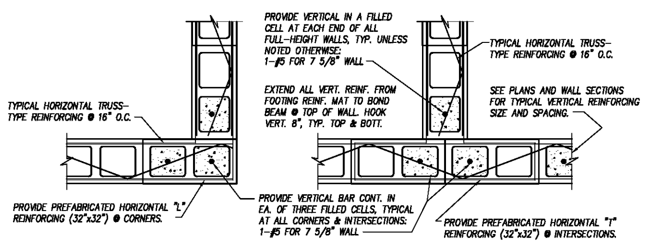
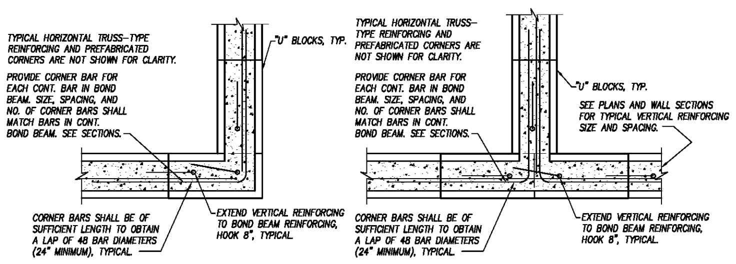


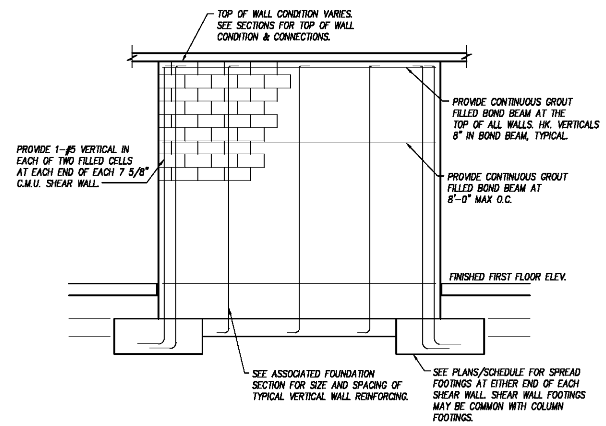
TYPICAL CORNER — TYPICAL INTERSECTION

PLAN DETAILS @ WALL INTERSECTIONS



TYPICAL CORNER — TYPICAL INTERSECTION

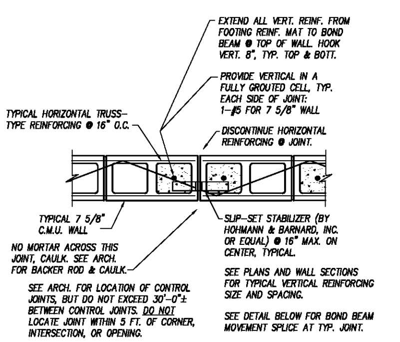
PLAN DETAILS @ BOND BEAM INTERSECTIONS



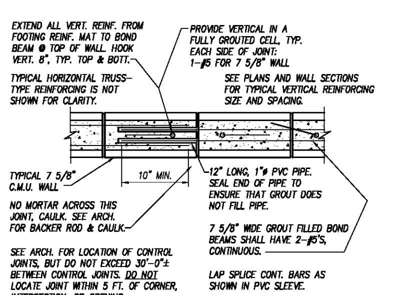
TYPICAL MASONRY SHEAR WALL

NO SCALE

NOTES:
1. SEE PLAN FOR LOCATIONS OF MASONRY SHEAR WALLS.
2. EXTEND ALL VERTICALS FROM FOOTING REINFORCING MAT TO TOP OF WALL.
3. FOOTING REINFORCING MAT IS NOT SHOWN FOR CLARITY. EXTEND WALL FOOTING REINFORCING THROUGH SHEAR WALL/COLUMN FOOTING.
4. SHEAR WALLS MAY BE ORIENTED UNDER OR ADJACENT TO STRUCTURE AT SECOND FLOOR, LOW ROOF, OR HIGH ROOF ELEVATIONS. SEE PLANS AND DETAILS FOR MORE INFORMATION.
5. ALL SHEAR WALLS SHALL BE ANCHORED AT THE TOP. SEE DETAILS.
6. THIS DETAIL IS PROVIDED FOR GENERAL CONFIGURATION OF FOUNDATION ELEMENTS AND REINFORCING. SEE DETAILS FOR MORE INFORMATION.



TYP. EXPANSION/CONTROL JOINT AT C.M.U. WALL



TYP. EXPANSION/CONTROL JOINT AT CONTINUOUS BOND BEAM

LINTEL SCHEDULE

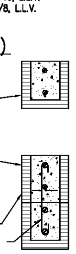
FOR EACH 4" MASONRY VENEER

SPAN	LINTEL
0' TO 1'-0"	FB 3 1/2 x 1/4
OVER 1'-0" TO 4'-0"	L 3 1/2 x 3 1/2 x 1/4
OVER 4'-0" TO 6'-0"	L 4 x 3 1/2 x 5/16, L.L.V.
OVER 6'-0" TO 8'-0"	L 5 x 3 1/2 x 3/8, L.L.V.

FOR CONCRETE BLOCK (C.M.U.)

FOR EACH 8" WYTHE OR FRACTION THEREOF

SPAN	LINTEL
LINTEL ONE COURSE DEEP	
0' TO 3'-0"	1-#4 T; 1-#5 B
OVER 3'-0" TO 5'-0"	1-#5 T; 1-#6 B
LINTEL TWO COURSES DEEP	
OVER 5'-0" TO 7'-0"	2-#6 T; 2-#7 B
OVER 7'-0" TO 11'-0"	2-#7 T; 2-#8 B
OVER 11'-0" TO 14'-0"	2-#8 T; 2-#8 B



NOTES:

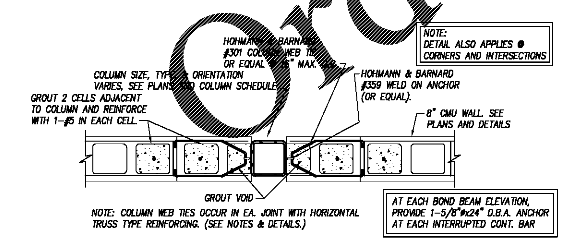
1- LINTELS SCHEDULED ABOVE SHALL BE USED UNLESS SHOWN OR NOTED OTHERWISE. SIZES SHOWN ARE MINIMUMS. IF FLASHING DETAILS REQUIRE OFFSET/ADDITIONAL SPACE BETWEEN LINTEL AND BACK OF VENEER, CONTACT PROJECT ENGINEER.

2- STEEL LINTELS SUPPORTING 4" VENEER SHALL HAVE 1" BEARING, EACH END, FOR EACH 1'-0" OF SPAN, BUT NOT LESS THAN 8" BEARING, EACH END.

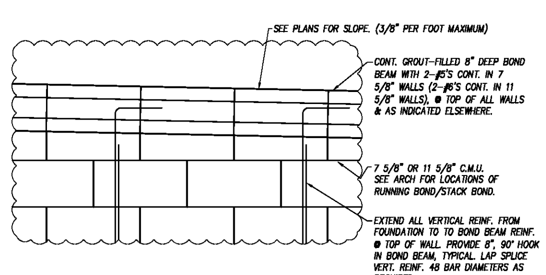
3- C.M.U. LINTELS AND REINFORCING SHALL EXTEND PAST EACH SIDE OF OPENING 48 BAR DIAMETERS (BASED ON LARGEST BAR) WHERE SPACE PERMITS. WHERE SPACE DOES NOT PERMIT EXTENSION, BEND BARS 90° INTO FULLY GROUTED CELL EACH SIDE OF OPENING.

4- CONCRETE BLOCK (C.M.U.) LINTELS SHALL BE MADE WITH FILLED "U" BLOCKS, EXCEPT AS NOTED. FILL SHALL BE COARSE GROUT CONFORMING TO ASTM C476, 3/8" MAXIMUM STONE SIZE. SHORE POURED LINTELS 7 DAYS, MINIMUM.

5- WHERE C.M.U. LINTEL SPAN EXCEEDS 6'-0", FILL CELLS OF CONCRETE BLOCK UNDER BEARING FOR 16" LENGTH FROM FOOTING TO TOP OF WALL & PROVIDE 1-#5 VERTICAL BAR IN EACH OF 2 FILLED CELLS, EACH SIDE OF OPENING. AT OTHER LINTELS, PROVIDE 1-#5 VERTICAL BAR, EACH SIDE OF OPENING, IN FILLED CELL. EXTEND ALL VERTICALS FROM FOOTING MAT TO BOND BEAM AT TOP OF WALL.



TYP. REINFORCING & TIES @ COLUMN



TYPICAL BOND BEAM SLOPE

REINFORCED C.M.U. GENERAL NOTES

1. ALL MASONRY CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF ASCE 6-11 ("SPECIFICATION FOR MASONRY STRUCTURES"). ALL SUPERVISORY PERSONNEL HAVING ANY CONNECTION WITH THE MASONRY WORK SHALL CERTIFY THAT THEY HAVE FAMILIARIZED THEMSELVES WITH THESE PUBLICATIONS.
2. BLOCK SHALL BE LAID IN A RUNNING BOND.
3. ALL FACE SHELLS OF BLOCKS SHALL BE COMPLETELY MORTARED IN BED JOINTS; HEAD JOINTS SHALL BE COMPLETELY MORTARED VERTICALLY FOR SAME WIDTH AS BED JOINTS. WHERE CELLS ARE TO BE FILLED WITH GROUT, FACE AND WEB SHELLS SHALL BE COMPLETELY MORTARED EXCEPT WHERE ADJACENT CELLS ARE TO BE FILLED. WHERE ADJACENT CELLS ARE TO BE FILLED, THE WEBS COMMON TO BOTH CELLS NEED NOT BE MORTARED.
4. CONCRETE MASONRY UNITS SHALL COMPLY WITH ASTM C 90, NORMAL WEIGHT. UNIT COMPRESSIVE STRENGTH SHALL BE 1900 PSI, MINIMUM. MANUFACTURER SHALL PROVIDE A WRITTEN CERTIFICATION OF THE UNIT COMPRESSIVE STRENGTH WHICH SHALL BE SAMPLED AND TESTED IN ACCORDANCE WITH ASTM C140-03.
5. ALL BLOCK WORK BELOW THE FINISHED FLOOR ELEVATION SHALL BE LAID USING TYPE M MORTAR. ALL BLOCK WORK ABOVE THE FINISHED FLOOR ELEVATION SHALL BE LAID USING TYPE S OR TYPE M MORTAR. MORTAR SHALL COMPLY WITH ASTM C 270-04.
6. ASSUMED F'_m FOR EXTERIOR WALLS = 1500 PSI @ 28 DAYS.
7. ASSUMED F'_m FOR INTERIOR WALLS = 1500 PSI @ 28 DAYS.
8. PROVIDE CONT. HORIZONTAL REINFORCING AT 16" MAX. O.C. VERTICALLY. HORIZONTAL REINFORCING SHALL BE W1.7 (9 GA. x 9 GA.) TRUSS-TYPE JOINT REINFORCING. HORIZONTAL JOINT REINFORCING SHALL COMPLY WITH ASTM A951-02 AND SHALL BE HOT DIP GALVANIZED PER ASTM A153-03. LAP SPLICE ALL JOINT REINFORCING IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTRUCTIONS. USE PREFABRICATED SECTIONS AT INTERSECTIONS AND CORNERS. <u>DO NOT CUT OR BEND.</u>
9. ALL CELLS AND VOIDS NOTED AS FILLED SHALL BE FILLED SOLID WITH GROUT CONFORMING TO ASTM C476-02. COARSE AGGREGATE SHALL NOT EXCEED 3/8" IN DIMENSION.
10. PROVIDE A CONTINUOUS GROUT FILLED BOND BEAM (WITH 2-CONTINUOUS #5'S) AT 8'-0" MAX. O.C. VERTICALLY IN ALL 7 5/8" REINFORCED MASONRY WALLS.
11. PROVIDE A CONTINUOUS GROUT FILLED BOND BEAM (WITH 2-CONTINUOUS #5'S) AT THE TOPS OF ALL 7 5/8" REINFORCED MASONRY WALLS.
12. PROVIDE KNOCK-OUT BLOCKS FOR BOND BEAMS WHERE SHOWN OR NOTED. PROVIDE U-BLOCKS FOR BOND BEAMS IN ALL OTHER WALLS.
13. VERTICAL WALL REINFORCING IN CENTER OF FULLY GROUTED CELLS WILL BE REQUIRED AT:
    - 13.A. SPACINGS SHOWN ON PLANS & DETAILS.
    - 13.B. IN EACH OF 3 FILLED CELLS AT ALL CORNERS AND INTERSECTIONS (SEE ASSOC. DETAILS, THIS SHEET)
    - 13.C. IN FILLED CELLS ON EA. SIDE OF ALL OPENINGS (AS INDICATED IN LINTEL SCHEDULE)
    - 13.D. IN EACH OF 2 FILLED CELLS AT ALL BEAM BEARINGS (SEE ASSOC. DETAILS, THIS SHEET)
    - 13.E. IN FILLED CELLS ON EACH SIDE OF EACH EXPANSION/ CONTROL JOINT (SEE ASSOC. DETAILS, THIS SHEET)
    - 13.F. AT EACH END OF EACH SHEAR WALL (SEE S3.1)
    - 13.G. IN FILLED CELL AT EACH 'FREE END' OF EACH WALL.
14. EXTEND ALL VERTICALS FROM FOOTING MAT TO TOP OF WALL. SEE SECTIONS & DETAILS. LAP SPLICE VERTICALS 48 BAR DIAMETERS AS REQUIRED. PROVIDE 8" LONG, 90° HOOKS @ FOUNDATION MAT & T/WALL BOND BEAM, TYPICAL ALL VERTICALS.
15. PROVIDE 1-#5 IN EACH OF 3 FULLY GROUTED CELLS AT ALL CORNERS AND INTERSECTIONS OF MASONRY UNITS.
16. AT EACH END OF EACH SHEAR WALL, PROVIDE 1-#5 DOWEL IN EACH OF 2 FULLY GROUTED CELLS
17. CONTRACTOR IS RESPONSIBLE FOR PROVIDING ADEQUATE SHORING FOR WALLS DURING CONSTRUCTION.
18. SHOP DRAWINGS SHALL BE SUBMITTED FOR APPROVAL SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING REINFORCING STEEL AND ACCESSORIES. SHOW WALL STEEL IN PLAN & ELEVATION. INDICATE CONTROL JOINT LOCATIONS. NO FABRICATION SHALL BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.

REINFORCED CONCRETE GENERAL NOTES

1. ALL CONCRETE WORK SHALL CONFORM TO ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS". DESIGN IS BASED ON ACI 318, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE."
2. CONCRETE SHALL BE NORMAL WEIGHT (150 PCF) AND SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS (EXCEPTIONS NOTED).
3. SUBMIT CONCRETE MIX DESIGNS FOR REVIEW, IN ACCORDANCE WITH ACI 318 TO THE ARCHITECT AND TESTING AGENCY.
4. THE PROPOSED MATERIALS AND MIX DESIGN SHALL BE FULLY DOCUMENTED AND REVIEWED. RESPONSIBILITY FOR OBTAINING THE REQUIRED DESIGN STRENGTH IS THE CONTRACTOR'S.
5. USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED.
6. THE AIR CONTENT AT THE POINT OF PLACEMENT SHALL BE PER ACI 318 TABLE 4.2.1 FOR MODERATE EXPOSURE. HARD TROWELED SLABS OR SLABS ON GRADE THAT WILL RECEIVED TOPPINGS SHALL NOT RECEIVE AIR ENTRAINMENT UNLESS PERMITTED BY TOPPING MANUFACTURER.
7. THE TESTING AGENCY SHALL SAMPLE AND TEST EACH 100 CU. YARDS OR FRACTION THEREOF OF EACH CLASS OF CONCRETE PLACED EACH DAY. SAMPLE CONCRETE IN ACCORDANCE WITH ASTM C172. PERFORM THE FOLLOWING TESTS IN ACCORDANCE WITH THE INDICATED STANDARD:

SLUMP:	ASTM C143
AIR CONTENT:	ASTM C231 (NORMAL WEIGHT CONCRETE) ASTM C173 (LIGHT WEIGHT CONCRETE)
COMPRESSIVE STRENGTH:	ASTM C39, WITH ONE CYLINDER AT 7 DAYS, 2 CYLINDERS AT 28 DAYS, AND ONE SPECIMEN HELD IN RESERVE.
CONCRETE TEMPERATURE:	ASTM C1064
UNIT WEIGHT:	ASTM C567

8. MOIST CURE CONCRETE WITH MOISTURE PROTECTIVE COVER FOR A MINIMUM OF 7 DAYS.
9. HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED ONLY WHERE INDICATED. THE LOCATION OF VERTICAL CONSTRUCTION JOINTS SHALL BE APPROVED BY THE ARCHITECT. CONSTRUCTION JOINTS SHALL BE THOROUGHLY ROUGHENED BY MECHANICAL MEANS, CLEANED, AND CAULKED.
10. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
11. WELDED WIRE REINFORCING (W.W.R.) SHALL BE PLAIN WIRE CONFORMING TO ASTM A1064 AND SHALL BE PROVIDED IN FLAT SHEETS. LAP MESH AT LEAST ONE MESH SPACING AT SPLICES.
12. TIE ALL REINFORCING STEEL AND EMBEDMENTS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN THE POSITION OF REINFORCEMENT WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES. "STICKING" DOWELS INTO WET CONCRETE IS NOT PERMITTED.
13. REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE:

CONCRETE CAST AGAINST EARTH (NOT FORMED):	3"
FORMED CONCRETE EXPOSED TO EARTH OR WEATHER	
#6 BARS THROUGH #12 BARS	2"
#5 BARS AND SMALLER	1 1/2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER	
SLABS AND WALLS	1"

14. DO NOT PLACE PIPES EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS WITHIN THE SLAB OR WALL UNLESS SPECIFICALLY SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.
15. DO NOT WELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE STRUCTURAL ENGINEER.
16. EXTERIOR SLABS SHALL DRAIN FREELY WITH A MAXIMUM VARIATION FROM THE INDICATED PLANE OF 1/4" PER 1'-0".

STRUCTURAL STEEL GENERAL NOTES

1. ALL STRUCTURAL STEEL CONSTRUCTION SHALL CONFORM TO THE 2005 AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."
2. UNLESS NOTED OTHERWISE:
   - 2.A. STEEL SHALL CONFORM TO ASTM A36 (36 KSI YIELD STRENGTH), EXCEPTIONS NOTED.
   - 2.B. ALL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992 (50 KSI YIELD STRENGTH).
   - 2.C. ALL HSS SECTIONS SHALL CONFORM TO ASTM A500 (46 KSI YIELD STRENGTH).
   - 2.D. ELECTRODES SHALL CONFORM TO E70-XX.
   - 2.E. ALL BOLTS TO BE A325X, WITH THREADS EXCLUDED FROM THE SHEAR PLANE.
   - 2.F. ALL ANCHOR RODS SHALL CONFORM TO ASTM F1554 (36 KSI YIELD STRENGTH) WITH HEAVY HEX NUTS AND STANDARD WASHERS.
   - 2.G. ALL PL'S SHALL BE SUPPLIED FREE OF LAMINATIONS & IMPURITIES & SHALL CONFORM TO ASTM A36, U.N.O.
   - 2.H. ALL MOMENT CONNECTION PLATES (TOP, BOTTOM, STIFFENER & CONNECTING CAP PL'S) & JOIST GIRDER BEARINGS SHALL CONFORM TO ASTM A 572, GRADE 50. THESE PLATES SHALL BE CLEARLY MARKED FOR EASE OF IDENTIFICATION.
3. HEADED ANCHORS AND SHEAR CONNECTORS (STUDS) SHALL CONFORM TO ASTM A108 (SEE SPECS. FOR YIELD STRENGTH).
4. SHOP DRAWINGS PREPARED IN ACCORDANCE WITH THE LATEST "STRUCTURAL STEEL DETAILING MANUAL" OF AISC SHALL BE SUBMITTED FOR APPROVAL. NO FABRICATION SHALL BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED AND APPROVED.
5. STEEL FRAMING CONNECTIONS SHALL BE BOLTED OR WELDED, AS INDICATED.
   - 5.A. BOLTED JOINTS SHALL CONFORM TO AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". BOLTS SHALL CONFORM TO ASTM A325 AND SHALL BE MINIMUM 3/4" DIAMETER, UNLESS NOTED OTHERWISE.
   - 5.B. WELDS SHALL CONFORM TO THE "STRUCTURAL WELDING CODE" OF THE AMERICAN WELDING SOCIETY, AWS D1.1. USE E70XX ELECTRODES. WEDING PROCESSES AND OPERATORS SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATIONS PROCEDURES". WELDERS SHALL CARRY PROOF OF QUALIFICATIONS TO JOBSITE.
6. ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 36, WELDABLE UNLESS NOTED OTHERWISE.
7. PROVIDE SHOP COAT OF STANDARD PRIMER PAINT. PRIMER TO BE COMPATIBLE WITH FINISH COAT. TOUCH-UP AREAS DAMAGED IN HANDLING AND ERECTION WITH THE SAME PAINT USED FOR SHOP COAT. STEEL SURFACES TO BE WELDED OR ENCASED IN CONCRETE OR FIREPROOFING, OR SURFACES RECEIVING WELDED SHEAR CONNECTORS SHALL NOT BE PAINTED.
8. PROVIDE A NON-SHRINK, NON-METALLIC, SELF-LEVELING GROUT (F'c = 5000 PSI, MIN.) UNDER BASE PLATES, AFTER SETTING AND LEVELING. NO "DRY-PACKING" PERMITTED.

METAL DECK GENERAL NOTES

1. UNLESS NOTED OTHERWISE, METAL DECK SHALL BE GALVANIZED, CONFORMING TO STEEL DECK INSTITUTE (SDI) STANDARDS, LATEST EDITION.
2. SEE DRAWINGS FOR METAL DECK TYPE AND GAUGE.
3. UNLESS NOTED OTHERWISE, THE ROOF AND FLOOR DECK SHALL BE FASTENED TO STEEL FRAMING WITH 5/8" DIAM. PUDDLE WELDS AT THE FOLLOWING SPACING:
   - 3.A. AT FRAMING PERPENDICULAR TO DECK FLUTES: 12" ON CENTER MAXIMUM AND AT EACH EDGE FLUTE OF EACH DECK UNIT.
   - 3.B. AT FRAMING PARALLEL TO DECK FLUTES: 12" ON CENTER MAXIMUM SIDE LAPS SHALL BE FASTENED BY WELDING, OR WITH #10 SELF TAPPING SCREWS AT A MINIMUM SPACING OF 16" ON CENTER BETWEEN SUPPORTS.
   - 3.C. ADDITIONAL DECKING SHEET EDGE REQUIREMENTS: ONE FASTENER SHALL BE PLACED NEAR THE CORNER OF EACH SHEET OR AT OVERLAPPING CORNERS OF SHEETS. AT ENDS AND SIDES OF DECK SHEETS, WELDS TO STRUCTURAL MEMBERS SHALL BE 12" O.C., MAXIMUM.
4. DECKING SHALL BE CONTINUOUS OVER 3 SPANS MINIMUM WHERE SUPPORTING STRUCTURE PERMITS.
5. DECKING SHALL BE ERECTED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.
6. MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE. SUBMIT PRODUCT DATA, INCLUDING SPAN TABLES, FOR REVIEW.
7. TOUCH UP AREAS DAMAGED IN HANDLING AND ERECTION WITH SAME PAINT USED FOR SHOP COAT.

GENERAL NOTES

1. <u>LIVE LOADS</u> (WITH APPROPRIATE REDUCTIONS)
   - A. 1ST FLOOR SLAB.......... 100 PSF
   - B. ROOF.......... 20 PSF
2. <u>DESIGN WIND LOADS</u>
   - A. BASIC WIND SPEED (V_{ULT}).......... 115 MPH
   - B. NOMINAL WIND SPEED (V_{ASD}).......... 89 MPH
   - C. RISK CATEGORY..........II
   - D. WIND EXPOSURE CATEGORY "C"
   - E. INTERNAL PRESSURE COEFFICIENT.........."GC_{PI}" = ±0.18
   - F. COMPONENTS & CLADDING DESIGN WIND PRESSURES:

RESTROOM FACILITIES:	ZONE	A = EFFECTIVE WIND AREA (FT²) A=10	A=20	A=50	A=100
WALLS	CORNER (5)	+26 / -29	+25 / -33	+24 / -30	+23 / -28
	OTHER (4)	+26 / -29	+25 / -27	+24 / -26	+23 / -25
ROOF: (1 ON 12)	CORNER (3)	+12 / -48	+11 / -44	+10 / -42	+9 / -41
	EDGE (2)	+12 / -44	+11 / -43	[illegible]	+9 / -41
	OTHER (1)	+12 / -29	[illegible]	+10 / -29	[illegible] / -29
ROOF OVERHANGS:	CORNER (3)	-73	[illegible]	-44	[illegible]
	EDGE (2)	[illegible]	-45	-44	[illegible]

SHADE CANOPY:	ZONE	A = EFFECTIVE WIND AREA (FT²) A=10	A=20	A=50	A=100
[illegible]	CORNER (3)	[illegible] / -71	[illegible] / -65	+8 / -56	+7 / -49
	EDGE (2)	[illegible] / -40	+9 / -36	+8 / -32	+7 / -30
	[illegible] (1)	+10 / -32	+9 / -31	+8 / -28	+7 / -27

ALL PRESSURES ARE INTENDED TO BE APPLIED AND INTERPRETED IN STRICT ACCORDANCE WITH ASCE 7-10.

FOR THE PURPOSES OF APPLYING THESE LOADS IN ACCORDANCE WITH COMPONENTS & CLADDING FIGURES IN ASCE 7-10, a=7 FT.

3. <u>SEISMIC DESIGN</u>
   - A. SEISMIC IMPORTANCE FACTOR.......... I = 1.00
   - B. MAPPED SPECTRAL RESPONSE ACCELERATIONS.......... S_s = 0.149, S_1 = 0.078
   - C. SITE CLASS "D"
   - D. SPECTRAL RESPONSE COEFFICIENTS.......... S_{DS} = 0.159, S_{D1} = 0.125
   - E. SEISMIC DESIGN CATEGORY "B"
   - F. BASIC LATERAL FORCE RESISTING SYSTEM: – ORDINARY REINFORCED MASONRY SHEAR WALLS
   - G. DESIGN BASE SHEAR .... V = KIPS
   - H. SEISMIC RESPONSE COEFFICIENT .... C_s =
   - I. RESPONSE MODIFICATION FACTOR .... R = 2
   - J. ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE PROCEDURE FOR SEISMIC DESIGN OF BUILDINGS
4. <u>SNOW LOADS</u>
   - A. GROUND SNOW LOAD.........."P_g" = 5 PSF
5. <u>CODES</u>
   - A. INTERNATIONAL BUILDING CODE, 2012 ED. WITH 2018 GEORGIA AMENDMENTS.
   - B. REINFORCED CONCRETE: ACI 318-11.
   - C. ALL STEEL WORK SHALL COMPLY WITH APPLICABLE STANDARDS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INCLUDING THE SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS, LATEST EDITION AND THE AMERICAN WELDING SOCIETY'S STRUCTURAL WELDING CODE, AWS D1.1-LATEST EDITION.
6. PROVIDE TEMPORARY BRACING OF STRUCTURAL FRAMING UNTIL ALL PERMANENT BRACING, SHEAR WALLS, MOMENT CONNECTIONS, AND ROOF DIAPHRAGMS ARE COMPLETELY INSTALLED. THE STRUCTURAL ELEMENTS ARE UNSTABLE UNTIL THE STRUCTURE IS COMPLETED IN ACCORDANCE WITH THE PLANS.
7. <u>SHOP DRAWINGS GENERAL NOTE:</u> THE PLANS, SECTIONS, AND DETAILS PROVIDED AS PART OF THE CONTRACT DOCUMENTS SHALL NOT BE USED IN A REPRODUCIBLE FORM AS PART OF THE SHOP DRAWINGS.

SOIL PARAMETERS

1. <u>SOIL PARAMETERS:</u> FOOTINGS ARE DESIGNED FOR A MINIMUM ALLOWABLE SOIL BEARING PRESSURE OF 2000 POUNDS PER SQUARE FOOT. SPECIFIC SITE PREPARATION GUIDELINES MAY BE REQUIRED TO ACHIEVE THIS SOIL BEARING PRESSURE.
2. IF SITE CONDITIONS DIFFER FROM THOSE INDICATED, THE DESIGN PROFESSIONAL SHALL BE NOTIFIED IMMEDIATELY.
3. REFERENCE CIVIL GRADING AND DRAINAGE PLANS, DETAILS AND SPECIFICATIONS.
4. ALL FOUNDATION WORK SHALL BE DONE 'IN THE DRY'.
5. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL EXISTING SITE CONDITIONS THROUGH AN INDEPENDENT SITE INVESTIGATION PRIOR TO COMMENCING WORK.

INSPECTIONS

1. INDEPENDENT STRUCTURAL TESTS AND INSPECTIONS SHALL COMPLY WITH THE REQUIREMENTS OF THE 2012 INTERNATIONAL BUILDING CODE WITH GEORGIA STATE AMENDMENTS. THE INSPECTOR SHALL BE RESPONSIBLE FOR COMPLIANCE WITH THE APPROVED STRUCTURAL PLANS AND SHALL SUBMIT PROGRESS REPORTS AND INSPECTION REPORTS TO THE ARCHITECT OF RECORD.
2. SATISFY MINIMUM INSPECTION AND QUALITY CONTROL REQUIREMENTS OF THE 2012 INTERNATIONAL BUILDING CODE WITH GEORGIA STATE AMENDMENTS.

LIGHT GAUGE NOTES

1. <u>CODES</u>
   - A. ALL LIGHT GAUGE STEEL WORK SHALL COMPLY WITH THE AMERICAN IRON AND STEEL INSTITUTE: NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, 2007 AND THE AMERICAN IRON AND STEEL INSTITUTE: CODE OF STANDARD PRACTICE FOR COLD-FORMED STEEL STRUCTURAL FRAMING, 2011
2. <u>LIGHT GAUGE STEEL</u>
   - A. STEEL SHALL CONFORM TO ASTM A1003, STRUCTURAL GRADE, TYPE H, METALLIC COATED. ALL MEMBERS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A653, OR ALUMINUM-ZINC COATED IN ACCORDANCE WITH ASTM A792. F_Y = 33 KSI (MATERIAL THICKNESS ≤ 18 GAUGE) F_Y = 50 KSI (MATERIAL THICKNESS > 18 GAUGE)
   - B. ALL SELF DRILLING SCREWS USED IN CONNECTIONS TO BE HILTI KWIK-PRO SELF DRILLING SCREWS (OR EQUAL) OF THE SIZE AND QUANTITY SHOWN.
   - C. ALL FASTENERS, ANCHORS, TIES, ETC. ARE TO BE INSTALLED EXACTLY PER MANUFACTURER'S WRITTEN INSTRUCTIONS.

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Seal:

STRUCTURAL DESIGN BY PI-TECH, INC.

GEORGIA REGISTERED No. 32195 PROFESSIONAL ENGINEER CLAIRE E. DENT

CENTER PARK RESTROOM FACILITIES AND SHADE PAVILLION

City of Centerville

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CENTERVILLE, GA
31028

REVISIONS

\# DATE DESCRIPTION

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Date:	4 NOV 2019
Drawn By:	CED
Checked By:	CED
Project Number:	2018-077

Drawing Name:

GENERAL NOTES AND DETAILS

Drawing Number:

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