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THE CENTER FOR CONNECTIVE ARCHITECTURE

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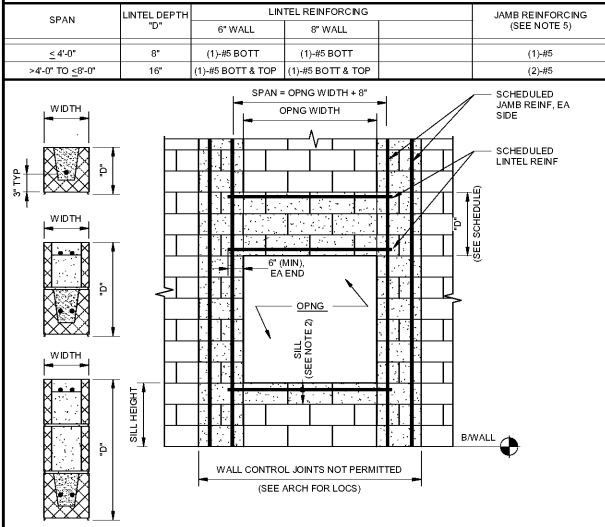


ISSUANCES		
No.	Drawing Issue Description	Date
1	ISSUED FOR B/D	11/04/19

ABBREVIATIONS

ACI	AMERICAN CONCRETE INSTITUTE	K	KIPS (KILOPOUNDS)
ADDL	ADDITIONAL	KLF	KIPS PER LINEAL FOOT
AESS	ARCHITECTURAL EXPOSED STRUCTURAL STEEL	KSI	KIPS PER SQUARE INCH
AFF	ABOVE FINISHED FLOOR	KSF	KIPS PER SQUARE FOOT
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	L	LENGTH
AISI	AMERICAN IRON AND STEEL INSTITUTE	LFH	LONG FACE HORIZONTAL
ALTN	ALTERNATE	LFV	LONG FACE VERTICAL
AR	ANCHOR ROD	LG	LONG
ARCH	ARCHITECT	LL	LIVE LOAD
ASD	ALLOWABLE STRESS DESIGN	LLH	LONG LEG HORIZONTAL
ASTM	AMERICAN SOCIETY OF TESTING AND MATERIALS	LLV	LONG LEG VERTICAL
AWS	AMERICAN WELDING SOCIETY	LO	LOW
B	BOTTOM OF BOARD	LOCs	LOCATIONS
BD	BOARD	LRFD	LOAD RESISTANCE FACTORED DESIGN
BETW	BETWEEN	LSH	LONG SIDE HORIZONTAL
BLDG	BUILDING	LSV	LONG SIDE VERTICAL
BM	BEAM	LW	LONG WAY
BOT	BOTTOM	LWC	LIGHT WEIGHT CONCRETE
BP	BASE PLATE	MAX	MAXIMUM
BRDG	BRIDGING	MEP	MECHANICAL, ELECTRICAL & PLUMBING
BRG	BEARING	MEZZ	MEZZANINE
C/C	CENTER-CENTER	MFR	MANUFACTURER
CFSF	COLD FORMED STEEL FRAMING	MN	MINIMUM
CJ	CONTROL JOINT	MISC.	MISCELLANEOUS
CL	CENTERLINE	MPII	MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS
CLR	CLEAR	MTL	METAL
CMU	CONCRETE MASONRY UNIT	NC	NOT IN CONTACT
COL	COLUMN	NS	NEAR SIDE
CONC	CONCRETE	NTS	NOT TO SCALE
CONC	CONNECTION	OC	ON CENTER
CONT	CONTINUOUS	OD	OUTSIDE DIAMETER
CTR	CENTER	OPNG	OPENING
DAE	DRILL & EPOXY	PAF	POWDER ACTUATED FASTENERS
D	DEEP	PEMB	PRE-ENGINEERED METAL BUILDING
DBA	DEFORMED BAR ANCHOR	PJF	PREFORMED JOINT FILLER
DBL	DOUBLE	PL	PLATE
DEP	DERESSED	PLF	POUNDS PER LINEAL FOOT
DLA	DIMETER	PPHCC	PRESTRESSED PRECAST HOLLOW CORE CONCRETE
DLG	DIAGONAL	PREFAB	PRE-FABRICATED
DL	DEAD LOAD	PSI	POUNDS PER SQUARE INCH
DWL	DOWEL	PSF	POUNDS PER SQUARE FOOT
DN	DOWN	PT	POST TENSIONED
EA	EACH	P.T.	PRESSURE TREATED
EAF	EACH FACE	QTY	QUANTITY
EJ	EXPANSION JOINT	RAD	RADIUS
ELEV	ELEVATION	RD	ROOF DRAIN
ENG	ENGINEER OR ENGINEERING	REF	REFERENCE
EOS	EDGE OF SLAB	REINF	REINFORCING
EQ	EQUAL	REQD	REQUIRED
EW	EACH WAY	REV	REVISION
EXIST	EXISTING	RTU	ROOF TOP UNIT
EXP	EXPANSION	SCHED	SCHEDULE
EXT	EXTERIOR	SER	STRUCTURAL ENGINEER OF RECORD
F	FACE OF	SF	SQUARE FOOT
FD	FLOOR DRAIN	SHNG	SHEATHING
FDN	FOUNDATION	SH	SHEATHING
FF	FINISH FLOOR	SM	SIMILAR
FLR	FLOOR	SLH	SHORT LEG HORIZONTAL
FRT	FIRE RETARDANT TMBER	SLV	SHORT LEG VERTICAL
FS	FAR SIDE	SPA	SPACES
FTG	FOOTING	SPEC	SPECIFICATION
FV	FIELD VERIFY	SS	STAINLESS STEEL
GA	GAUGE, GAGE	STD	STANDARD
GALV	GALVANIZED	STIFF	STIFFENER
GC	GENERAL CONTRACTOR	STL	STEEL
GDR	GRIDER	SW	SHORT WAY
GENL	GENERAL	SYM	SYMMETRICAL
GYP	GYPSSUM	TI	TOP
HCA	HEADED CONCRETE ANCHORS	T&B	TOP & BOTTOM
HDR	HEADER	T&G	TONGUE & GROOVE
HGR	HANGER	TEMP	TEMPORARY
HI	HIGH	TRG	TRUSS GRIDER
HID	HOOKED	THK	THICKENED or THICK
HORIZ	HORIZONTAL	THRU	THROUGH
HSS	HOLLOW STRUCTURAL SECTION	TYP	TYPICAL
H.T.	HEAVY TIMBER	UNO	UNLESS NOTED OTHERWISE
ID	INSIDE DIAMETER	VERT	VERTICAL
IW	INVERT	W	WIDE
ISL	INSULATION	WI	WITH
INSUL	INSULATION OR INSULATING	WO	WOOD
INT	INTERIOR	WP	WORK POINT
JST	JOIST	WWR	WELDED WIRE REINFORCEMENT
JOINT	JOINT		

CONCRETE MASONRY UNIT LINTEL & JAMB SCHEDULE



SECTIONS ELEVATION

- NOTES:
- LINTELS SHALL BE CONSTRUCTED WITH U-BLOCKS AT THE BOTTOM AND DEPRESSED WEB BLOCKS ABOVE AND SHALL BE FULLY GROUTED.
 - SILL REINFORCING REQUIREMENTS:
a. OPENING WIDTH ≤ 6'-0" AND SILL HEIGHT ≤ 3'-0": SILL REINFORCING NOT REQUIRED.
b. OPENING WIDTH > 6'-0" OR SILL HEIGHT > 3'-0": PROVIDE REINFORCED LINTEL ACCORDING TO SCHEDULE.
 - NOTIFY STRUCTURAL ENGINEER OF RECORD IF OPENING WIDTH EXCEEDS SCHEDULED WIDTHS.
 - SEE CONCRETE MASONRY GENERAL NOTES AND RELEVANT SCHEDULES, SECTIONS, AND DETAILS FOR ADDITIONAL REINFORCING AND REQUIREMENTS NOT SHOWN IN THIS SCHEDULE.
 - PROVIDE (1) BAR PER CELL IN JAMBS, TYPICAL. WHERE SCHEDULED JAMB REINFORCING EXCEEDS THE NUMBER OF CELLS, PROVIDE (2) BARS PER CELL LOCATED 3/4" CLEAR FROM EACH FACE SHELL.

CAST-IN-PLACE CONCRETE (NONPRESTRESSED) CLEAR COVER SCHEDULE

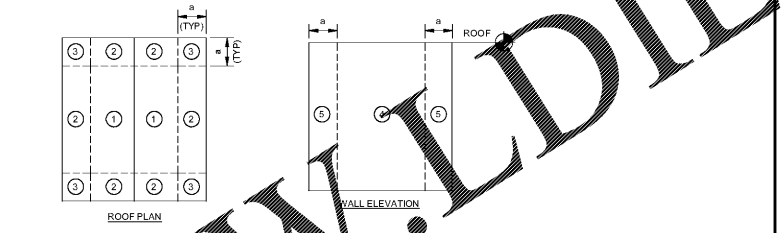
CONCRETE CAST AGAINST AND PERMANENTLY IN CONTACT WITH GROUND	CONCRETE COVER
3 IN	3 IN
CONCRETE IN CONTACT WITH GROUND OR WEATHER:	
#5 THROUGH #18 BARS	2 IN
#5 BAR, W31 OR D31 WIRE, AND SMALLER	1 1/2 IN
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS, WALLS, JOISTS:	1 1/2 IN
#14 AND #18 BARS	3/4 IN
#11 BAR AND SMALLER	
BEAMS, COLUMNS:	1 1/2 IN
PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS	

COMPONENTS & CLADDING EXTERNAL PRESSURE LOADS (PSF)

IBC 2012/2015, LOCATION PER ASCE 7-10, FIGURE 30.4-1, 30.4-2A

EFFECTIVE WIND AREA (FT²)	1	2	3	4	5
<10	16.6	16.6	16.6	28.8	28.8
	-26.4	-45.9	-67.9	-31.3	-38.6
20	16.0	16.0	16.0	27.5	27.5
	-25.6	-42.2	-63.5	-30.0	-36.0
50	16.0	16.0	16.0	25.8	25.8
	-24.7	-37.4	-57.7	-28.3	-32.6
>100	16.0	16.0	16.0	24.5	24.5
	-23.9	-33.7	-53.3	-27.0	-30.0
>500				21.5	21.5
				-23.9	-23.9

NOTES:
1. a = 5'-0". SEE ROOF PLAN MAP BELOW FOR LOCATION OF a-ZONES. WALL a-ZONE LOCATIONS TO MATCH ROOF a-ZONES.
2. POSITIVE PRESSURE VALUES REFER TO FORCES ACTING TOWARDS BUILDING OR COMPONENT FACE. NEGATIVE PRESSURE VALUES REFER TO FORCES ACTING AWAY FROM BUILDING OR COMPONENT FACE.
3. EACH COMPONENT AND ITS CONNECTION SHALL BE DESIGNED FOR MAXIMUM POSITIVE AND NEGATIVE FORCES.
4. FOR COMPONENTS HAVING EFFECTIVE AREAS IN BETWEEN TABULATED VALUES, DESIGN LOADS MAY BE INTERPOLATED. OTHERWISE DESIGN LOAD SHALL BE TAKEN FROM THE NEXT LOWEST TABULATED EFFECTIVE AREA.
5. DESIGN VALUES SHOWN IN THIS TABLE ARE ULTIMATE VALUES FOR USE WITH LRFD DESIGN. VALUES MAY BE MULTIPLIED BY 0.6 FOR USE WITH SERVICE LEVEL OR ASD DESIGN. REFER TO THE BUILDING CODE FOR APPLICABLE LOAD COMBINATIONS.



NOTES:
6. ROOF CLADDING DESIGN WIND PRESSURE VALUES WHICH SHALL BE APPLIED AT ROOF OVERHANGS TO TOP SURFACE OF CLADDING OR SHEATHING AND ITS CONNECTION. SOFFIT CLADDING OR SHEATHING SHALL BE DESIGNED FOR SIMILAR PRESSURE TO THE ADJACENT WALL PRESSURE. A COMBINATION OF THESE FORCES SHALL BE APPLIED TO THE STRUCTURAL ELEMENT OF THE OVERHANG AND ITS CONNECTION, INCLUDING BUT NOT LIMITED TO THE STUD FRAMING OF THE OVERHANG.

EFFECTIVE WIND AREA (FT²)	ROOF	ROHS
<10	-28.1	-28.1
50	-58.1	-74.5
>500	-58.1	-65.5

CLASS B TENSION LAP SPlice LENGTHS (ACI 318, SECTION 12.2.2 AND 12.15)

BAR SIZE	F'c = 3000 PSI				F'c = 4000 PSI			
	TOP BARS		OTHER BARS		TOP BARS		OTHER BARS	
	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2
#3	28	42	21	32	24	36	18	28
#4	37	56	28	43	32	48	25	37
#5	46	69	36	53	40	60	31	46
#6	56	83	43	64	48	72	37	53
#7	81	131	62	93	70	105	53	74
#8	93	139	71	107	80	120	62	87
#9	104	157	80	120	90	135	70	97
#10	118	176	90	136	102	153	78	117
#11	131	196	100	151	113	170	87	130

- NOTES:
- TABULATED VALUES ARE BASED ON MINIMUM 6" STRENGTH OF BARS. LENGTHS ARE IN INCHES.
 - CASE 1 AND CASE 2 DEPEND ON THE TYPE OF STRUCTURAL MEMBER, CONCRETE COVER, AND BAR SPACING AND ARE DEFINED AS FOLLOWS:
CASE 1: CLEAR SPACING ≥ 2.0 BAR DIA
CASE 2: CLEAR SPACING < 2.0 BAR DIA
COLUMN: CASE 1: CONCRETE COVER ≥ 1.0 BAR DIA AND CLEAR SPACING ≥ 2.0 BAR DIA; CASE 2: CONCRETE COVER < 1.0 BAR DIA OR CLEAR SPACING < 2.0 BAR DIA
WALL: CASE 1: CONCRETE COVER ≥ 1.0 BAR DIA AND CLEAR SPACING ≥ 2.0 BAR DIA; CASE 2: CONCRETE COVER < 1.0 BAR DIA OR CLEAR SPACING < 2.0 BAR DIA
 - TOP BARS: HORIZONTAL BARS WITH MORE THAN 12 INCHES OF FRESH CONCRETE PLACED BELOW THE DEVELOPMENT LENGTH OR SPlice.
 - REBAR IS ASSUMED TO BE UNCOATED (NO EPOXY COATING). INCREASE LAP SPlice AND DEVELOPMENT LENGTHS SHOWN BY 1.3 FOR TOP, AND 1.5 FOR OTHER EPOXY COATED BARS.
 - FOR LIGHTWEIGHT CONCRETE, MULTIPLY TABULATED VALUES BY 1.3.
 - LAP SPlice LENGTHS SHALL BE AS SHOWN IN THE TABLE ABOVE, UNLESS NOTED OTHERWISE.
 - WHERE BARS OF DIFFERENT SIZES ARE LAP SPliced, THE LAP SPlice LENGTH SHALL BE THE LARGER OF THE TENSION DEVELOPMENT LENGTH OF THE LARGER BAR AND THE TENSION LAP SPlice LENGTH OF THE SMALLER BAR.

TENSION DEVELOPMENT LENGTHS (ACI 318, SECTION 12.2.2)

BAR SIZE	F'c = 3000 PSI				F'c = 4000 PSI			
	TOP BARS		OTHER BARS		TOP BARS		OTHER BARS	
	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2
#3	21	32	16	25	18	28	14	21
#4	28	43	22	33	25	37	19	28
#5	36	53	27	41	31	46	24	36
#6	43	64	33	49	37	55	28	43
#7	62	93	48	72	54	81	42	62
#8	71	107	55	82	62	92	47	71
#9	80	120	62	93	70	104	54	80
#10	90	136	70	104	78	117	60	90
#11	100	151	77	116	87	130	67	100

CONCRETE MASONRY UNITS REINFORCING LAP SPlICE LENGTHS

SIZE	BAR SIZE							
	#3	#4	#5	#6	#7	#8	#9	#10
8" CMU	16"	21"	26"	43"	60"	M	M	
12" CMU	16"	21"	26"	40"	46"	61"	74"	

- NOTES:
- F_{tm} = 1500 psi
 - REBAR IS ASSUMED TO BE UNCOATED (NO EPOXY COATING)
 - REBAR IS LOCATED IN CENTER OF CELL.
 - "M" DENOTES MECHANICAL BAR SPlICE IS REQUIRED. SPlICE LENGTH SHALL BE DEVELOPMENT LENGTH OF THE BAR IN TENSION OR COMPRESSION.

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SF1.01

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GENERAL SCHEDULES

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ISSUED FOR CONSTRUCTION