

GENERAL NOTES

GENERAL CONSTRUCTION NOTES

- THESE NOTES COMPLEMENT THE DRAWINGS AND SPECS AND SHOULD NOT BE CONSIDERED INCLUSIVE OF ALL ITEMS. IF A CONFLICT EXISTS BETWEEN THE DRAWING AND SPECIFICATIONS, THE MORE STRINGENT WILL APPLY.
- WHERE THE DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL LIKE OR SIMILAR CONDITIONS, EVEN THOUGH NOT SPECIFICALLY SHOWN ON THE DRAWINGS.
- CONTRACTORS SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE EXECUTING WORK.
- ALL WALLS RELYING ON TOP OF SLAB FOR SUPPORT SHALL BE BRACED UNTIL SLAB IS CURED FOR A MINIMUM OF THREE DAYS.
- DRAWINGS ARE TO BE CAREFULLY COORDINATED AND LINTELS ACCORDING TO THE STANDARD LINTEL SCHEDULE ARE TO BE PROVIDED AT ALL MASONRY OPENINGS SHOWN ON ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND STRUCTURAL PLANS.
- SHOP DRAWINGS SHALL BE INDEPENDENTLY DEVELOPED. REPRODUCTION OF STRUCTURAL DESIGN DRAWINGS FOR USE AS SHOP DRAWINGS IS PROHIBITED. STEEL FABRICATION DRAWINGS SHALL BE PREPARED BY AN ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.

APPLICABLE CODES FOR DESIGN

- INTERNATIONAL BUILDING CODE IBC 2012, WITH 2012 GA. AMENDMENTS
- AMERICAN INSTITUTE OF STEEL CONSTRUCTION AISC 14th ED
- AMERICAN CONCRETE INSTITUTE ACI 318-11
- STEEL JOIST INSTITUTE SJI 2010
- BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES ACI 530-11

CLASSIFICATION OF RISK FOR BUILDING

- RISK CATEGORY III
- SEISMIC FACTOR, I_e 1.25
- SNOW FACTOR, S 1.1
- WIND FACTOR, I_w 1.0

DESIGN LOADS

- FLOOR LIVE LOAD: CLASSROOMS 40 PSF, RESTROOMS 40 PSF, CORRIDORS 40 PSF, STAIRS & BALCONIES 100 PSF, STORAGE MECHANICAL & ELECTRICAL (LIVE LOADS HAVE BEEN REDUCED ALLOWING SECTION 1607.10.1 OF THE 2012 IBC) (PARTIAL LOADS NOT APPLIED WHERE LIVE LOADS EXCEEDS 80 PSF)
- ROOF LIVE LOAD: 20 PSF (REDUCIBLE PER SECTION 1607.12.2)
- GROUND SNOW LOAD: 5 PSF
- WIND EXPOSURE: PER SECTION 16.5.2, ASCE 7-10
- BASIC WIND SPEED: 115 MPH (3 SECOND GUST)
- WIND EXPOSURE COEFFICIENT: $C_e = 0.18, 0.18$
- WIND FROM GRADE TO 500 FT: VELOCITY PRESSURE FOR CLADDING DESIGN.
- SEISMIC DESIGN CATEGORY: C
- SEISMIC BASE SHEAR: $V = 0.073 \times W @ R=3; V = 0.0550 \times W @ R=4$
- SEISMIC FORCES DETERMINED BY: EQUIVALENT LATERAL FORCE PROCEDURE
- SEISMIC RESISTING SYSTEM: 1. BUILDING FRAME SYSTEM, INTERMEDIATE REINFORCED MASONRY SHEAR WALL (R=4, $C_e=1$); 2. STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE ($R=3, C_e=3$)

FOUNDATION

- IF AFTER EXCAVATION, THE CONDITION OF SOIL INDICATES A SAFE BEARING CAPACITY OF LESS THAN 2500 PSF, THE ARCHITECT/ENGINEER SHALL BE NOTIFIED PRIOR TO PLACEMENT OF ANY FOUNDATION.
- FOUNDATIONS AND RETAINING WALLS HAVE BEEN DESIGNED FOR ACTIVE PRESSURE OF 42 PSF AT REST PRESSURE OF 66 PSF. PASSIVE PRESSURE OF 300 PSF AND A COEFFICIENT OF FRICTION OF 0.50. THESE VALUES ARE BASED ON SOIL REPORTS AND TYPICAL VALUES FOR SOILS IN THIS AREA.
- ALL BACKFILL SHALL BE COMPACTED TO THE REQUIREMENTS OF THE EARTHWORK SPECIFICATION.

CONCRETE & REINFORCING

- CONCRETE SLABS ON GRADE, FOOTINGS, RETAINING WALLS, AND BUILDING FRAME MEMBERS TO HAVE A 28 DAY COMPRESSIVE STRENGTH OF 3000 psi WITH A MAXIMUM AGGREGATE SIZE OF 1" UNLESS NOTED OTHERWISE ON PLANS. SUSPENDED SLABS TO HAVE A 28 DAY COMPRESSIVE STRENGTH OF 3500 psi WITH A MAXIMUM AGGREGATE SIZE OF 1". ALL CONCRETE IS NORMAL WEIGHT (NW) CONCRETE EXCEPT FOR THE SECOND FLOOR SUSPENDED SLAB THAT IS TO BE LIGHTWEIGHT (LW) CONCRETE WITH A MAXIMUM UNIT DENSITY OF 110 PCF.
- CONSTRUCTION OF CONTROL JOINTS SHALL BE PROVIDED IN FLOOR SLABS ON GRADE SUCH THAT THE MAXIMUM SPAN BETWEEN JOINTS IS 12.0 R, OR AS NOTED ON THE DRAWINGS.
- PROVIDE #4 BAR x 4'-0" LONG AT THE MID-DEPTH OF SLAB AT ALL RE-ENTRANT CORNERS AND INTERSECTIONS AND AT ALL DISCONTINUOUS CONTROL JOINTS IN SLAB ON GRADE.
- ALL REINFORCING BARS SHALL BE ASTM A615, GRADE 60.
- ALL BAR SPLICES SHALL BE AT A MINIMUM THE FOLLOWING LENGTHS (UNLESS NOTED OTHERWISE ON THE DRAWINGS): 30" FOR #3 BARS, 30" FOR #5 BARS, AND 24" FOR #4 BARS.
- CORNER BARS ARE TO BE PROVIDED AT ALL CORNERS AND INTERSECTIONS OF REINFORCEMENT. EXTEND WALL FOOTING REINFORCING BARS CONTINUOUSLY THROUGH COLUMN FOOTINGS.

CONCRETE MASONRY WALL

- THE DESIGN COMPRESSIVE STRENGTH IS 1500 psi.
- GROUT FOR MASONRY FILL SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 psi @ 28 DAYS. CONCRETE WITH ASTM C1502 TYPE III PORTLAND CEMENT SHALL BE USED.
- ALL MASONRY WALLS SHALL HAVE HORIZONTAL JOINT REINFORCEMENT AT 16" O.C.
- VERTICAL JOINT REINFORCEMENT SHALL BE TYPE S.
- REINFORCEMENT SHALL BE SHOWN ON PLANS. REINFORCED WALLS MUST BE REINFORCED AS SHOWN IN DETAIL 8.1.5.
- UNLESS NOTED OTHERWISE, ALL BAR SPLICES SHALL BE AS NOTED IN DETAIL 8.1 S301.
- ANY OF THE FOLLOWING MECHANICAL SPLICES MAY BE USED:
 - STANDARD COUPLERS-A2 (ERICO PRODUCTS, INC 1-800-248-2677)
 - BRACKEN S-SERIES COUPLER (BAR-LOCK COUPLER SYSTEMS 1-800-755-4889)
 - DOUBLE BARREL ZAP-SERIES COUPLERS (ERICO PRODUCTS, INC - SOLD THROUGH REBAR SUPPLIER)
 - OTHER COUPLERS TO BE APPROVED BY THE ENGINEER OF RECORD.

STEEL

- ALL WIDE FLANGE STRUCTURAL STEEL TO HAVE A MINIMUM YIELD STRENGTH OF 50 ksi (ASTM A992), UNLESS NOTED. STRUCTURAL TUBE ARE TO HAVE A MINIMUM YIELD STRENGTH OF 46 ksi (ASTM A500). ANGLES AND CHANNELS SHALL BE ASTM A36.
- ALL WELDS ARE TO BE MADE WITH E70XX ELECTRODES.
- ALL CONNECTIONS EXCEPT THOSE INDICATED ON THE DRAWINGS AS "WELDED CONNECTIONS" ARE TO BE MADE USING 3/4" DIA. A325 BOLTS. CONNECTIONS DESIGNATED AS SLIP-CRITICAL SHALL BE MADE USING LOAD-INDICATING WASHERS OR TENSION-CONTROL BOLTS. DESIGN ALL CONNECTIONS FOR ASD (UNFACTORED) REACTIONS SHOWN ON THE PLANS.

METAL DECK

- ROOF METAL DECKING SHALL BE 1/2" x 22 GA. WIDE RIB WITH $F_y = 33.0$ ksi.
- AT THE ROOF DECK ATTACHMENT, IN LIEU OF THE 5/8" PUDDLE WELDS, THE HLT DECK FASTENING SYSTEM CAN BE USED WITH THE X-ENP-19 L15 PIN. ONE ADDITIONAL SIDELAP PER SPAN MUST BE INSTALLED WHEN THE HLT FASTENING SYSTEM IS USED. SEE PLANS FOR THE NUMBER OF SIDELAPS REQUIRED WHEN THE DECKING IS ATTACHED USING THE 5/8" PUDDLE WELD.
- FLOOR COMPOSITE METAL DECKING SHALL BE 1/2" x 22 GA. MEETING THE FOLLOWING CRITERIA: $I = 0.158 \text{ in}^4, S_x = 0.184 \text{ in}^3, S_y = 0.199 \text{ in}^3, F_y = 40.0$ ksi.
- AT THE COMPOSITE FLOOR DECK ATTACHMENT, IN LIEU OF THE 5/8" PUDDLE WELDS, THE HLT DECK FASTENING SYSTEM CAN BE USED WITH X-ENP-19 L15 PIN AS AN EQUIVALENT (NO ADDITIONAL SIDELAPS REQUIRED ATTACHMENT TO THE 5/8" PUDDLE WELD).

BAR JOISTS

- JOIST MANUFACTURER IS TO CHECK ADEQUACY OF THE JOIST DESIGN AND BRIDGING SYSTEM. MODIFY SYSTEM AS REQUIRED FOR A NET UPLIFT (0.50 + 0.6W) BASED ON THE STRENGTH LEVEL LOADS SHOWN IN THE WIND PRESSURE PLAN ON DRAWINGS 5001 COMBINED WITH A DEAD LOAD OF 5 psf (0.60 + 5 psf).
- IN STEEL FRAMING, WHERE BAR JOISTS ARE UTILIZED, AND COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STRUCTURAL STEEL MEMBERS, A BAR JOIST SHALL BE FIELD-BOLT AT COLUMNS TO PROVIDE LATERAL STABILITY DURING CONSTRUCTION.
- WHERE POINTS ARE APPLIED TO TOP OR BOTTOM CHORD OF JOISTS BETWEEN PANEL POINTS, AN L2 X 2 X 1/8 ANGLE SHALL BE WELDED BETWEEN POINT LOAD OPPOSITE CHORD PANEL POINT.

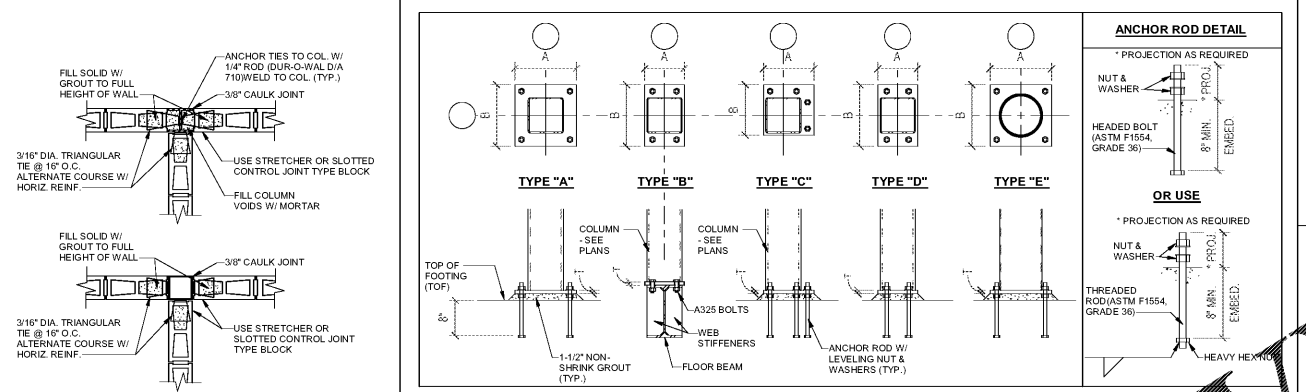
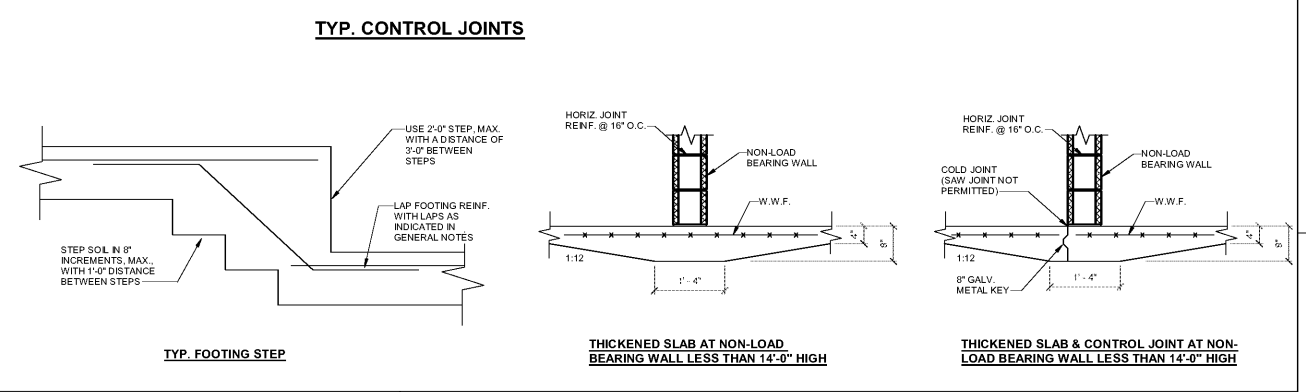
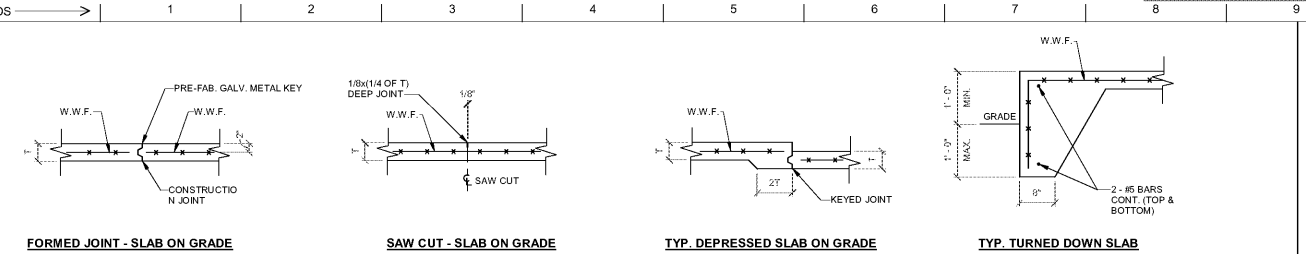
SPECIAL INSPECTIONS

- SEE SPECIFICATIONS SECTION 01 41 00 FOR REQUIREMENTS.

SA&E PROJECT NUMBER: 01-615-056
 BID PACKAGE: BP-1
 ISSUED FOR CONSTRUCTION: 10/15/2019
 Southern AE architects & engineers
 7951 Traxon Circle
 Austell, Ga 30168
 (770) 519-7777
 R # Doc # Date

FACILITY CODE NUMBER: 674-0110
 GYMNASIUM/AUDITORIUM FOR:
HEARD COUNTY SCHOOLS
 545 MAIN STREET, FRANKLIN, GA 30217
HEARD COUNTY SCHOOL SYSTEM
 FRANKLIN, GEORGIA

GENERAL NOTES & LEGENDS
 DRAWING NUMBER: S001



BASE PLATE SCHEDULE

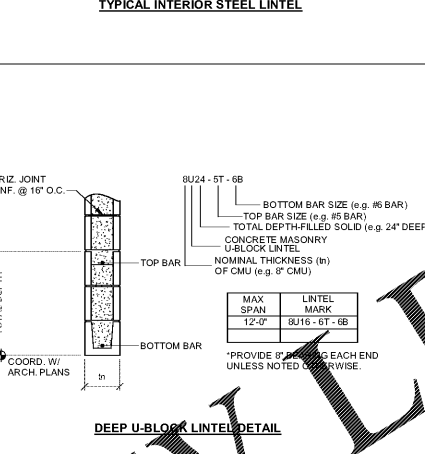
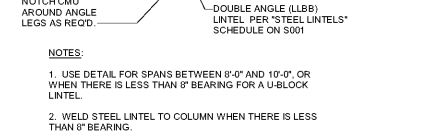
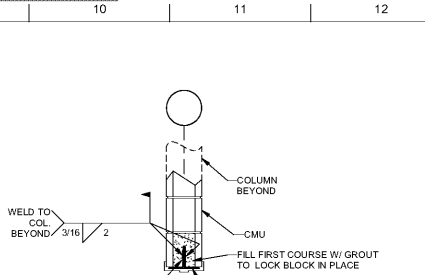
MARK	A	B	T	ANCHOR BOLT DIMETER	PLATE TYPE
BP-1	1'-2"	1'-2"	0'-0.34"	0'-0.34"	A
BP-2	1'-2"	1'-2"	0'-1"	0'-0.34"	A
BP-3	1'-2"	1'-2"	0'-1.14"	0'-0.34"	A
BP-4	1'-8"	1'-8"	0'-1"	0'-0.34"	A
BP-5	1'-2"	2'-2"	0'-1"	0'-0.34"	A
BP-6	0'-9"	0'-9"	0'-0.34"	0'-0.34"	A
BP-7	1'-10"	1'-10"	0'-1"	0'-0.34"	A
BP-8	0'-8.10"	1'-2"	0'-1"	0'-0.34"	D
BP-9	0'-11.10"	0'-11.10"	0'-1"	0'-0.34"	C
BP-10	0'-8.10"	1'-2"	0'-0.34"	0'-0.34"	B
BP-11	0'-10"	0'-10"	0'-0.34"	0'-0.34"	A

BEAM & PLATE LINTEL SCHEDULE

MARK	BEAM SIZE	PLATE SIZE	MARK	BEAM SIZE	PLATE SIZE
L-1	HSS16x6.5	3/8" x 12"	L-6	HSS12x12.5	NO PLATE
L-2	HSS20x6.5	3/8" x 12"	L-7	HSS16x6.5	NO PLATE
L-3	HSS8x14	3/8" x 12"	L-8	HSS8x14	NO PLATE
L-4	HSS8x14	NO PLATE	L-9	HSS8x14	NO PLATE
L-5	HSS12x12.5	NO PLATE	L-10	HSS12x12.5	NO PLATE

FOOTING SCHEDULE

MARK	DIMENSIONS	REINFORCEMENT
F-3	3'-0" x 3'-0" x 1'-0"	4-#4 EACH WAY, BOT
F-3.5	3'-0" x 3'-0" x 1'-0"	5-#4 EACH WAY, BOT
F-4	4'-0" x 4'-0" x 1'-0"	5-#4 EACH WAY, BOT
F-5	5'-0" x 5'-0" x 1'-0"	7-#4 EACH WAY, BOT
F-6	5'-0" x 5'-0" x 1'-2"	7-#5 EACH WAY, BOT
F-7	7'-0" x 7'-0" x 1'-0"	8-#6 EACH WAY, BOT
F-8	8'-0" x 8'-0" x 1'-0"	8-#7 EACH WAY, BOT
F-9	8'-0" x 8'-0" x 1'-0"	8-#7 EACH WAY, BOT
F-10	10'-0" x 10'-0" x 2'-0"	8-#7 EACH WAY, BOT



DEEP U-BLOCK LINTEL DETAIL

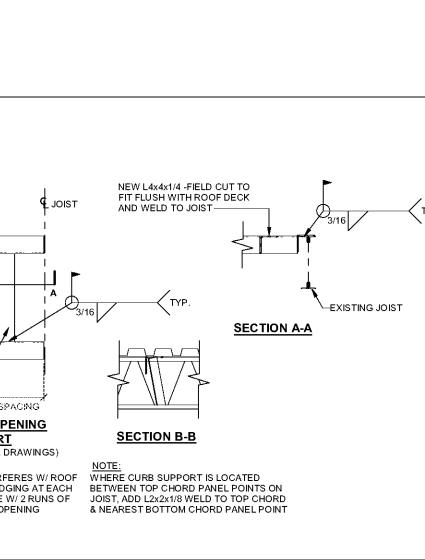
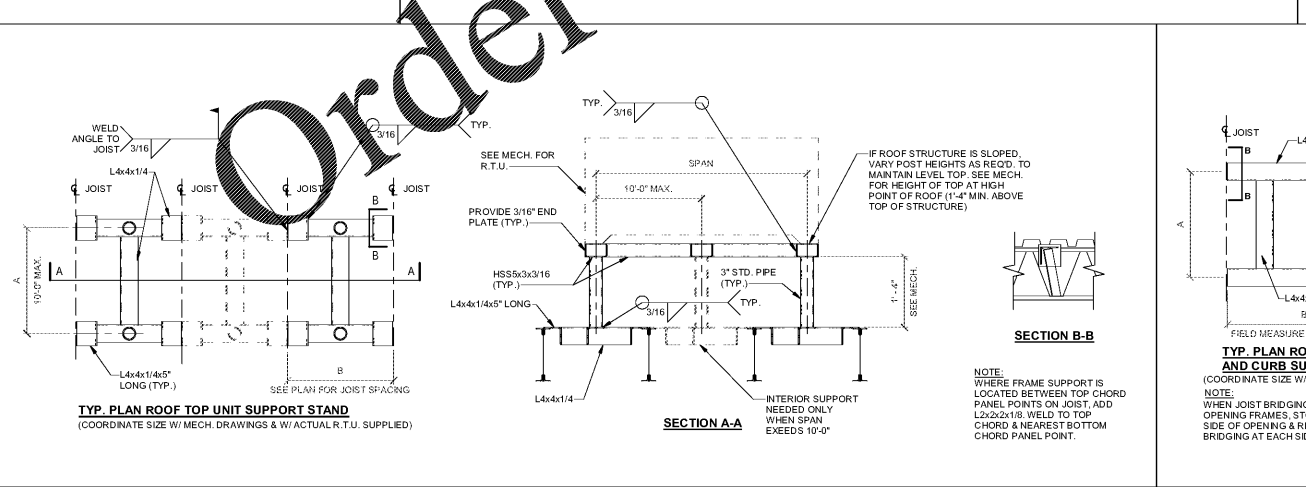
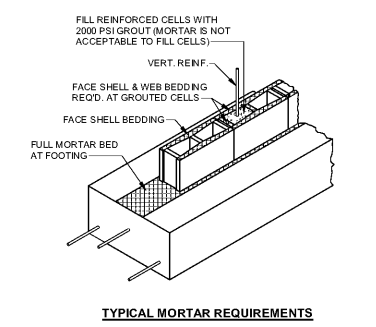
MAX. SPAN	LINTEL MARK
12'-0"	8U16-5T-6B

STEEL LINTELS (NON-LOAD BEARING WALLS)

MAX. PER 4" THICKNESS SPAN, BRCK OR BLOCK	6" LIGHT WEIGHT BLOCK	8" LIGHT WEIGHT BLOCK	12" LIGHT WEIGHT BLOCK
8'-0"	L3 12x3 1/2x14	L2 3x2 1/2x14	L1 3x2 1/2x14
6'-0"	L5x3 1/2x14	L2 3x2 1/2x14	L2 3x2 1/2x14
7'-0"	L5x3 1/2x14	L2 3x2 1/2x3/8	L2 3x2 1/2x14
8'-0"	L6x3 1/2x16	L2 3x2 1/2x3/8	L2 3x2 1/2x14
9'-0"	L6x3 1/2x16	WT 7x11	L2 3x2 1/2x16
10'-0"	L7x4x3/8	WT 7x11	L2 3x2 1/2x16

UBLOCK AND BRICK ANGLE SCHEDULE

MARK	UBLOCK	BRICK ANGLE
UB-1	UB-1	BA-1
UB-2	UB-2	BA-2
UB-3	UB-3	BA-3
UB-4	UB-4	BA-4
UB-5	UB-5	BA-5



Order Plans @