

GENERAL:

- 1. THESE NOTES APPLY UNLESS OTHERWISE INDICATED BY DRAWINGS OR SPECIFICATIONS.
2. STRUCTURAL DRAWINGS INDICATE TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY. SHOP DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH THE SPECIFIED STANDARDS AND THE SPECIFIC REQUIREMENTS OF THIS PROJECT AS INDICATED ON THE DRAWINGS.
3. THE STRUCTURAL TENANT IMPROVEMENTS ARE DESIGNED TO THE 2012 INTERNATIONAL BUILDING CODE, REFERENCED MATERIAL CODES, AND GEORGIA AMENDMENTS.
4. THE STRUCTURE SHOWN IN THESE DRAWINGS IS STRUCTURALLY SOUND ONLY IN ITS COMPLETED FORM. THE CONTRACTOR SHALL TEMPORARILY BRACE ALL EARTH, FORMS, CONCRETE, STEEL, WOOD, MASONRY, ETC. TO RESIST GRAVITY, EARTH, WIND, SEISMIC, AND CONSTRUCTION LOADS DURING THE CONSTRUCTION PERIOD.

EXISTING CONDITIONS:

- 1. THE CONTRACTOR SHALL SURVEY THE EXISTING SITE TO DETERMINE THAT ALL STRUCTURES AS INDICATED IN THE DRAWINGS CAN BE CONSTRUCTED AS SHOWN AND SHALL REPORT ANY DISCREPANCIES OR UNUSUAL CONDITIONS, SUCH AS EXISTING STRUCTURES AND UTILITIES, TO THE ENGINEER.
2. FIELD MEASUREMENTS SHALL BE TAKEN AS REQUIRED TO DIMENSION ALL STRUCTURAL ELEMENTS PRIOR TO THE SUBMISSION OF SHOP DRAWINGS.

STRUCTURAL OBSERVATIONS:

- 1. THE CONTRACTOR SHALL PROVIDE THE STRUCTURAL ENGINEER ONE WEEK NOTICE PRIOR TO ALL OF THE FOLLOWING CONSTRUCTION ACTIVITIES SO THAT THE APPROPRIATE STRUCTURAL OBSERVATION MAY BE PERFORMED: (EVEN IF WORK IS SEPARATED INTO PHASES)
- PLACEMENT OF NEW CONCRETE
- METAL DECK INSTALLATION
- SHEAR WALL SHEATHING INSTALLATION
2. IF ANY FIELD CONDITION FOR LAYOUT OR CONNECTIONS VARIES FROM THESE DRAWINGS, CONTACT THE SPECIALTY ENGINEER FOR REVIEW AND SUBMITTAL TO THE STRUCTURAL ENGINEER OF RECORD.

SHALLOW FOUNDATIONS:

- 1. THE DESIGN ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED TO BE 2000 PSF AND SOILS SHALL BE PREPARED IN ACCORDANCE WITH THE RECOMMENDATIONS OF A QUALIFIED GEOTECHNICAL ENGINEER. CONDITIONS SHALL BE VERIFIED BY A QUALIFIED GEOTECHNICAL ENGINEER REGISTERED IN GEORGIA PRIOR TO THE PLACEMENT OF CONCRETE.
2. ALL FOOTINGS SHALL BEAR ON ORIGINAL, UNDISTURBED SOIL, WHERE POSSIBLE. IF BACKFILL OR REMEDIATION IS REQUIRED, IT SHALL BE PREPARED BASED ON THE RECOMMENDATIONS OF A QUALIFIED GEOTECHNICAL ENGINEER REGISTERED IN GEORGIA.
3. ANY FILL PLACED WITHIN 10'-0" OF THE BUILDING PERIMETER SHALL BE COMPACTED BASED ON THE RECOMMENDATIONS OF A QUALIFIED GEOTECHNICAL ENGINEER REGISTERED IN GEORGIA.
4. ALL EXTERIOR FOOTINGS AND EXTERIOR SLAB TURNDOWNS SHALL BEAR A MINIMUM OF 16" BELOW EXTERIOR FINISHED GRADE OR AS DIRECTED BY A QUALIFIED GEOTECHNICAL ENGINEER REGISTERED IN GEORGIA.

SHOP DRAWINGS:

- 1. WRITTEN PERMISSIONS MUST BE OBTAINED FROM D. ERIC SMITH, P.E. PRIOR TO THE REPRODUCTIVE USE OF THE STRUCTURAL CONTRACT DOCUMENTS IN ANY FASHION AS STRUCTURAL SHOP DRAWING DOCUMENTS.
2. THE FOLLOW SHOP DRAWINGS AND MATERIAL DATA SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW. A MINIMUM OF 4 COPIES OF EACH ARE REQUIRED.
- CONCRETE MIX DESIGNS WITH TEST DATA (30 SAMPLES) AND MATERIAL CERTIFICATES FOR EACH TYPE OF CONCRETE
- REINFORCING STEEL (SHALLOW FOUNDATIONS, FLOOR, CONCRETE WALLS, & SLAB ON GRADE)
- METAL ROOF DECK, STEEL JOIST, & STEEL JOIST GIRDERS
- STRUCTURAL STEEL & MISCELLANEOUS STEEL
- CONCRETE MASONRY & REINFORCING STEEL DETAILS
- METAL STUD MATERIAL SUBMITTALS
3. COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF ALL APPLICABLE SPECIALTY ITEMS INCLUDING, BUT NOT LIMITED TO, PRECAST CONCRETE, CURTAIN WALL GLAZING SYSTEMS, LIGHT GAGE METAL FRAMING, ORNAMENTAL GUARD RAILS, SKYLIGHTS, AND STAIRS SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN GEORGIA AND SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.

DESIGN LOADS:

- 1. BUILDING LOADS:
A. DEAD LOADS:
- ROOF DEAD LOAD: 13 PSF
- FLOOR DEAD LOAD: 20 PSF
B. LIVE LOADS:
- ROOF LIVE LOAD: 20 PSF
- FLOOR LIVE LOAD: 80 PSF
- REDUCED AS ALLOWED BY THE BUILDING CODE
- 3000# CONCENTRATED LOAD OVER 30"x30"
C. SNOW LOADS:
- GROUND SNOW LOAD (pg): 5 PSF
- ROOF SNOW LOAD (pf): 8.5 PSF
- SNOW IMPORTANCE FACTOR (Is): 1.0
- SNOW THERMAL FACTOR (st): 1.0
- SNOW EXPOSURE FACTOR (ce): 0.9
E. WIND LOAD:
- BASE ULTIMATE WIND SPEED: V = 115 MPH
- BASE SERVICE WIND SPEED: V = 90 MPH
- IMPORTANCE FACTOR: I = 1.0
- BUILDING CATEGORY: II
- INTERNAL PRESSURE COEFFICIENT: 0.18
- EXPOSURE CATEGORY: B
- UNLESS NOTED OTHERWISE, PRESSURES GIVEN ARE SERVICE LOADS.
- COMPONENTS & CLADDING FOR WALL: 18 PSF SERVICE (AREA IS GREATER THAN 100 SF)
F. EARTHQUAKE:
- SEISMIC IMPORTANCE FACTOR: I = 1.0
- MAPPED SPECTRAL RESPONSE ACCELERATIONS
- S1 = 0.223
- S1 = 0.097
- SITE CLASS IS ASSUMED TO BE "D"
- SPECTRAL RESPONSE COEFFICIENTS
- Sps = 0.238
- Sps = 0.155
- SEISMIC FORCE RESISTING SYSTEMS
- ORDINARY SHEATHED FRAMED BEARING WALLS
- SEISMIC RESPONSE COEFFICIENT: C = 0.04
- DESIGN BASE SHEAR (MEZZANINE)
- 6 KIPS
- RESPONSE MODIFICATION FACTOR: R = 6.5
- SEISMIC DESIGN CATEGORY: B
- ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE (IBC 1617.1, ASCE 7-05 SECTION 9.5.5)

STEEL JOIST AND JOIST GIRDERS:

- 1. STRUCTURAL STEEL JOISTS AND JOIST GIRDERS SHALL BE FABRICATED AND ERECTED IN STRICT CONFORMANCE TO THE LATEST EDITION OF THE STEEL JOIST INSTITUTE STANDARDS. JOIST ERECTION PROCEDURE SHALL CONFORM STRICTLY TO S.J.I. STANDARDS. STEEL JOISTS AND JOIST GIRDERS SHALL BE MANUFACTURED BY A CURRENT MEMBER OF S.J.I.
2. CONTRACTOR IS TO SUBMIT DESIGN CALCULATIONS FOR ALL JOISTS AND JOIST GIRDERS WITH CONCENTRATED LOADS FROM OTHER STRUCTURAL COMPONENTS OR EQUIPMENT AND NET UPLIFT LOADS. PROVIDE SPECIAL MARKINGS FOR THESE JOISTS. DESIGN CALCULATIONS ARE TO BE SUBMITTED WITH SHOP DRAWINGS. DESIGN CALCULATIONS ARE TO BE STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.
3. ALL DAMAGED JOISTS AND JOIST GIRDERS DELIVERED TO THE JOB SITE SHALL BE REJECTED OR REPAIRED BY THE JOIST FABRICATOR. REPAIR METHOD SHALL BE SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF GEORGIA AND SUBMITTED FOR APPROVAL.
4. SUSPENSION FROM ROOF STRUCTURE:
A. ATTACHMENT TO METAL DECK JOIST ENDING OR JOIST STRUTS IS PROHIBITED.
B. PIPE HANGERS SHALL BE SUPPORTED BY JOIST BOTTOM CHORDS IF THE CHORD BEARING WITH DOUBLE NUT AND WASHER IS USED AND BY JOIST TOP CHORDS UNDER DECK APPLICATION "C" CLAMP IS USED.
C. JOIST PIPES AT JOIST PANEL POINTS OR NO FURTHER THAN 3" AWAY FROM PANEL POINT WITHOUT REINFORCING JOIST PER DETAIL 2/S3.2.
C. FOR PIPES GREATER THAN 5" IN DIAMETER, NOTIFY THE ENGINEER OF THEIR LOCATION. PIPES GREATER THAN 5" IN DIAMETER RUNNING PARALLEL TO JOISTS SHALL BE LOCATED EQUALLY BETWEEN TWO JOISTS AND SUSPENDED FROM STRUTS, PROVIDED BY PIPE INSTALLER, SPANNING BETWEEN THE JOISTS. STRUTS NOT LOCATED AT JOIST PANEL POINTS SHALL REQUIRE JOIST REINFORCING PER DETAIL 2/S3.2. ONLY ONE SINGLE PIPE CAN BE SUPPORTED BY A PAIR OF JOISTS. IF PIPES ARE GREATER THAN 8" DIAMETER, NOTIFY STRUCTURAL ENGINEER.

STEEL DECK NOTES:

- 1. FLOOR DECK SHALL BE OF THE TYPE SHOWN ON THE PLANS AND SHALL BE DETAILED TO SPAN 3 SPANS MINIMUM.
2. THE DECK SHALL BE INSTALLED IN STRICT CONFORMANCE WITH THE STEEL DECK INSTITUTE REQUIREMENTS.
3. DECK UNDERSIDE SHALL BE PAINTED GRAY, TOP SHALL BE PAINTED GRAY.
4. DECK ATTACHMENT SHALL BE AS SHOWN ON THE MEZZANINE FRAMING PLAN OR AS REQUIRED BY THE STEEL DECK INSTITUTE. THE MORE SEVERE REQUIREMENT SHALL GOVERN.
5. DECK SHALL BE MANUFACTURED BY A CURRENT MEMBER OF THE STEEL DECK INSTITUTE.
6. DECK MANUFACTURED FROM ANNEALED STEEL COILS ("SOFT DECK") SHALL NOT BE PERMITTED.

CONCRETE:

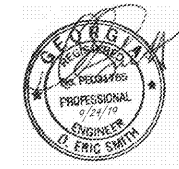
- 1. CONSTRUCTION OR CONTROL JOINTS SHALL BE PROVIDED IN SLABS ON GRADE SO THAT THE MAXIMUM AREA OF SLAB BETWEEN JOINTS SHALL BE 144 SQUARE FEET OR AS SHOWN ON THE PLANS.
2. REINFORCING BARS SHALL CONFORM TO ASTM A 615-96a. ALL REBAR SHALL BE GRADE 60. REINFORCING TO BE WELDED SHALL CONFORM TO ASTM A 706 AND ASTM A 496, GRADE 70. WELDING SHALL BE IN ACCORDANCE WITH AWS D1.4 STRUCTURAL WELDING CODE - REINFORCING STEEL BY THE AMERICAN WELDING SOCIETY FOR COMPLIANCE WITH ACI 318, SECTION 3.5.
3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-82 AND A-185. PROVIDE 2 SQUARE MINIMUM LAP AT SPLICES. WELDED WIRE FABRIC SHALL BE IN FLAT SHEETS ONLY, WELDED WIRE FABRIC IN ROLLS IS NOT PERMITTED.
4. CONCRETE MIXES SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW. MIXES SHALL CONFORM TO THE REQUIREMENTS IN THE "CONCRETE MIX SCHEDULE."
5. ALL CONTINUOUS HORIZONTAL BARS SHALL HAVE 42 BAR DIAMETER LAP SPICE WITH CORNER BARS AT ALL CORNERS AND WALL INTERSECTIONS.
6. ALL CONCRETE SHALL BE IN ACCORDANCE WITH ACI 318/318R AND ACI 301.
7. THE DESIGN OF CONCRETE ELEMENTS INCLUDING WALLS, FORMED SLAB, BEAMS, AND COLUMNS IS IN ACCORDANCE WITH ACI 318 (BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.)
8. GROUT FOR BASEPLATES SHALL BE NON-SHRINK & NON-METALLIC CONFORMING TO ASTM C827 AND SHALL HAVE A SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS OF 5000 PSI. PRE-GROUTING BASEPLATES WILL NOT BE PERMITTED.
9. SLABS WILL BE POURED TO A MINIMUM TOLERANCE OF FF = 25 (OVERALL AVERAGE) AND FF = 25 (PER LOCAL MAXIMUM) AS DETERMINED BY ASTM E 1155-87. PROVIDE TROWEL FINISH FOR TROWELED FINISH TO ALL INTERIOR SLABS.
10. COURSE AGGREGATE FOR INTERIOR SLABS ON GRADE WITH A THICKNESS OF 4" OR MORE SHALL CONFORM TO THE AASHTO #467 STONE GRADING REQUIREMENTS (1/2" TO 1/4" NOMINAL SIZE.) ALTERNATIVELY PROVIDE AT LEAST ONE COARSE AGGREGATE TO ACHIEVE A SIMILAR GRADING TO THE AASHTO #467 STONE. PROVIDE A COMPLETE SIEVE ANALYSIS INDICATING THE REMOVAL OF GAPS BETWEEN GRADES.
11. A MINIMUM OF 70% OF SAND IN ALL CONCRETE SHALL BE NATURAL SAND.
12. THE USE OF SMOOTH GRAVEL SHALL NOT BE PERMITTED IN ANY SLAB ON GRADE CONCRETE. COARSE AGGREGATE FOR ALL SLABS SHALL CONSIST OF QUARRIED ANGULAR CRUSHED STONE.
13. NO WATER SHALL BE ADDED TO CONCRETE MIXES BY THE CONTRACTOR OR SUBCONTRACTORS. IF WATER IS LEFT OUT OF THE MIX FOR TRANSPORTATION, ANY WATER ADDED BY THE CONCRETE SUPPLIER SHALL BE MEASURED AND MONITORED BY THE SPECIAL INSPECTOR BASED ON SUBMITTED REQUIREMENTS BY THE CONCRETE SUPPLIER.

STRUCTURAL STEEL:

- 1. STRUCTURAL STEEL DETAILING, FABRICATION, AND ERECTION TO BE IN ACCORDANCE WITH THE LATEST EDITION OF THE "MANUAL OF STEEL CONSTRUCTION" OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (ASD METHOD). SHOW DRAWINGS SHALL GIVE COMPLETE WELDING INFORMATION (FOR BOTH SHOP AND FIELD WELDS) USING AWS SYMBOLS.
2. ALL CONNECTIONS, EXCEPT THOSE INDICATED ON THE DRAWINGS AS WELDED CONNECTIONS, ARE TO BE MADE USING 3/4" DIAMETER ASTM A 325 BOLTS. BOLTS SHALL BE DESIGNED AS BEARING-TYPE BOLTS, EXCEPT WHERE NOTED AS "SLIP CRITICAL." BEARING CONNECTION BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH THE "SNUG TIGHT" CONDITION AS OUTLINED IN THE "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A 325 OR A 490 BOLTS." BOLTS SHALL HAVE A HARDENED WASHER PLACED UNDER THE ELEMENT TO BE TIGHTENED. BOLTS IN BRACING CONNECTIONS, MOMENT CONNECTIONS, OR OTHER CONNECTION NOTED ON THE DRAWINGS SHALL BE CONSIDERED TO BE OF CRITICAL BOLTS, AND SHALL BE DESIGNED AS FRICTION BOLTS. SLIP CRITICAL TYPE CONNECTIONS SHALL BE TIGHTENED BY ONE OF THE FOLLOWING PRETENSIONING METHODS:
A. TURN-OF-NUT WITH MATCH MARKING
B. TWIST OFF BOLT
C. DIRECT TENSION INDICATOR
D. TURN-OF-NUT WITH MATCH MARKING
E. CALIBRATED WRENCH
3. STRUCTURAL STEEL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A 992. (Fy = 50 KSI)
4. STRUCTURAL STEEL PLATES AND ANGLES SHALL CONFORM TO ASTM A 36. (Fy = 36 KSI)
5. STRUCTURAL TUBES SHALL CONFORM TO ASTM A 500 GRADE B. (Fy = 46 KSI)
6. STRUCTURAL PIPES SHALL CONFORM TO ASTM A 501. (Fy = 46 KSI)
7. ANCHOR RODS SHALL CONFORM TO ASTM F 1554, GRADE 36 (Fy = 36 KSI)
8. CONNECTIONS FOR BEAMS WHICH CANNOT CONFORM TO THE TYPICAL CONNECTION DETAILS SHALL BE IN ACCORDANCE WITH THE FOLLOWINGS:
A. WHERE BEAM REACTIONS ARE NOT SHOWN IN THE DRAWINGS, CONNECTIONS SHALL BE DETAILED FOR THE MAXIMUM UNIFORM LOAD WHICH THE BEAM WILL SUPPORT (AS A SIMPLE BEAM) FOR THE SPAN SHOWN ON THE DRAWINGS.
B. WHERE BEAM REACTIONS ARE SHOWN ON THE DRAWINGS, THE CONNECTIONS SHALL DEVELOP THE REACTIONS SHOWN. WHERE CONNECTIONS ARE SUBJECT TO ECCENTRICITY, SUCH ECCENTRICITY SHALL BE TAKEN INTO ACCOUNT WHEN DETAILING THE CONNECTION.
9. THE DESIGN OF SPECIAL CONNECTIONS BETWEEN STEEL FRAMING COMPONENTS BY OTHER THAN THE PROJECT STRUCTURAL ENGINEER OF RECORD SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF PROJECT INCLUDING, BUT NOT LIMITED TO, BRACE END CONNECTIONS, MOMENT RESISTING CONNECTIONS, MODIFIED BEAM SEAT CONNECTIONS, AND MEMBER SPLICE CONNECTIONS.
10. ALL WELDS SHALL BE PER AWS D1.1 AND ELECTRODES FOR SHOP AND FIELD WELDS SHALL CONFORM TO AWS 5.1 OR AWS A5.5, CLASS E70XX.
11. ALL HIGH STRENGTH BOLTS FOR STEEL CONNECTIONS SHALL CONFORM TO ASTM A 325 OR A 490. HIGH STRENGTH TWIST-OFF BOLTS SHALL CONFORM TO ASTM F 1852.
12. NUTS FOR STEEL CONNECTIONS SHALL CONFORM TO ASTM A 563 OR ASATM A 194.
13. WASHERS FOR STEEL CONNECTIONS SHALL CONFORM TO ASTM F 436.
14. DIRECT TENSION INDICATORS SHALL CONFORM TO ASTM F 959.
15. REMOVE SLAG FROM ALL WELDED CONNECTIONS. PAINT ALL CONNECTIONS TO MATCH PAINT COLOR.
16. STEEL ERECTOR SHALL INSTALL THE STRUCTURAL STEEL, JOISTS, AND DECK PER THE STRUCTURAL DRAWINGS AND ANY MODIFICATIONS MADE TO THE ERECTION DRAWINGS BY THE STRUCTURAL ENGINEER. ANY CONFLICTS BETWEEN DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR CLARIFICATION PRIOR TO STEEL ERECTION.

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CONSTRUCTION DRAWINGS