FROM THE OUTER CORNER OF HEEL UPWARD AT 1H:1V, U.N.O. THE STONE SHALL BE DENSIFIED WITH A VIBRATORY SLED AND PLACED IN LIFTS NOT EXCEEDING 16 INCHES.
9. UNDERGROUND UTILITIES SHALL NOT BE LOCATED UNDER OR PENETRATE COLUMN OR WALL FOOTINGS, AND SHALL BE LOCATED OUTSIDE THE FOOTING BEARING INFLUENCE ZONE (ASSUME SLOPE 1.5 (H) : 1 (V) DOWN FROM EXTENTS OF FOOTING BASE), EXCEPT, UTILITY LINES ENTERING THE BUILDING MAY CROSS PERPENDICULAR UNDER EXTERIOR WALL FOOTINGS. SEE SPECIFICATIONS FOR TRENCH BACKFILL REQUIREMENTS.

CONCRETE

1. CONCRETE WORK SHALL BE IN CONFORMANCE WITH THE "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE," (ACI 318-14).
2. MAXIMUM CONCRETE SLUMP SHALL BE 4 INCHES WITHOUT A HIGH-RANGE WATER-REDUCING ADMIXTURE (HRWR) OR 8 INCHES WITH HRWR AT POINT OF DEPOSIT.
3. SIZE OF AGGREGATE AND GRADATION SHALL CONFORM TO ASTM C33 FOR NORMAL WEIGHT CONCRETE.
4. THE COMBINED TOTAL MASS PERCENT OF DELETERIOUS SUBSTANCES, INCLUDING, BUT NOT LIMITED TO, COAL AND LIGNITE, FOR BOTH COARSE AND FINE AGGREGATES SHALL BE LIMITED TO 0.5% AS DETERMINED BY ASTM C123.
5. ALL CONCRETE SHALL BE MECHANICALLY VIBRATED DURING PLACEMENT.
6. CHAMFER EXPOSED CORNERS OF WALLS 3/4 INCH, U.N.O.
7. THE STRUCTURAL ENGINEER SHALL APPROVE THE LOCATION OF ALL CONSTRUCTION JOINTS NOT LOCATED ON THE DRAWINGS.
8. ALL SLAB-ON-GRADE SAW CUT CONTROL JOINTS SHALL BE MADE PER THE ACI 301 EARLY-ENTRY DRY-CUT PROCESS. SEE DETAILS FOR ADDITIONAL CONTROL JOINT REQUIREMENTS.
9. ALL REINFORCING STEEL AND ITEMS TO BE EMBEDDED IN CONCRETE (INCLUDING ANCHOR RODS) SHALL BE LOCATED AND SECURELY TIED IN PLACE PRIOR TO PLACING CONCRETE. "WET-SETTING" OF REINFORCING STEEL OR EMBEDDED ITEMS IN FLUID CONCRETE IS NOT PERMITTED AND SHALL BE CAUSE FOR REJECTION OF INSTALLED WORK.
10. THE CONTRACTOR SHALL VERIFY THE COMPATIBILITY AND ALLOWABLE USE OF CONCRETE CURING METHODS WITH APPLICABLE FLOORING MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS. THE USE OF CURING COMPOUNDS IS NOT PERMITTED, SEE SPECIFICATIONS.
11. THE CONTRACTOR SHALL VERIFY THE COMPATIBILITY AND ALLOWABLE USE OF CONCRETE CURING METHODS WITH APPLICABLE FLOORING MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS. THE USE OF CURING COMPOUNDS IS NOT PERMITTED, SEE SPECIFICATIONS.
12. CONCRETE SHALL BE MAINTAINED ABOVE 50°F AND IN A MOIST CONDITION FOR AT LEAST THE FIRST 7 DAYS AFTER PLACEMENT. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
13. WHEN AIR TEMPERATURE HAS FALLEN TO OR IS EXPECTED TO FALL BELOW 40°F AT ANY TIME DURING THE FIRST THREE DAYS FOLLOWING PLACEMENT, MAINTAIN CONCRETE MIXTURE TEMPERATURE WITHIN THE TEMPERATURE RANGE REQUIRED BY ACI 301.
14. REFER TO ARCHITECTURAL OR LANDSCAPE DRAWINGS FOR LOCATIONS AND EXTENTS OF SLAB RECESSES REQUIRED FOR FINISHES, UNLESS NOTED ON S1.1 PLAN.
15. REFER TO ARCHITECTURAL DRAWINGS FOR FLOOR FINISHES.
16. CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES:

CONCRETE USE:	UNIT WEIGHT (PCF)	28-DAY STRENGTH (PSI)	MAXIMUM W/C RATIO	CEMENT TYPE	MAX. % FLY ASH	MAX. AGG. SIZE	AIR %
FDS. AND WALLS	NWC 140-150	4,000	0.45	I OR II	25%	1"	6% ± 1.5%
SLAB-ON-GRADE	NWC 140-150	4,000	0.45	I OR II	25%	1"	3% MAX.

REINFORCING STEEL

1. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 (GR. 60) U.N.O.
2. REINFORCING STEEL SHALL BE SPLICED ONLY WHERE SHOWN OR NOTED. THE STRUCTURAL ENGINEER SHALL APPROVE SPLICES AT OTHER LOCATIONS.
3. ALL REINFORCING STEEL SHALL BE CONTINUOUS AND SPLICED WITH CLASS "B" TENSION LAP SPLICES IN CONFORMANCE WITH ACI 318-14 U.N.O.
4. PROVIDE THE FOLLOWING CLEAR CONCRETE COVER FOR PLACEMENT OF REINFORCING STEEL, UNLESS OTHERWISE DETAILED.

A.	FOOTINGS:	2" AT FORMED EDGES 3" CAST AGAINST GROUND
B.	WALLS:	2" AT FORMED EDGES
C.	SLABS-ON-GRADE:	1-1/2" FROM TOP OF SLAB, U.N.O.

5. PROVIDE CORNER BARS TO MATCH LONGITUDINAL REINFORCING AT ALL CORNERS AND INTERSECTIONS OF ALL FOOTINGS AND WALLS, UNLESS OTHERWISE DETAILED.
6. PROVIDE DOWELS IN FOOTINGS THE SAME SIZE AND NUMBER AS VERTICAL WALL OR COLUMN REINFORCING. SECURELY TIE DOWELS INTO PLACE PRIOR TO PLACING CONCRETE. "WET-SETTING" DOWELS IS NOT PERMITTED.

REINFORCING STEEL (CONT'D)

7. ALL OPENINGS IN CONCRETE FOUNDATION WALLS UP TO 4'-0" X 4'-0" SQUARE OR 4'-0" IN DIAMETER SHALL BE REINFORCED WITH (2)-#4 BARS ON ALL SIDES AND EXTENDING 2'-6" MINIMUM BEYOND THE EDGE OF OPENING, U.N.O.
8. REINFORCING BARS AND ACCESSORIES SHALL NOT BE IN CONTACT WITH ANY PIPE, PIPE FLANGE, CONDUIT, OR METAL PARTS EMBEDDED IN CONCRETE. A MINIMUM OF 2 INCHES CLEARANCE SHALL BE PROVIDED AT ALL TIMES.
9. STEEL REINFORCEMENT FABRICATION AND PLACEMENT SHOP DRAWINGS SHALL CONTAIN ALL NECESSARY INFORMATION FOR THE INSTALLER TO ACCURATELY INSTALL REINFORCEMENT. SHOP DRAWINGS MUST INCLUDE ALL NECESSARY DIMENSIONS, PLANS, SECTIONS, WALL ELEVATIONS, SPLICE AND LAP LENGTHS, REQUIRED CONCRETE COVER, ETC. REQUIRED FOR COMPLETE INSTALLATION.

POST-INSTALLED ANCHORS

1. ALL POST-INSTALLED ANCHORS IN CONCRETE AND CMU ARE TO BE INSTALLED IN STRICT CONFORMANCE WITH THE MANUFACTURER'S RECOMMENDATIONS, INCLUDING BUT NOT LIMITED TO DRILL BIT SIZE, PROPER CLEANING OF HOLES, INSTALLATION TORQUE, AND TEMPERATURE CONSTRAINTS.
2. WHEN A SPECIFIC PRODUCT AND MANUFACTURER IS REFERENCED IN THE CONTRACT DOCUMENTS, THAT SPECIFIC PRODUCT SHALL BE USED UNLESS THE CONTRACTOR SUBMITS A REQUEST FOR A PRODUCT SUBSTITUTION OF AN ANCHOR WITH EQUIVALENT RESISTANCE VALUES IN THE APPROPRIATE BASE MATERIAL. ALL REQUESTS FOR SUBSTITUTION SHALL INCLUDE PRODUCT SPECIFICATIONS AND DESIGN DATA FOR REVIEW BY THE STRUCTURAL ENGINEER.
3. THE ANCHOR MANUFACTURER'S REPRESENTATIVE SHALL BE PRESENT DURING THE INITIAL INSTALLATION OF EACH TYPE OF ANCHOR TO REVIEW AND APPROVE OF THE CONTRACTOR'S INSTALLATION PROCEDURES. THE OWNER'S TESTING AGENCY SHALL ALSO OBSERVE THE INITIAL INSTALLATION OF EACH ANCHOR TYPE, AND PROVIDE THE INSPECTION OF ALL ANCHORS DURING INSTALLATION TO VERIFY CONFORMANCE WITH THE MANUFACTURER'S INSTALLATION RECOMMENDATIONS.
4. CONCRETE CHEMICAL ANCHORING SYSTEMS GENERICALLY REFERRED TO AS "EPOXY" ANCHORING SYSTEMS SHALL BE ONE OF THE FOLLOWING:
   A. "HIT-HY 200 SAFE SET SYSTEM" EPOXY ADHESIVE SYSTEM BY HILTI
   B. "SET-3G" HIGH-STRENGTH EPOXY ADHESIVE BY SIMPSON STRONG-TIE ANCHOR SYSTEMS
5. THREADED ANCHOR RODS USED WITH THE CHEMICAL ANCHORING SYSTEMS SHALL BE STANDARD STRENGTH STEEL RODS (ASTM A36) U.N.O. REINFORCING STEEL USED WITH THESE SYSTEMS SHALL COMPLY WITH ASTM A615 (GR. 60).
6. FASTENERS GENERICALLY REFERRED TO AS "SCREW ANCHOR" ON THE DRAWINGS SHALL BE ONE OF THE FOLLOWING:
   A. "TITEN HD" BY SIMPSON STRONG-TIE ANCHOR SYSTEMS
   B. "KWIK HUS-EZ" (KH-EZ) BY HILTI
7. USE MATCHING BIT PROVIDED BY MANUFACTURER FOR INSTALLATION.
8. FASTENERS GENERICALLY REFERRED TO AS "P.A.F." OR POWER-ACTUATED FASTENERS SHALL BE "PDP" FASTENER BY SIMPSON STRONG-TIE ANCHOR SYSTEMS.

STRUCTURAL STEEL

1. STRUCTURAL STEEL FABRICATOR SHALL BE AISC CERTIFIED.
2. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN CONFORMANCE WITH THE AISC "MANUAL OF STEEL CONSTRUCTION", 14TH EDITION.
3. STRUCTURAL STEEL ROLLED SHAPES SHALL CONFORM TO ASTM A992, U.N.O. STEEL COLUMN BASE PLATES SHALL CONFORM TO ASTM A36, U.N.O. ANGLES, CHANNELS, AND ALL OTHER MISCELLANEOUS STEEL SHALL CONFORM TO ASTM A36, U.N.O.
4. HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A500, GRADE B.
5. PIPE MEMBERS SHALL CONFORM TO ASTM A53, TYPE E OR S, GR. B OR ASTM A500 GRADE C.
6. STEEL FRAMING CONNECTIONS MAY BE WELDED OR BOLTED. BOLTED CONNECTIONS SHALL BE FABRICATING 3/4" MINIMUM DIAMETER "TWIST-OFF TYPE TENSION CONTROL BOLT ASSEMBLIES" CONFORMING TO ASTM F1852, TYP., U.N.O. WELDED CONNECTIONS SHALL BE COMPLETED BY CERTIFIED WELDERS (FOR WELD TYPE AND POSITION) AND SHALL CONFORM TO AWS SPECIFICATIONS. ALL WELDING SHALL BE E70XX ELECTRODES.
7. ANCHOR RODS SHALL BE THREADED AND SHALL CONFORM TO ASTM F554, GRADE 36, U.N.O. MINIMUM ANCHOR ROD EMBEDMENT SHALL BE 12 BOLT DIAMETERS, U.N.O. CLEAN ANCHOR RODS OF ALL GREASE, DIRT, ETC., BEFORE INSTALLATION.
8. COLUMN BASE PLATE GROUT SHALL CONFORM TO ASTM C 1107, FACTORY-PACKAGED, NON-METALLIC AGGREGATE GROUT, NON-CORROSIVE AND NON-STAINING, MIXED WITH WATER TO CONSISTENCY SUITABLE FOR APPLICATION AND A 30 MINUTE WORK TIME.
9. ALL STEEL ELEMENTS AND CONNECTIONS EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED. CLEAN WELDS AND AREAS WHERE GALVANIZING IS DAMAGED OR MISSING AND REPAIR WITH GALVANIZING REPAIR PAINT MEETING ASTM A 780.
10. THE STEEL FABRICATOR SHALL PROVIDE CONNECTIONS FOR BEAMS AS SHOWN IN DETAILS 1/S3.0.
    A. THE STEEL FABRICATOR SHALL VERIFY THAT ALL BEAM AND GIRDER CONNECTION DETAILING MEETS OR EXCEEDS MINIMUM EDGE DISTANCES, BOLT SPACING, AND OTHER DESIGN ASSUMPTIONS LISTED IN THE CONNECTION DETAILS.
    B. IF THE FABRICATOR CHOOSES TO DETAIL AND FABRICATE THE CONNECTIONS OTHER THAN AS SPECIFIED ABOVE, THE FABRICATOR MUST SUBMIT CALCULATIONS DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROJECT STATE, ALONG WITH COORDINATED SHOP DRAWINGS FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. REVIEW AND APPROVAL OF CONNECTION ALTERNATES BY THE STRUCTURAL ENGINEER FOR THE PROJECT WILL BE PERFORMED AS AN ADDITIONAL COST TO THE FABRICATOR/CONTRACTOR.

COLD-FORMED METAL FRAMING

1. ALL COLD-FORMED METAL STUDS AND/OR JOISTS AND ACCESSORIES SHALL BE OF THE TYPE, SIZE, AND MINIMUM GAGE INDICATED.
2. A SPECIALTY STRUCTURAL ENGINEER LICENSED IN THE PROJECT STATE SHALL DESIGN THE NON-LOAD-BEARING CURTAIN WALL FRAMING INCLUDING THE ANCHORAGE TO THE PRIMARY STRUCTURAL FRAME AND SHALL PROVIDE SEALED CALCULATIONS AND SEALED SHOP DRAWINGS.
3. ALL STRUCTURAL MEMBERS SHALL BE DESIGNED ACCORDING TO THE AISI "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS."
4. ALL STRUCTURAL MEMBERS SHALL BE FORMED FROM G90 ZINC COATED STEEL, CORRESPONDING TO THE REQUIREMENTS OF ASTM A653.
5. THE FRAMING INSTALLER IS TO ENSURE PUNCHOUT ALIGNMENT WHEN ASSEMBLING LATERAL BRACING AND FIELD CUTTING STUDS TO LENGTH. LATERAL BRACING MUST BE INSTALLED AT THE TIME OF WALL ERECTION.
6. STUDS SHALL BE CUT SQUARE AND PLACED TIGHT TO TRACKS WITH NO GAPS, EXCEPT AT DEFLECTION CLIP LOCATIONS.
7. FASTENING OF COMPONENTS SHALL BE WITH SELF-DRILLING SCREWS OF SUFFICIENT SIZE TO ENSURE THE STRENGTH OF THE CONNECTIONS. WIRE TYING OF COMPONENTS IS NOT PERMITTED. SPLICES IN FRAMING COMPONENTS OTHER THAN THE RUNNER TRACK ARE NOT PERMITTED.
8. NON-LOAD-BEARING CURTAIN WALL FRAMING SHALL COMPLY WITH THE FOLLOWING:
   A. NON-LOAD-BEARING STUDS SHALL BE AS REQUIRED BY DESIGN BUT NOT LESS THAN 18 GAGE. HEAVIER GAGE STUDS AND/OR REDUCED STUD SPACING BUILDING CORNERS MAY BE NECESSARY AS REQUIRED BY DESIGN TO ACCOMMODATE END-ZONE WIND PRESSURES.
   B. STRUCTURAL TRACKS AT EXTERIOR WINDOW HEADS AND SILLS SHALL BE 16 GA. MINIMUM.
   C. INSTALLATION OF CURTAIN WALL FRAMING SHALL ACCOMMODATE DEFLECTION OF PRIMARY STRUCTURAL FRAMING USING DEFLECTION CLIPS AS INDICATED. DESIGN FRAMING SYSTEM TO ACCOMMODATE UPWARD AND DOWNWARD MOVEMENT OF PRIMARY STRUCTURE AS INDICATED. DEFLECTION TRACKS ARE NOT PERMITTED.
   D. CURTAIN WALL FRAMING SHALL BE DESIGNED FOR HORIZONTAL WIND PRESSURES PER THE GOVERNING BUILDING CODE INCLUDING COMPONENTS AND CLADDING LOADING, BUT NOT LESS THAN 30 PSF ALLOWABLE AT INTERIOR AND EXTERIOR ZONES RESPECTIVELY. DESIGN CURTAIN WALL FRAMING TO ACCOMMODATE HORIZONTAL DEFLECTION WITHOUT REGARD FOR CONTRIBUTION OF SHEATHING MATERIAL.
   E. LIMIT OUT-OF-PLANE DEFLECTION TO L/600 AT L/175 WITH BRICK VENEER OR H/360 AT WALLS WITH NON-BRICK VENEER.
   F. CURTAIN WALL FRAMING SHALL BE DESIGNED TO SUPPORT SELF WEIGHT PLUS THE WEIGHT OF ALL FINISH MATERIALS AND GLAZING, INCLUDING DEAD LOADS AT VERTICAL GLAZING MULLIONS.
   G. BOX HEADERS SHALL BE PROVIDED OVER ALL WALL OPENINGS.

STEEL DECK

1. STEEL DECK SHALL CONFORM TO THE SPECIFICATIONS OF THE STEEL DECK INSTITUTE, LATEST EDITION.
2. STEEL DECK TYPE, DEPTH, THICKNESS, AND FINISH SHALL BE AS SPECIFIED ON STRUCTURAL DRAWINGS.
3. STEEL DECK SHALL HAVE THE FOLLOWING MINIMUM YIELD STRENGTHS:
   A. ROOF DECK — 33 KSI
4. DECK SUPPLIER SHALL PROVIDE ALL NECESSARY DECK ACCESSORIES, INCLUDING DECK END CLOSURES, GIRDER CLOSURE STRIPS, COLUMN CLOSURES, ETC.
   A. STEEL DECK SHALL BE FURNISHED IN SHEET LENGTHS SUFFICIENT TO EXTEND OVER FOUR SUPPORTS (3 SPANS) WHEREVER POSSIBLE.

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clark regional MEDICAL CENTER

CONSTRUCTION DRAWINGS

GENERAL NOTES
CATH LAB SUITE ADDITION
CLARK REGIONAL MEDICAL CENTER
WINCHESTER, KENTUCKY

WILLIAM J. O'DONNELL 18724 LICENSED PROFESSIONAL ENGINEER

30 DECEMBER 2019
CRM1807

S0.0

SSE PROJECT NO. 11316
SSE FIRM NO. 1492