

EXTERIOR WALL PRESSURES (ULTIMATE)				
AREA	ZONE [4] (PSF)	ZONE [5] (PSF)	ZONE [6] (PSF)	ZONE [7] (PSF)
10 SF	-38	+36	-47	+36
20 SF	-37	+34	-44	+34
50 SF	-35	+32	-40	+32
100 SF	-33	+30	-37	+30
200 SF	-32	+29	-34	+29
500 SF	-30	+27	-30	+27

NOTES:

- DESIGN WIND PRESSURES ARE BASED UPON DESIGN AND CODE INFORMATION IN THE GENERAL NOTES.
- ROOF UPLIFT WIND PRESSURES IN ZONES 1, 2 AND 3 ARE GROSS UPLIFT VALUES. NET UPLIFT PRESSURES SHALL BE CONSIDERED EQUAL TO GROSS PRESSURES.
- TABULATED WIND PRESSURES SHALL BE USED IN THE DESIGN OF EXTERIOR COMPONENT AND CLADDING MATERIALS. INTERPRETATION OF THE BUILDING AREAS SHALL BE THE RESPONSIBILITY OF THE EXTERIOR COMPONENT AND CLADDING MATERIAL SUPPLIER.

COMPONENT & CLADDING WALL PRESSURES

ROOF UPLIFT PRESSURES (ULTIMATE)					
AREA	ZONE [1] (PSF)	ZONE [2] (PSF)	ZONE [3] (PSF)	ZONE [4] (PSF)	ZONE [5] (PSF)
10 SF	-39	+16	-65	+36	-65
20 SF	-37	+16	-56	+33	-56
50 SF	-36	+16	-49	+32	-49
100 SF	-36	+16	-42	+30	-42

NOTE: + INDICATES POSITIVE PRESSURE
- INDICATES NEGATIVE PRESSURE

PARAPET PRESSURES (ULTIMATE)		
WINDWARD	ZONE [4] (PSF)	ZONE [5] (PSF)
10 SF	-65	+36
LEEWARD	10 SF	-38

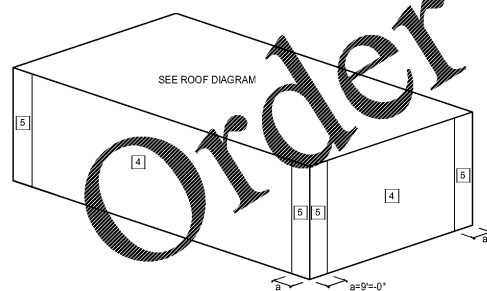
COMPONENT & CLADDING ROOF PRESSURES



a 10 PERCENT OF LEAST HORIZONTAL DIMENSION OR 0.4h, WHICHEVER IS SMALLER, BUT NOT LESS THAN EITHER 4% OF THE LEAST HORIZONTAL DIMENSION OR 3t

b MEAN ROOF HEIGHT, IN FEET, EXCEPT THAT EAVE HEIGHT SHALL BE USED FOR ROOF ANGLE(S) ≤ 10°

ROOF DIAGRAM



a 10 PERCENT OF LEAST HORIZONTAL DIMENSION OR 0.4h, WHICHEVER IS SMALLER, BUT NOT LESS THAN EITHER 4% OF THE LEAST HORIZONTAL DIMENSION OR 3t

b MEAN ROOF HEIGHT, IN FEET, EXCEPT THAT EAVE HEIGHT SHALL BE USED FOR ROOF ANGLE(S) ≤ 10°

WALL DIAGRAM

STRUCTURAL GENERAL NOTES

DESIGN AND CODE INFORMATION

- ALL CONSTRUCTION SHALL CONFORM TO THE FLORIDA BUILDING CODE, (2017) EDITION.
- VERIFY EXISTING CONDITIONS AND ALL DIMENSIONS AND NOTIFY ARCHITECT OF ANY CONDITIONS WHICH CONFLICT WITH OTHER PLANS AND SPECIFICATIONS. STRUCTURAL DRAWINGS MUST BE COORDINATED WITH ARCHITECTURAL DRAWINGS. STRUCTURAL DRAWINGS ARE NOT INTENDED FOR BUILDING LAYOUT.
- SHOP DRAWINGS WILL NOT BE REVIEWED BY THE DESIGNER UNTIL AFTER THE GENERAL CONTRACTOR HAS THOROUGHLY REVIEWED THE SHOP DRAWINGS, VERIFIED EXISTING CONDITIONS, AND COORDINATED THE SHOP DRAWINGS WITH OTHER AFFECTED TRADES. SUBMIT FOUR COPIES OF REVIEWED DRAWINGS FOR ENGINEER'S REVIEW. ONLY THREE SETS OF MARKED UP SHOP DRAWINGS SHALL BE RETURNED BY THE DESIGNER. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
- COMPLETE SHOP DRAWINGS AND CALCULATIONS FOR COMPONENTS NOT DESIGNED BY THE ENGINEER OF RECORD AND NOT SPECIFIED ON THE PROJECT CONSTRUCTION DOCUMENTS SHALL BE SEALED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROJECT STATE AND SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW AND APPROVAL PRIOR TO BEGINNING FABRICATION, INCLUDING BUT NOT LIMITED TO THE FOLLOWING COMPONENTS:

- (X) HANDRAILS
 - (X) WOOD TRUSSES
 - (X) POST-TENSION SLAB ON GRADE SYSTEM
- THE STRUCTURE IS UNSTABLE UNTIL ALL LOAD BEARING WALLS ARE ERECTED AND STEEL MEMBERS ARE ERECTED. CONNECTIONS ARE COMPLETELY BOLTED AND/OR WELDED AND INSPECTED. THE STEEL DECK ATTACHED TO THE STEEL FRAMING AND THE CONCRETE FLOORS PLACED AND ATTAINS 75% OF 28-DAY STRENGTH. UNTIL SUCH TIME, TEMPORARY BRACING IS REQUIRED. THE DESIGN ADEQUACY OF TEMPORARY BRACING AND SHORING IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

- DO NOT SCALE STRUCTURAL DRAWINGS, AND FOR LOCATION OF MISCELLANEOUS ITEMS (OPENINGS, BENT PLATES, INSERTS, ETC.) AFFECTING STRUCTURAL WORK, SEE ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.
- DEAD LOADS:
SELF-WEIGHT OF STRUCTURE
ROOF: 20 PSF
- LIVE LOADS:
ROOFS: 20 PSF (REDUCIBLE PER CODE)

- GARAGE LIVE LOADS:
GARAGE FLOORS: 40 PSF
CONCENTRATED: 3000 PSF
- ROOF LOADS:
GROUND SNOW LOAD: 10 PSF
SNOW EXPOSURE Co: 9
SNOW IMPORTANCE I: 1.0
THERMAL FACTOR Ct: 1.0
FLAT ROOF SNOW LOAD: 10 PSF
- WIND LOADS:
RISK CATEGORY: II
ULTIMATE WIND SPEED 3-SECOND GUST: 148 MPH V (ULTIMATE)
PROJECT IS IN WIND BORNE DEBRIS REGION
WIND IMPORTANCE I: 1.0
WIND EXPOSURE FACTOR: B
INTERNAL PRESSURE COEFFICIENT: .18
CLADDING LOAD: SEE DIAGRAM

- SEISMIC LOADS:
"NOT REQUIRED" 2012 FLORIDA BUILDING CODE CHAPTER 1, PART 101.2
- SPECIAL LOADS FOR ITEMS TO BE DESIGNED BY OTHERS:
STAIRS: 100 PSF
HANDRAILS: 50 PLF

SPECIAL INSPECTIONS AND TESTING

- THE CONTRACTOR/OWNER SHALL EMPLOY AN INDEPENDENT TESTING COMPANY TO PERFORM SITE INSPECTIONS AND TESTING IN ACCORDANCE WITH THE QUALITY ASSURANCE PLAN SHEET 5002.

FOUNDATION NOTES

- FOUNDATION DESIGN IS BASED ON A REPORT MADE BY TERRACON DATED 7-12-2019 (REPORT NO. HE195006).
- INDIVIDUAL FOOTINGS ARE DESIGNED TO BEAR ON SHALLOW FOUNDATIONS CAPABLE OF SUPPORTING 3000 PSF. CONTINUOUS FOOTINGS ARE DESIGNED TO BEAR ON SHALLOW FOUNDATIONS OF SUPPORTING 200 PSF. DESIGN ASSUMES DIFFERENTIAL AND TOTAL SETTLEMENT ARE WITHIN ACCEPTED TOLERANCES FOR THE TYPE OF CONSTRUCTION USED.
- THE BEARING CAPACITY AND CONSISTENCY SHALL BE VERIFIED FOR THE BUILDING LIMITS BY A REGISTERED GEOTECHNICAL ENGINEER WHEN FOUNDATION EXCAVATIONS HAVE BEEN CARRIED DOWN TO THE PROPOSED ELEVATIONS. THE BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM BELOW FINISHED GRADE.
- WHERE FOOTING EXCAVATIONS ARE TO BE OPEN AND HARBOR EXPOSED TO RAINFALL, THE EXCAVATIONS SHALL BE UNDERCUT AND A 3-INCH THICK MUD MAT OF 2000 PSI CONCRETE SHALL BE PLACED IN THE BOTTOM TO PROTECT THE BEARING SOILS.
- WHERE FOOTING STEPS ARE NECESSARY, THEY SHALL BE NO STEEPER THAN 1 VERTICAL TO 2 HORIZONTAL, UNLESS SHOWN OTHERWISE ON PLANS.

THE BUILDING SHALL BE REPAIRED TO "EARTHWORK" SECTION OF THE GEOTECHNICAL REPORT.

REINFORCED CONCRETE

- ALL CONCRETE WORK SHALL CONFORM TO THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE," (ACI 318).
- REINFORCING STEEL SHALL BE DEFORMED BARS ASTM A-615 (GRADE 60).
- MATERIAL PROPERTIES - CONCRETE:

	F'c psi at 28 days	Max. W/C Ratio	Max. Total Air Shump Inches	Non. Content (% ± 3%)	Max. Aggregate Size
1. Cast-in-place concrete					
a. Footings	4,000	0.45	4	No test	2"
b. Slab on grade	4,000	0.45	4		1"
c. Stairs, landings, lobbies	5,000	0.45	4	5	3/4"
d. All other	4,000	0.45	4	5	3/4"
2. Other concrete					
a. NSNS grout	DV 8,000		0	No test	No. 4

- *Prior to adding water reducer
 - **No slump requirement
 - ***After water reducer addition
- LAP SPLICES FOR REINFORCING BARS SHALL BE CLASS B IN ACCORDANCE WITH ACI 318, UNLESS NOTED OTHERWISE.
 - THE LONGITUDINAL REINFORCING STEEL IN BOND BEAMS, WALLS, AND FOOTINGS SHALL BE CONTINUOUS AROUND CORNERS. SEE TYPICAL DETAILS.
 - CLEAR CONCRETE COVER FOR REINFORCING STEEL:

	WALLS	MASONRY WALLS SLAB ON GRADE	FOOTINGS
2" EXTERIOR FACES			
1/2" INTERIOR FACES			
LOCATE IN CENTER OF WALL (U.N.O.)			
1/2" TOP STEEL			
1-1/2" BOTTOM STEEL			
2" FORMED EDGES			
3" CAST AGAINST GROUND			

- CONCRETE WALLS AND SLABS SHALL BE REINFORCED AROUND ALL OPENINGS WITH 2-#6 BARS IN EACH FACE, ON ALL SIDES AND EXTENDED 2'-0" BEYOND THE OPENING, UNLESS SHOWN OTHERWISE.
- MECHANICAL VIBRATORS SHALL VIBRATE ALL CONCRETE.
- CHAMFER EXPOSED CORNERS OF BEAMS, COLUMNS AND WALLS 3/4 INCH.
- COORDINATE ALL VAPOR RETARDERS, VAPOR BARRIERS, AND WATERPROOFING OF CONCRETE SLABS ON GRADE AND CONCRETE WALLS WITH FINISH MATERIAL REQUIREMENTS AND ARCHITECTURAL SPECIFICATIONS.

POST-TENSIONED CONCRETE SLABS ON GRADE

- PRESTRESSING STEEL WILL BE 1/2" DIAMETER 270 KSI, SEVEN WIRE STRAND, GREASE WITH PLASTIC SHEATHING (ENCAPSULATED). STRAND SHALL BE IN ACCORDANCE WITH ASTM A1818.
- ALL ANCHORAGE HARDWARE SHALL MEET TEST CRITERIA ESTABLISHED BY THE "PTI GUIDE SPECIFICATIONS OF POST TENSIONING MATERIALS."
- ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. STRANDS SHALL NOT BE STRESSED TO FINAL REQUIRED FORCE UNTIL CONCRETE STRENGTH IS A MINIMUM OF 2000 PSI.
- SITE GRADING AND DRAINAGE AROUND THE FOUNDATION SHALL BE MAINTAINED AT ALL TIMES IN SUCH A MANNER THAT SURFACE OR GROUND WATER WILL NOT COLLECT UNDER OR ADJACENT TO THE SLAB.
- TRENCHES FOR DEEP PLUMBING LINES SHOULD BE LOCATED AWAY FROM UNDER BEAMS.
- THE POST-TENSIONING CONTRACTOR SHALL SUBMIT TO THE STRUCTURAL ENGINEER, FOR REVIEW, DETAIL AND PRESTRESSING PROCEDURES AND SEQUENCES.
- TENDONS IN GRAB BEAMS ARE TO BE PLACED AS SHOWN ON DRAWINGS.
- TENDONS IN SLABS ON GRADE ARE TO BE PLACED IN CENTER OF SLAB UNLESS OTHERWISE SPECIFIED ON DRAWINGS.
- THE POST-TENSIONING CONTRACTOR SHALL PROVIDE AND INSTALL ALL ADDITIONAL REINFORCING BARS REQUIRED FOR SUPPORT OF TENDONS AND ANCHORS.
- ALL STRANDS AND BARS SHALL BE SUPPORTED ON CHAIRS NOT TO EXCEED FEED IN CENTER EACH WAY.
- STRANDS WITH POCKET FORMERS MUST BE SECURELY FASTENED TO FORM BOARDS TO PREVENT CEMENT PASTE FROM ENTERING INTO WEDGE CANAL (2-20 D NAILS ARE RECOMMENDED). FIXED ANCHORS SHOULD BE ATTACHED WITH 3/4" CLEARANCE FROM FORM BOARD.
- STRANDS MUST BE TIED AT ALL INTERSECTIONS STARTING AT FIXED END ANCHORS WORKING TOWARD LIVE END TO REMOVE SLACK AND ELIMINATE EXCESSIVE DISPLACEMENT DURING POUR.
- ALL ANCHORS SHALL BE INSTALLED A MINIMUM OF 4" BELOW REQUIRED ELEVATION OF CONCRETE AT EDGE OF SLAB AND A MINIMUM OF 6" AWAY FROM ALL CORNERS.
- IF PLASTIC SLEEVE IS DAMAGED THE STRAND SHOULD BE REPAIRED TO PREVENT BONDING OF CONCRETE TO TENDON.
- CONCRETE MUST BE WELL CONSOLIDATED AROUND ALL ANCHORS TO AVOID BEARING FAILURES AT STRESSING.
- TENDONS SHALL BE TEMPORARILY OVERSTRESSED TO 0.8 Fpu AND LOCKED OFF AT A STRESS OF 0.7 Fpu.
- CUT TENDONS ONLY AFTER ACCEPTANCE AND APPROVAL OF STRESSING RECORDS BY THE STRUCTURAL ENGINEER.
- THE EXPOSED PORTION OF THE TENDON AND THE GRIPPING PART OF THE ANCHORAGE SHALL BE COATED WITH AN EPOXY-RESIN COMPOUND BEFORE BEING DRY-PACKED WITH A NON-SHRINK GROUT.
- STRESSING POCKETS SHOULD BE GROUTED WITHIN 7 DAYS OF THE FINAL STRESSING OPERATION TO PREVENT CORROSION OF THE ANCHOR AND WEDGE ASSEMBLY.
- IN THE EVENT OF BROKEN TENDONS OR BLOWOUTS, THE POST-TENSIONING TENDON SUPPLIER SHALL SUBMIT FOR APPROVAL TO THE STRUCTURAL ENGINEER DETAILED CALCULATIONS AND PROCEDURES FOR REMEDYING THE PROBLEM.
- CORING OF SLABS WILL NOT BE PERMITTED WITHOUT THE WRITTEN CONSENT OF THE STRUCTURAL ENGINEER. ALL OPENINGS AND/OR SLEEVES MUST BE SHOWN ON THE SHOP DRAWINGS. ANY ADDITIONAL OPENINGS NOT SHOWN ON THE APPROVED DRAWINGS WILL REQUIRE APPROVAL FROM THE STRUCTURAL ENGINEER PRIOR TO PLACEMENT.

STRUCTURAL STEEL

- ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE AISC "MANUAL OF STEEL CONSTRUCTION ALLOWABLE STRESS DESIGN" FOURTEENTH EDITION.
- STRUCTURAL STEEL ROLLED SHAPES SHALL BE ASTM A-992 GRADE 50 UNLESS NOTED OTHERWISE. STRUCTURAL STEEL PLATES AND ANGLES SHALL BE ASTM A-36.
- STRUCTURAL PIPE COLUMNS SHALL BE ASTM A-53, TYPE E OR S, GRADE B. STRUCTURAL TUBES SHALL BE ASTM A500, GRADE B.
- STEEL FRAMING CONNECTIONS SHALL BE BOLTED OR WELDED. BOLTS SHALL BE 3/4 INCH DIAMETER MINIMUM AND SHALL BE ASTM A-325-N, UNLESS NOTED OTHERWISE.
- USE DIRECT TENSION INDICATORS AND HARDENED WASHERS WITH ALL HIGH STRENGTH BOLTS OR USE LOAD INDICATOR BOLTS.
- METAL DECK SHALL BE INSTALLED IN ACCORDANCE WITH THE STEEL DECK INSTITUTE SPECIFICATIONS, LATEST EDITION.
- WELD WASHERS SHALL BE USED WITH METAL DECK THINNER THAN 22 GAGE.
- MISCELLANEOUS ANCHOR BOLTS SHALL BE ASTM A-307 HEADED BOLTS.
- ANCHOR RODS AT COLUMN BASE PLATES SHALL BE ASTM F-1554 GRADE 55. MINIMUM ANCHOR BOLT EMBEDMENT SHALL BE 16 BOLT DIAMETERS UNLESS NOTED OTHERWISE. CLEAN ANCHOR BOLTS OF ALL GREASE, DIRT, ETC., BEFORE INSTALLATION. COLUMN ANCHOR RODS SHALL BE HELD IN PLACE BY TEMPLATES AND POSITIONED PRIOR TO CASTING CONCRETE.
- FRAMED BEAM CONNECTIONS SHALL BE DESIGNED BY A QUALIFIED PROFESSIONAL ENGINEER EMPLOYED BY THE FABRICATOR TO DEVELOP THE REACTION SHOWN FOR THE ENDS OF BEAMS ON STRUCTURAL PLANS. IN NO CASE SHALL THE LENGTH OF THE FRAMED CONNECTION BE LESS THAN 1/2 THE "T" DIMENSION OF THE BEAM WEB. WHERE REACTIONS ARE NOT SHOWN, THE CONNECTION SHALL DEVELOP ONE-HALF THE ALLOWABLE UNIFORM LOAD FOR LATERALLY SUPPORTED BEAMS AS SHOWN IN PART 2 OF THE AISC MANUAL.
- MEMBERS SHOWN ON THE STRUCTURAL DRAWINGS ARE THE MINIMUM REQUIRED BY DESIGN. THE FABRICATOR'S DRAWINGS SHALL SHOW WELDS AND THEY SHALL CONFORM TO AWS SPECIFICATIONS. ALL WELDING SHALL BE DONE TO E-70 SERIES ELECTRODES.
- HARDENED WASHERS SHALL BE INSTALLED OVER SHORT SLOPED OR CURVED HOLES OCCURRING IN AN OUTER PLY OF A CONNECTION.
- PAINT ALL STRUCTURAL STEEL THAT DOES NOT RECEIVE SPRAY-ON FIREPROOFING WITH ONE COAT OF RUST INHIBITIVE PRIMER 2.5 MILS IN THICKNESS. THE COMPLETENESS OF PRIMER AND ANY TOP COAT SHALL BE VERIFIED BEFORE ANY PAINTING IS PERMITTED. TOUCH-UPS SHALL EXPOSED METAL AFTER INSTALLATION. ALL STRUCTURAL STEEL WHICH IS EXPOSED TO THE ELEMENTS SHALL RECEIVE TWO COATS OF EXTERIOR ENAMEL WHICH IS TO BE APPLIED TO THE EXPOSED SURFACE.
- STRUCTURAL STEEL SHOP DRAWINGS SHALL INCLUDE COMPLETE DETAILS, CONNECTIONS, AND SCHEDULES FOR FABRICATION AND ASSEMBLY OF STRUCTURAL STEEL MEMBERS. SHOP DRAWINGS SHALL INCLUDE MEMBER SPACING, MEMBER DISTANCE FROM BEAM CENTERLINE OF SHEAR STUDS, STRUCTURAL STEEL SHOP DRAWINGS SHALL NOT INCLUDE MISCELLANEOUS STEEL.
- THE STRUCTURAL DESIGN OF STEEL STAIRS, LANDINGS AND GUARDRAILS (INCLUDING ENDS) SHALL BE PERFORMED BY A STRUCTURAL ENGINEER REGISTERED IN THE PROJECT STATE. CALCULATIONS AND SHOP DRAWINGS WITH THE ENGINEER'S SEAL SHALL BE SUBMITTED FOR APPROVAL. NO FABRICATION SHALL BEGIN UNTIL THE SUBMITTAL IS APPROVED. DESIGN DETAILS SHALL BE AS SPECIFIED BY THE CONTRACT DOCUMENTS AND/OR THE APPLICABLE CODES, WHICHEVER IS MORE STRINGENT.

WOOD TRUSSES

- ROOF TRUSSES SHALL BE DESIGNED TO SUPPORT THE FOLLOWING LOADS:
TOP CHORD: 12 PSF DEAD LOAD
20 PSF LIVE LOAD
BOTTOM CHORD: 8 PSF DEAD LOAD
- IN ADDITION TO THE UNIFORM LOADING SPECIFIED FOR TRUSS DESIGN, THE TRUSS SUPPLIER SHALL INCLUDE ANY CONCENTRATED LOADS CAUSED BY ARCHITECTURAL FEATURES OR MECHANICAL EQUIPMENT IN THE TRUSS DESIGN.
- SEE ARCHITECTURAL DRAWINGS FOR BEARING CONDITIONS AND DIMENSIONS OF TRUSSES.
- A REGISTERED ENGINEER IN THE PROJECT STATE SHALL DESIGN TRUSSES, SHOP DRAWINGS, INCLUDING INDIVIDUAL TRUSS DESIGNS, PLAN LAYOUT, ALL TEMPORARY BRACING AND PERMANENT TRUSS MEMBER BRACING BEARING THE ENGINEER'S SEAL SHALL BE SUBMITTED FOR REVIEW.
- TRUSSES SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH APPLICABLE STANDARDS OF THE TRUSS PLATE INSTITUTE.
- UNLESS NOTED OTHERWISE ON DRAWINGS, TRUSS SUPPLIER SHALL BE RESPONSIBLE FOR DESIGNING AND SUPPLYING OR SPECIFYING ALL TEMPORARY BRACING AND PERMANENT INDIVIDUAL TRUSS MEMBER BRACING REQUIRED BY DESIGN. ALL TRUSS-TO-TRUSS CONNECTIONS, AND ALL UPLIFT CONNECTIONS AT BEARING LOCATIONS. ALL PERMANENT BRACING OF TRUSS MEMBERS SHALL BE CONTINUOUS AND BE ATTACHED TO AN END WALL STUD OR HIP TRUSS TOP CHORD WITH 2 #16D COMMON NAILS.

LUMBER FRAMING

- ALL NON-PREFABRICATED LOAD BEARING FRAMING MEMBERS SHALL BE #2 SOUTHERN YELLOW PINE 19% MOISTURE CONTENT UNLESS OTHERWISE NOTED.
- ALL PLYWOOD SHEATHING SHALL BE APA RATED, SEE PLAN.
- THE ALLOWABLE STRESSES FOR FIRE RETARDANT TREATED LUMBER SHALL BE REDUCED 10%.
- LVL AND PSL LUMBER SHALL BE MICROLAM OR PARALLAM LUMBER AS MANUFACTURED BY TRUS JOIST.
- WOOD JOISTS SHALL BE TJ100P SERIES AS MANUFACTURED BY TRUS JOIST.

POST-INSTALLED ANCHORS

- UNLESS NOTED OTHERWISE, POST-INSTALLED CONCRETE ANCHORS SHALL COMPLY WITH ICC-ES ACCEPTANCE CRITERIA FOR ANCHORS IN CRACKED CONCRETE AND SEISMIC APPLICATIONS.
- POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS UNLESS APPROVED OTHERWISE BY THE ENGINEER.
- PLACE POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR AND EMBEDS.
- DRILL AND PREPARE HOLES AND INSTALL ANCHORS IN ACCORDANCE WITH EVALUATION REPORTS.
- POST-INSTALLED ANCHORS SHALL BE INSPECTED BY A QUALIFIED SPECIAL INSPECTOR IN ACCORDANCE WITH THE PROJECT STATEMENT OF SPECIAL INSPECTION AND THE ICC-ES REPORT.

UNLESS OTHERWISE NOTED IN THE ICC-ES REPORT, THE SPECIAL INSPECTOR SHALL INSPECT THE INITIAL INSTALLATION OF EACH TYPE OF ANCHOR AND PERIODICALLY INSPECT INSTALLATION THEREAFTER.

- MECHANICAL ANCHORS FOR USE IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ACI 308.2 AND ACI 308.3. ACCEPTABLE MECHANICAL ANCHORS FOR USE IN CONCRETE SHALL BE THE FOLLOWING:
HILTI KWIK BOLT TZ (ICC-ES ESR 1917)
HILTI KWIK HUS-EZ (ICC-ES ESR 1077)
SIMPSON STRONG-TIE STRONG-TIE 2 (ICC-ES ESR 3187)
SIMPSON STRONG-TIE STRONG-TIE HD (ICC-ES ESR 3183)
- ADHESIVE ANCHORS (INCLUDING REBAR) FOR USE IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ACI 308.2 AND ICC-ES AC308. ADHESIVE ANCHORS SHALL BE INSTALLED INTO DRY HOLES DRILLED USING A CARTRIDGE DRILL BIT THAT IS INSURED FOR AT LEAST 21 DAYS. ACCEPTABLE ADHESIVE ANCHORS FOR USE IN CONCRETE INCLUDE THE FOLLOWING:
HILTI HIT RE 500 30 (ICC-ES ESR 2322)
HILTI HIT HY 70 (ICC-ES ESR 1385)
SIMPSON STRONG-TIE SET-XP (ICC-ES ESR 2508)
- MECHANICAL ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC01. ACCEPTABLE MECHANICAL ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY INCLUDE THE FOLLOWING:
HILTI KWIK BOLT 3 (ICC-ES ESR 1385)
SIMPSON STRONG-TIE WEDGE-ALL (ICC-ES ESR 1386)
- ADHESIVE ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC08. ACCEPTABLE ADHESIVE ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY INCLUDE THE FOLLOWING:
HILTI HIT-HY 70 (ICC-ES ESR 3342)
SIMPSON STRONG-TIE SET-XP (IPMPO UES ER 265)

1925 Prospect Ave.
Orlando, FL 32814
P (407) 661-9100
F (407) 661-9101
www.cfp.com

Chalaco & Peterson
Architects & Planners



CLIENT NAME
HUTTON DEVELOPMENT

PROJECT NAME
ROCKLEDGE FLATS - BUILDING D
220 Barton Boulevard
Rockledge, FL 32955

SHEET TITLE
GENERAL NOTES



Revision Schedule	
No.	Description

PROJECT NO.	210012
DATE	2/20/21
DRAWN	EMC
CHECKED	EMC
BY	EMC

S001