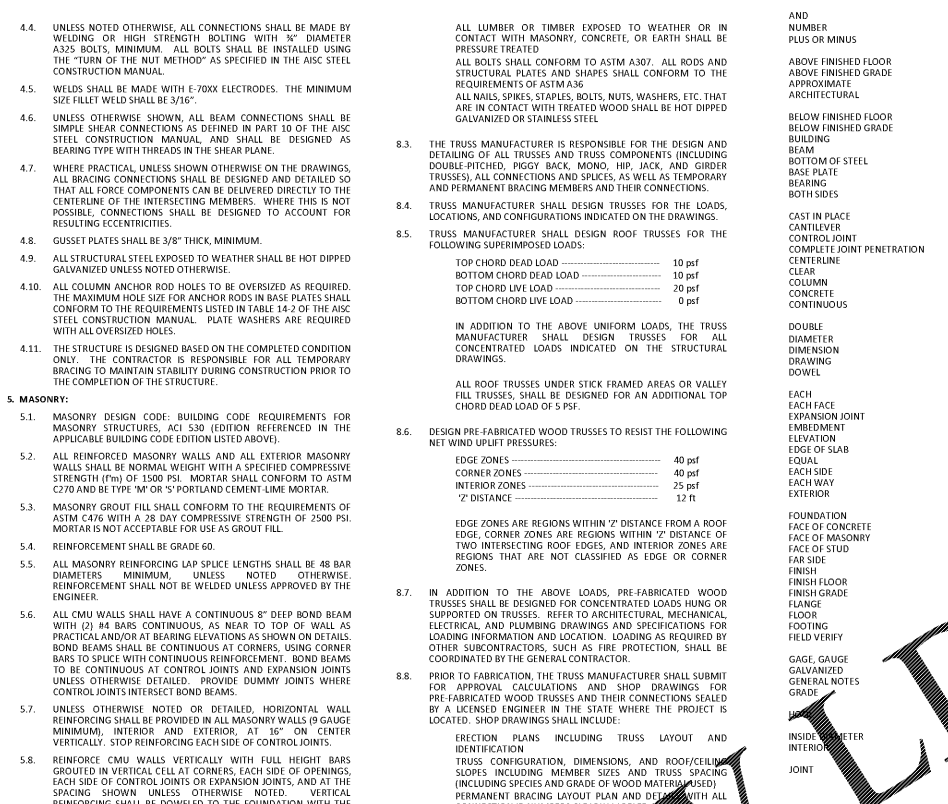


**GENERAL NOTES:**

- GENERAL:
    - GENERAL BUILDING CODE: INTERNATIONAL BUILDING CODE, 2015 EDITION.
    - THE STRUCTURAL DRAWINGS AND SPECIFICATIONS ARE PART OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL OBTAIN ALL CONTRACT DOCUMENTS AND NOTIFY THE STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES OR OMISSIONS.
    - THE CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS AND STAMP ALL SHOP DRAWINGS WITH HIS SUBMITTAL REVIEW STAMP PRIOR TO SUBMITTING THEM TO THE ARCHITECT FOR FINAL REVIEW. SHOP DRAWINGS NOT BEARING THE CONTRACTOR'S SUBMITTAL REVIEW STAMP WILL BE RETURNED WITHOUT ACTION.
    - ALL SHOP DRAWINGS RELATED TO THE ITEMS SPECIFIED IN THE STRUCTURAL CONTRACT DOCUMENTS SHALL BEAR THE ARCHITECT'S SUBMITTAL REVIEW STAMP PRIOR TO PROCEEDING. SHOP DRAWINGS WILL BE RETURNED TO THE ARCHITECT AFTER THE SUBMITTAL REVIEW IS COMPLETE.
    - DO NOT SCALE THESE DRAWINGS. WHERE DIMENSIONAL INFORMATION IS REQUIRED, OR DISCREPANCIES ARE NOTED, CONTACT THE ARCHITECT AND/OR STRUCTURAL ENGINEER.
    - CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO CONSTRUCTION AND SHALL NOTIFY ENGINEER IF ANY DISCREPANCIES ARE NOTED.
    - THE CONTRACTOR IS RESPONSIBLE FOR ALL MEANS, METHODS, AND SEQUENCE OF CONSTRUCTION.
    - THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT ALL CONSTRUCTION MATERIALS ARE SPREAD OUT ON FRAMED FLOORS/ROOF SUCH THAT THE DESIGN LOADS LISTED BELOW ARE NOT EXCEEDED.
    - DESIGN LOADS:
      - DEAD LOADS:
        - SEE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR THE CONSTRUCTION MATERIALS USED IN THE PROJECT. ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE DRAWINGS SHALL BE REPORTED TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF THE CAPACITY OF THE STRUCTURE.
      - LIVE LOADS (psf):
        - ROOF \_\_\_\_\_ 20
        - FLOORS \_\_\_\_\_ 100
      - SNOW LOADS:
        - GROUND SNOW LOAD (Pg) \_\_\_\_\_ 5.0 psf
      - D. WIND LOADS:
        - ULTIMATE DESIGN WIND SPEED (Vult) \_\_\_\_\_ 115 mph
        - NOMINAL DESIGN WIND SPEED (Vasd) \_\_\_\_\_ 90 mph
        - RISK CATEGORY \_\_\_\_\_ II
        - WIND EXPOSURE CATEGORY \_\_\_\_\_ C
        - INTERNAL PRESSURE COEFFICIENT (Gcpi) \_\_\_\_\_ +/- 0.18
        - DESIGN WIND PRESSURE FOR COMPONENTS & CLADDING (puf) \_\_\_\_\_ SEE TABLE
      - E. SEISMIC LOADS:
        - RISK CATEGORY \_\_\_\_\_ II
        - SEISMIC IMPORTANCE FACTOR (Ie) \_\_\_\_\_ 1.0
        - MAPPED SPECTRAL RESPONSE ACCELERATIONS:
          - (Sa) \_\_\_\_\_ 0.237
          - (S1) \_\_\_\_\_ 0.102
        - SITE CLASS \_\_\_\_\_ D
        - DESIGN SPECTRAL RESPONSE ACCELERATIONS:
          - (SD1) \_\_\_\_\_ 0.252
          - (SD1) \_\_\_\_\_ 0.162
        - SEISMIC DESIGN CATEGORY \_\_\_\_\_ C
        - BASE SEISMIC FORCE RESISTING SYSTEM: LIGHT-FRAME (WOOD) WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE.
        - DESIGN BASE SHEAR (Vub) \_\_\_\_\_ 7.1
        - SEISMIC RESPONSE COEFFICIENT (Cs) \_\_\_\_\_ 0.05
        - RESPONSE MODIFICATION FACTOR (R) \_\_\_\_\_ 6.5
        - ANALYSIS PROCEDURE \_\_\_\_\_ ELF
  - FOUNDATION:
    - ALL FOOTINGS AND FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF. FOOTINGS ARE TO BEAR ON UNDISTURBED SOIL. UNSATISFACTORY COMPACTED STRUCTURAL FILL AS APPROVED BY THE GEOTECHNICAL ENGINEER.
    - FOOTINGS SHALL BE NEATLY EXCAVATED WITH ALL SURFACES FREE OF LOOSE AND WET MATERIAL. WHERE NEAT EXCAVATIONS ARE NOT POSSIBLE, FOOTING EDGES SHALL BE FORMED AND BRACED. EDGES OF FOOTING SHALL BE BACKFILLED WITH COMPACTED FILL OR LEAN CONCRETE AFTER FORMS ARE REMOVED.
    - ALL FOUNDATION BEARING SURFACES SHALL BE REVIEWED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE TO ENSURE THEIR COMPLIANCE WITH RECOMMENDATIONS AND ALLOWABLE SOIL BEARING PRESSURE NOTED ABOVE. ALL FOOTING ELEVATIONS ARE ESTIMATED AND MAY REQUIRE ADJUSTMENT IN THE FIELD.
    - WHERE UNSATURABLE SOILS ARE ENCOUNTERED, THE GEOTECHNICAL ENGINEER SHALL PROVIDE RECOMMENDATIONS TO OBTAIN THE ALLOWABLE SOIL BEARING PRESSURE NOTED ABOVE (LEAN CONCRETE, APPROVED STRUCTURAL FILL, SOIL REMEDIATION, ETC.).
    - UNLESS OTHERWISE DIRECTED BY THE GEOTECHNICAL ENGINEER, PROVIDE 4" OF COMPACTED #57 STONE AND POLYETHYLENE VAPOR BARRIER UNDER ALL INTERIOR SLABS ON GRADE.
    - RETAINING WALLS SHALL BE BACKFILLED WITH #57 STONE PLACING IN A 45-DEGREE WEDGE EXTENDING FROM THE BACK OF THE FOOTING HEEL. STONE SHALL BE COMPACTED IN 1'-0" MAX LIFTS UNLESS OTHERWISE DIRECTED BY THE GEOTECHNICAL ENGINEER.
    - THE GENERAL CONTRACTOR SHALL OBTAIN A COPY OF THE GEOTECHNICAL REPORT BY I.T.L. INC. TITLED "NEW BIBB COUNTY ANNEX BUILDING", AND DATED 08/28/2019 (PROJECT NO. 00019101817400) FROM THE OWNER AND FOLLOW ALL REQUIREMENTS WITHIN THE RECOMMENDATIONS SECTION.
    - THE OWNER SHALL HIRE A GEOTECHNICAL ENGINEER TO REVIEW THE FOUNDATION BEARING SURFACE TO ENSURE THE BEARING SURFACE COMPLIES WITH THE ALLOWABLE SOIL BEARING LISTED ABOVE.
  - CONCRETE:
    - CONCRETE DESIGN CODE: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE - ACI 318 EDITION REFERENCED IN THE APPLICABLE BUILDING CODE EDITION LISTED ABOVE).
    - UNLESS OTHERWISE NOTED ON THE DRAWINGS OR IN THE SPECIFICATIONS, CONCRETE SHALL DEVELOP THE FOLLOWING MINIMUM 28 DAY COMPRESSIVE STRENGTH (F'c) AND MEET THE FOLLOWING REQUIREMENTS:
 

LOCATION	TYPE	SUMP	AIR CONTENT (%)	F'c (psi)
FOOTINGS	NORMAL WGT	3" - 5"	+3%	1000
SLAB ON GRADE	NORMAL WGT	3" - 5"	0% - 2%	1000
    - WHERE CONCRETE MIXTURES CONTAIN FLY ASH CLASS C TO REDUCE THE TOTAL CEMENTitious MATERIAL, THE MIXTURE WOULD OTHERWISE BE TESTED TO THE FULL DESIGN LIMIT, PLUS 25% BY WEIGHT, OF THE TOTAL WEIGHT OF THE CEMENTitious MATERIALS USED.
    - UNLESS OTHERWISE NOTED, REINFORCING SHALL BE MADE OF DEFORMED BARS, REFER TO ASTM A603.
    - UNLESS OTHERWISE NOTED, ALL DETAILING, FABRICATION, AND PLACING OF REINFORCING STEEL SHALL CONFORM TO THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 309-16 EDITION).
    - ALL BAR SPLICES SHALL BE CLASS "B" TENSION LAP SPLICES, AS SPECIFIED IN ACI 318, UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL NOT BE WELDED UNLESS APPROVED BY THE ENGINEER.
    - ALL EMBEDDED STRUCTURAL STEEL ITEMS, EXCEPT FOR ANCHOR RODS, SHALL CONFORM TO THE REQUIREMENTS OF ASTM A36, UNLESS NOTED OTHERWISE. ANCHOR RODS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F1554, GRADE 36, UNLESS NOTED OTHERWISE.
    - INTERSECTING WALLS SHALL BE REVEY IF POURED SEPARATELY. RUN HORIZONTAL WALL REINFORCING CONTINUOUSLY INTO INTERSECTING WALL.
  - PROVIDE (2) #4 DIAGONAL BARS IN THE TOP FACE OF SLAB ON GRADE AT ALL RE-ENTRANT CORNERS WITHOUT INTERSECTING CONTROL JOINTS IN THE SLAB ON GRADE.
  - CHAMFER ALL EXPOSED CONCRETE CORNERS WITH "N" x 45-DEGREE CHAMFER, UNLESS NOTED OTHERWISE.
  - UNLESS NOTED OTHERWISE, ALL CONCRETE SLABS ON GRADE SHALL BE 4" THICK WITH 6#6 - W2.9xw2.9 WELDED WIRE REINFORCING PLACED AT MID-DEPTH OF THE SLAB.
  - WELDED WIRE REINFORCING TO BE INSTALLED IN FLAT SLABS (NO ROLLS) AND SHALL BE SUPPORTED PROPERLY TO ENSURE THAT IT IS POSITIONED IN THE SLAB AS NOTED IN THE DRAWINGS.
  - WELDED WIRE REINFORCING SHALL LAP TWO FULL MESHES AND BE SECURELY WIRED AT EACH SIDE AND END.
  - CONTRACTOR TO REFER TO DRAWINGS OF OTHER TRADES AND VENDOR DRAWINGS FOR EMBEDDED ITEMS AND RECESSES NOT SHOWN ON THE STRUCTURAL DRAWINGS.
  - CONTRACTOR SHALL VERIFY SIZES AND LOCATIONS OF ALL MECHANICAL AND ELECTRICAL OPENINGS WITH THE MECHANICAL AND ELECTRICAL DETAILS AND SHOP DRAWINGS. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ALL OPENINGS AND SLEEVES FOR PROPER DISTRIBUTION FOR ALL UTILITY LINES THROUGHOUT THE BUILDING.
  - SHEAR KEYS TO BE 2x4 NOMINAL UNLESS NOTED OTHERWISE.
  - COORDINATE ALL FLOOR DRAIN REQUIREMENTS, INCLUDING ANY SLOPES THAT MAY BE REQUIRED AROUND THE DRAIN, WITH THE ARCHITECTURAL AND MEP DRAWINGS.
  - PROVIDE CONCRETE COVERAGE OF REINFORCEMENT AS FOLLOWS (PER ACI 318):
 

FOOTINGS	3" TOP
3" BOT & SIDES	3" BOT & SIDES
WALLS	1 1/2" EF
BEAMS	1 1/2" CLR OF TIES
PEDESTALS	1 1/2" CLR OF THIS COLUMN
1 1/2" CLR OF THIS COLUMN	1 1/2" CLR OF THIS COLUMN
SLABS EXPOSED TO WEATHER	1 1/2"
#3, #4, AND #5	1 1/2"
#6 AND LARGER	2"
SLABS NOT EXPOSED TO WEATHER	3/4"
  - PROVIDE CORNER BARS AT ALL INTERSECTING WALLS AND FOOTINGS TO SPLICE WITH ALL CONTINUOUS REINFORCEMENT.
  - ALL CONCRETE SHALL BE CURED USING WET METHODS OR CURING COMPOUND PER ACI 301. COMPLY WITH ACI FOR MIXING, TRANSPORTING, FORMING, PLACING, AND CURING CONCRETE.
  - MAXIMUM SPACING OF CONTROL JOINTS IN SLAB ON GRADE SHALL BE 15'-0" EACH WAY UNLESS SHOWN OTHERWISE ON THE PLANS. THE MAXIMUM ASPECT RATIO FOR ANY SLAB PANEL CREATED BY INTERSECTING CONTROL JOINTS (LONG SIDE/SHORT SIDE) AND/OR SLAB EDGE IS 2.0, UNLESS NOTED OTHERWISE ON THE PLANS.
  - CONCRETE FORMWORK: FORMWORK FOR ELEVATED CONCRETE SHALL BE DESIGNED, ERECTED, SHORED, BRACED, AND MAINTAINED ACCORDING TO ACI 301, TO SUPPORT VERTICAL, LATERAL, AND CONSTRUCTION LOADS THAT MIGHT BE APPLIED UNTIL STRUCTURE CAN SUPPORT SUCH LOADS. RETIGHTEN FORMS AND BRACING BEFORE PLACING CONCRETE, AS REQUIRED, TO PREVENT MORTAR LEAKS AND MAINTAIN PROPER ALIGNMENT.
  - FORM-FACING MATERIALS SHALL INCLUDE EXTERIOR GRADE PLYWOOD PANELS (HIGH-DENSITY OVERLAY, CLASS 1 OR BETTER), METAL, OR OTHER APPROVED PANEL MATERIALS.
  - FORMWORK FOR SIDES OF ELEVATED BEAMS, WALLS, COLUMNS, AND SIMILAR PARTS OF THE WORK THAT DO NOT SUPPORT THE WEIGHT OF THE CONCRETE MAY BE REMOVED 4 HOURS AFTER PLACING CONCRETE. LEAVE FORMWORK AND SHORING FOR BEAM SOFFITS, POSTS, SLABS, AND OTHER STRUCTURAL ELEMENTS THAT SUPPORT THE WEIGHT OF THE CONCRETE IN PLACE UNTIL THE CONCRETE HAS ACHIEVED AT LEAST 75% OF ITS 28 DAY DESIGN COMPRESSIVE STRENGTH BUT NOT LESS THAN 7 DAYS.
  - CONCRETE COLD-WEATHER PLACEMENT: COMPLY WITH ACI 306.1 AND AS FOLLOWS. PROTECT CONCRETE WORK FROM PHYSICAL DAMAGE OR REDUCED STRENGTH THAT COULD BE CAUSED BY FROST, FREEZING ACTIONS, OR LOW TEMPERATURES. WHEN AVERAGE HIGH AND LOW TEMPERATURE IS EXPECTED TO FALL BELOW 40 DEGREES FAHRENHEIT FOR THREE SUCCESSIVE DAYS, MAINTAIN DELIVERED CONCRETE MIXTURE TEMPERATURE WITHIN THE TEMPERATURE RANGE REQUIRED BY ACI 301. DO NOT USE FROZEN MATERIALS OR MATERIALS CONTAINING ICE OR SNOW. DO NOT PLACE CONCRETE ON FROZEN SUBGRADE OR SUBGRADE CONTAINING FROZEN MATERIALS. DO NOT USE CALCIUM CHLORIDE, SALT, OR OTHER MATERIALS CONTAINING ANTI-FREEZE AGENTS OR CHEMICAL ACCELERATORS UNLESS OTHERWISE SPECIFIED AND APPROVED IN MIXTURE DESIGNS.
  - CONCRETE HOT-WEATHER PLACEMENT: COMPLY WITH ACI 301 AND AS FOLLOWS: MAINTAIN CONCRETE TEMPERATURE BELOW 90 DEGREES FAHRENHEIT AT TIME OF PLACEMENT. CHILLED MIXING WATER OR CHIPPED ICE MAY BE USED TO CONTROL TEMPERATURE. PROVIDED WATER EQUIVALENT OF ICE IS CALCULATED TO TOTAL AMOUNT OF MIXING WATER. USING LIQUID NITROGEN TO COOL CONCRETE IS CONTRACTOR'S OPTION. FOG-SPRAY FORMS, STEEL REINFORCEMENT, AND SUBGRADE JUST BEFORE PLACING CONCRETE. KEEP SUBGRADE UNIFORMLY MOIST WITHOUT STANDING WATER, SOFT SPOTS, OR DRY AREAS.
  - CONCRETE TESTING: THE OWNER SHALL HIRE AN INDEPENDENT QUALIFIED TESTING AGENCY TO PERFORM THE CONCRETE TESTING BELOW. COMPOSITE SAMPLES OF FRESH CONCRETE OBTAINED ACCORDING TO ASTM C 172, SHALL BE TESTED BASED ON THE FOLLOWING REQUIREMENTS:
    - OBTAIN ONE COMPOSITE SAMPLE FOR EACH DAY'S POUR OF FRESH CONCRETE EXCEEDING 100 CUBIC YARDS, BUT NOT LESS THAN 20 CUBIC YARDS, PLUS ONE FOR EACH ADDITIONAL 100 CUBIC YARDS.
    - SLUMP: PER ASTM C 193. ONE TEST PER POINT OF CEMENTATION. EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST PER EACH DAY'S POUR OF FRESH CONCRETE. CONSISTENCY TESTS TO CHANGE.
    - AIR CONTENT: PER ASTM C 231. ONE TEST FOR EACH COMPOSITE SAMPLE, NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF FRESH CONCRETE MIXTURE.
    - TEMPERATURE: PER ASTM C 1064. ONE TEST HOURLY WHERE THE TEMPERATURE IS 40 DEGREES FAHRENHEIT AND BELOW, AND 80 DEGREES FAHRENHEIT AND ABOVE, AND ONE TEST FOR EACH COMPOSITE SAMPLE.
    - CONCRETE SPECIMENS: PER ASTM C 31. CAST AND LABEL. CURE THREE SETS OF TWO STANDARD CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.
    - COMPRESSIVE STRENGTH TESTS: PER ASTM C 39. TEST ONE SET OF TWO LABORATORY CURED SPECIMENS AT 7 DAYS AND ONE SET OF TWO SPECIMENS AT 28 DAYS. HOLD ADDITIONAL SET OF TWO SPECIMENS FOR 56 DAY TEST IF REQUIRED. A COMPRESSIVE STRENGTH TEST SHALL BE THE AVERAGE COMPRESSIVE STRENGTH FROM A SET OF TWO SPECIMENS OBTAINED FROM THE SAME COMPOSITE SAMPLE AND TESTED AT THE AGE INDICATED.
- STRUCTURAL STEEL:
  - STRUCTURAL STEEL DESIGN CODE: AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AISC STEEL CONSTRUCTION MANUAL (EDITION REFERENCED IN THE APPLICABLE BUILDING CODE EDITION LISTED ABOVE).
  - ALL STRUCTURAL STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
    - WIDE FLANGE SHAPES \_\_\_\_\_ ASTM A992, GR 50
    - HSS AND TS \_\_\_\_\_ ASTM A500, GR C
    - PIPE \_\_\_\_\_ ASTM A53, GR B
    - PLATES, ANGLES, AND CHANNELS \_\_\_\_\_ ASTM A36
    - ANCHOR RODS \_\_\_\_\_ SEE CONCRETE NOTES
  - ALL DETAILING, FABRICATION, AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, UNLESS NOTED OTHERWISE.
- UNLESS NOTED OTHERWISE, ALL CONNECTIONS SHALL BE MADE BY WELDING OR HIGH STRENGTH BOLTING WITH "N" DIAMETER A325 BOLTS, MINIMUM. ALL BOLTS SHALL BE INSTALLED USING THE "TURN OF THE NUT METHOD" AS SPECIFIED IN THE AISC STEEL CONSTRUCTION MANUAL.
- WELDS SHALL BE MADE WITH E-70XX ELECTRODES. THE MINIMUM SIZE FILLET WELD SHALL BE 3/16".
- UNLESS OTHERWISE SHOWN, ALL BEAM CONNECTIONS SHALL BE SIMPLE SHEAR CONNECTIONS AS DEFINED IN PART 10 OF THE AISC STEEL CONSTRUCTION MANUAL, AND SHALL BE DESIGNED AS BEARING TYPE WITH THREADS IN THE SHEAR PLANE.
- WHERE PRACTICAL, UNLESS SHOWN OTHERWISE ON THE DRAWINGS, ALL BRACING CONNECTIONS SHALL BE DESIGNED AND DETAILD SO THAT ALL FORCE COMPONENTS CAN BE DELIVERED DIRECTLY TO THE CENTERLINE OF THE INTERSECTING MEMBERS. WHERE THIS IS NOT POSSIBLE, CONNECTIONS SHALL BE DESIGNED TO ACCOUNT FOR RESULTING ECCENTRICITIES.
- GUSSET PLATES SHALL BE 3/8" THICK, MINIMUM.
- ALL STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED UNLESS NOTED OTHERWISE.
- ALL COLUMN ANCHOR ROD HOLES TO BE OVERSIZED AS REQUIRED. THE MAXIMUM HOLE SIZE FOR ANCHOR RODS IN BASE PLATES SHALL CONFORM TO THE REQUIREMENTS LISTED IN TABLE 2 OF THE AISC STEEL CONSTRUCTION MANUAL. PLATE WASHERS ARE REQUIRED WITH ALL OVERSIZED HOLES.
- THE STRUCTURE IS DESIGNED BASED ON THE COMPLETED CONDITION ONLY. THE CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY BRACING TO MAINTAIN STABILITY DURING CONSTRUCTION PRIOR TO THE COMPLETION OF THE STRUCTURE.
- MASONRY:
  - MASONRY DESIGN CODE: BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES, ACI 530 (EDITION REFERENCED IN THE APPLICABLE BUILDING CODE EDITION LISTED ABOVE).
  - ALL REINFORCED MASONRY WALLS AND ALL EXTERIOR MASONRY WALLS SHALL BE NOMINAL WEIGHT WITH A COMPRESSIVE STRENGTH (F'm) OF 1500 PSI. MORTAR SHALL CONFORM TO ASTM C 770 AND BE TYPE "M" OR "S" PORTLAND CEMENT-LIME MORTAR.
  - MASONRY GROUT FILL SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 495 WITH A 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI. MORTARS NOT ACCEPTABLE FOR USE AS GROUT FILL.
  - REINFORCEMENT SHALL BE GRADE 60.
  - ALL MASONRY REINFORCING LAP SPLICE LENGTHS SHALL BE 48 BAR DIAMETERS, MINIMUM, UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL NOT BE WELDED UNLESS APPROVED BY THE ENGINEER.
  - ALL CMU WALLS SHALL HAVE A CONTINUOUS "R" DEEP BOND BEAM WITH (2) #4 BARS CONTINUOUS, AS NEAR TO TOP OF WALL AS PRACTICAL AND/OR AT BEARING ELEVATIONS AS SHOWN ON DETAILS. BOND BEAMS SHALL BE CONTINUOUS AT CORNERS USING CORNER BARS TO SPLICE WITH CONTINUOUS REINFORCEMENT. BOND BEAMS TO BE CONTINUOUS AT CONTROL JOINTS AND EXPANSION JOINTS UNLESS OTHERWISE DETAILD. PROVIDE DUMMY JOINTS WHERE CONTROL JOINTS INTERSECT BOND BEAMS.
  - UNLESS OTHERWISE NOTED OR DETAILD, HORIZONTAL WALL REINFORCING SHALL BE PROVIDED IN ALL MASONRY WALLS (9 GAUGE MINIMUM, INTERIOR AND EXTERIOR, AT 36" ON CENTER VERTICALLY. STOP REINFORCING EACH SIDE OF CONTROL JOINTS).
  - REINFORCE CMU WALLS VERTICALLY WITH FULL HEIGHT BARS GROUDED IN VERTICAL CELL AT CORNERS, EACH SIDE OF OPENINGS, EACH SIDE OF CONTROL JOINTS OR EXPANSION JOINTS, AND AT THE SPACING SHOWN UNLESS OTHERWISE NOTED. VERTICAL REINFORCING SHALL BE DOWELED TO THE FOUNDATION WITH THE SAME SIZE AND SPACING AND SHALL EXTEND THROUGHOUT THE BOND BEAM AT THE TOP OF WALL AND TERMINATE 2" FROM TOP OF MASONRY WALL.
  - GROUT FILL ALL REINFORCED CELLS FULL HEIGHT. GROUT FILL ALL CELLS OF MASONRY BELOW FINISHED FLOOR ELEVATION.
  - VERTICAL REINFORCING STEEL SHALL BE HELD IN POSITION AT INTERVALS OF 48 INCHES MAX. WIRE TIES OR SPECIALTY PRODUCTS MAY BE USED TO ACHIEVE THE REQUIRED VERTICAL BAR SUPPORT.
  - CMU SHALL BE LAID IN RUNNING BOND.
  - TEMPORARY SHORING OF THE MASONRY WALLS SHALL BE PROVIDED BY THE CONTRACTOR UNTIL THE STRUCTURAL FRAME, CONNECTIONS, AND SUPPORTING ELEMENTS ARE FULLY INSTALLED.
- WOOD FRAMING:
  - WOOD DESIGN CODE: NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION, BY THE AMERICAN WOOD COUNCIL (EDITION REFERENCED IN THE APPLICABLE BUILDING CODE EDITION LISTED ABOVE) AND THE DESIGN VALUES FOR WOOD CONSTRUCTION PROVIDED IN THE NDS SUPPLEMENT.
  - STRUCTURAL LUMBER:
    - ALL STRUCTURAL LUMBER (INCLUDING STUDS IN LOAD BEARING WALLS, SHALL BE SOUTHERN PINE NO. 2 (MOISTURE CONTENT OF 19% OR LESS), OR SPOKES OR LVL'S, AND AT THE ALL LUMBER OR TIMBER EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE, OR EARTH SHALL BE PRESURE TREATED.
    - LOAD BEARING WALL STUDS SHALL BE BACKSICK AT MID-HEIGHT OF EACH FLOOR BLOCKING TO THE SAME WIDTH AS THE WALL.
    - PROVIDE A MINIMUM OF (2) SPOKES BELOW ALL GIRDER TRUSS AND/OR BEAM BEARING LOCATIONS.
    - ANCHOR BOLTS SHALL CONFORM TO ASTM A307. ALL BOLTS AND STRUCTURAL PLATES AND SHAPES SHALL CONFORM TO THE REQUIREMENTS OF ASTM A36.
    - NAILS, STAPLES, BOLTS, NUTS, WASHERS, ETC. THAT COME IN CONTACT WITH TREATED WOOD SHALL BE HOT DIPPED GALVANIZED OR STAINLESS STEEL.
  - WOOD SHEATHING:
    - SOFT SHEATHING SHALL BE 5/8" TONGUE AND GROOVE APA RATED SHEATHING, EXPOSURE 1, WITH A MINIMUM SPAN RATING OF 40/20. FASTEN WITH 10d COMMON NAILS AT 6" ON CENTER AT EDGES AND 12" ON CENTER AT INTERIOR SUPPORTS.
    - SHEAR WALL SHEATHING SHALL BE 15/32" MINIMUM APA RATED SHEATHING, EXPOSURE 1 WITH A MINIMUM SPAN RATING OF 32/16. SEE DRAWINGS FOR SHEAR WALL LOCATION AND NAILING REQUIREMENTS.
  - ROOF SHEATHING SHALL BE INSTALLED AS INDICATED BELOW:
    - SHEATHING SHALL SPAN PERPENDICULAR TO DIRECTION OF SUPPORTING MEMBERS AND BE CONTINUOUS OVER 2 OR MORE SPANS.
    - ADJACENT PANELS SHALL BE ATTACHED TO SAME SUPPORTING MEMBERS.
    - END JOINTS SHALL BE STAGGERED.
    - PANELS SHALL BE ATTACHED TO SUPPORTING MEMBERS AT EACH END AND AT ALL INTERMEDIATE BEARING LOCATIONS.
    - PROVIDE BLOCKING AT PANEL EDGES BETWEEN TRUSSES AS REQUIRED.
    - WHERE TONGUE AND GROOVE EDGES ARE NOT PROVIDED, H-CUPS ARE REQUIRED ON ALL UNBLOCKED SHEATHING EDGES (REFERENCE TYPICAL DETAILS).
- LAMINATED VENEER LUMBER (LVL)
  - ALL LVL'S SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:
    - F<sub>b</sub> \_\_\_\_\_ 3100 psi
    - E \_\_\_\_\_ 285 psi
    - F<sub>v</sub> \_\_\_\_\_ 2000 ksi
- PRE-FABRICATED METAL PLATE CONNECTED WOOD TRUSSES:
  - ALL PRE-FABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION (ANSI/TPI 1), BY THE TRUSS PLATE INSTITUTE (TPI) (EDITION REFERENCED IN THE APPLICABLE BUILDING CODE EDITION LISTED ABOVE).
  - WOOD TRUSS MEMBERS:
    - ALL TRUSS MEMBERS SHALL BE SOUTHERN PINE NO. 2 (MOISTURE CONTENT OF 19% OR LESS), OR STRONGER.
    - ALL TRUSS TOP AND BOTTOM CHORD MEMBERS SHALL BE 2x4, MINIMUM.



**ABBREVIATIONS:**

&	KIP	K
NO. / #	LONG	LG
+/-	MAXIMUM	MAX
ABOVE FINISHED FLOOR	MOMENT CONNECTION	MC
APPROXIMATE	MECHANICAL	MCH
ARCHITECTURAL	MANUFACTURE(R)	MFR
BELOW FINISHED FLOOR	MINIMUM	MISC
BFF	MISCELLANEOUS	MIS
BEG	METAL	MTL
BUILDING	NEAR SIDE	NS
BM	NOT TO SCALE	NIS
BOS	ON CENTER	OC
BP	ON-DIAMETER	OD
BRG	OPENING	OPNG
BOTH SIDES	OPPOSITE HAND	OPPHD
CANTILEVER	PRE-ENGINEERED METAL BUILDING	PEMB
CONTROL JOINT	PARTIAL JOINT PENETRATION	PJP
CENTERLINE	PERPENDICULAR	PERP
CL	PLATE	PL
COL	PLUMBING	PLMB
CONC	PROTECTION	PROI
CONT	POUNDS PER SQUARE FOOT	PSF
	POUNDS PER SQUARE INCH	PSI
DOUBLE DIAMETER	RADIUS	R
DIA	REFERENCE	REF
DM	REINFORCING	REINF
DWG	REQUIRED	REQ'D
DWL	ROUGH OPENING	R.O.
EACH EACH FACE	SCHEDULE	SCHED
EXPANSION JOINT	SECTION	SECT
ELEVATION	SIMILAR	SIM
EDGE OF SLAB	SLAB ON GRADE	SOG
EQUAL	SEISMIC LOAD RESISTING SYSTEM	SLRS
EQUAL EACH SIDE	SPECIFICATIONS	SPC
EXTERIOR	SQUARE	SQ
	STAGER	STAG
	STANDARD	STD
	STIFFENER	STIFN
	STRIP	STRIP
	STEEL	STL
	STRUCTURAL	STR
	SYMMETRICAL	SYM
	TOP	TOP
	TOP AND BOTTOM	T&B
	THICK	THK
	TOP OF CONCRETE	TOC
	TOP OF DECK	TOP
	TOE	TOE
	TOE OF SHEATHING	TS
	TOP OF SHEET	TOS
	TOP OF WALL	TOW
	TYPICAL	TYP
	UNLESS NOTED OTHERWISE	UNO
	VERTICAL	VERT
	WITH WELDED WIRE REINFORCEMENT	W/W
	WEIGHT	WGT

**COMPONENTS & CLADDING WIND PRESSURES**

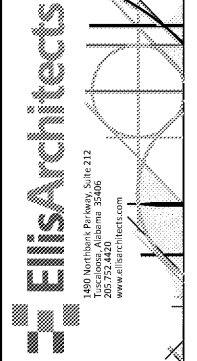
ZONE	EFFECTIVE WIND AREA (ft²)	MAX +ve PRESSURE (psf)	MAX -ve PRESSURE (psf)
1	10	10.0	-24.2
	20	10.0	-23.7
	50	10.0	-23.2
	100	10.0	-22.2
	10	10.0	-40.7
	20	10.0	-36.6
	50	10.0	-30.4
	100	10.0	-26.3
	10	10.0	-61.2
	20	10.0	-51.0
2	50	10.0	-26.3
	100	10.0	-22.2
	20	21.3	-23.1
	50	20.3	-22.2
	100	18.5	-20.3
	500	16.6	-18.5
	10	22.2	-29.6
	20	21.3	-27.7
	50	20.3	-25.0
	100	18.5	-23.1
3	500	16.6	-18.5
	500	16.6	-18.5

**CLASS "B" TENSION LAP SPLICE TABLE**

BAR SIZE	d (in)	F <sub>y</sub> = 3000 psi		F <sub>y</sub> = 4000 psi	
		TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	0.375	28"	22"	24"	19"
#4	0.500	37"	29"	32"	25"
#5	0.625	47"	36"	40"	31"
#6	0.750	56"	43"	48"	37"
#7	0.875	81"	63"	70"	54"
#8	1.000	93"	72"	80"	62"
#9	1.128	105"	81"	91"	70"
#10	1.270	118"	91"	102"	79"
#11	1.410	131"	101"	113"	87"

- TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE DEVELOPMENT LENGTH OR SPLICE.
- CLEAR COVER AND SPACING MUST MEET THE FOLLOWING:
  - BEAMS/COLUMNS: COVER AT LEAST 1d.
  - C-C SPACING AT LEAST 2d.
  - COVER AT LEAST 1d.
  - C-C SPACING AT LEAST 3d.
- TABLE IS BASED ON REINFORCING BAR YIELD STRENGTH (F<sub>y</sub>) OF 60 ksi AND NORMAL WEIGHT CONCRETE. WHERE LIGHT WEIGHT CONCRETE IS USED, SPLICE LENGTH SHALL BE INCREASED 30%.
- TO OBTAIN CLASS "A" TENSION LAP SPLICE LENGTHS (DEVELOPMENT LENGTH), DIVIDE SPLICE LENGTHS BY 1.3.

\*"a" DISTANCE = 6.6 FEET  
 ZONE 1 = INTERIOR ROOF  
 ZONE 2 = ROOF WITHIN "a" OF BUILDING EDGE  
 ZONE 3 = ROOF WITHIN "a" OF (2) EDGES (CORNER ZONE)  
 ZONE 4 = INTERIOR WALLS  
 ZONE 5 = WALLS WITHIN "a" OF BUILDING EDGE



New Construction for:  
**BIBB COUNTY COURTHOUSE ANNEX**  
 8 Court Square West  
 Centreville, Alabama



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Do Not Scale from Drawings.  
 Contractor must verify all dimensions prior to construction.

JOB No.	18004
DOCUMENT	DATE
FINAL CDS	09/06/2019
DRAWN BY:	SJD

GENERAL NOTES  
**S0.1**