GENERAL CONDITIONS:

1. THE FOLLOWING NOTES SHALL APPLY TO ALL STRUCTURAL DRAWINGS.

2. ALL DESIGN AND CONSTRUCTION SHALL BE BASED ON AND IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 6th EDITION 2017.

3. THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS AND NOTIFY THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS, SEE THE ARCHITECTURAL DRAWINGS.

4. IF MATERIAL QUANTITIES, STRENGTHS OR SIZES INDICATED BY THE DRAWINGS OR SPECIFICATIONS ARE NOT IN AGREEMENT WITH THESE NOTES, THE BETTER QUALITY AND/OR QUANTITY, STRENGTH OR NOTED SHALL BE PROVIDED. AND/OR QUANTITY, STRENGTH OR NOTED SHALL BE PROVIDED.

5. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE ERECTIONPROCEDURE AND SEQUENCE TO ENSURE THE SAFETY OF THE STRUCTURE AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF TEMPORARY WHATEVER BRACING, GUYS, TIE-DOWNS OR SHORING MAY BE ADDITION OF TEMPORARY WHATEVER BRACING, GUYS, TIE-DOWNS OR SHORING MAY BE NECESSARY. SUCH MATERIAL SHALL BE REMOVED AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER THE COMPLETION OF THE PROJECT. THE CONTRACTOR AFTER THE COMPLETION OF THE PROJECT.

6. IT SHALL BE THE SOLE RESPONSIBLITY OF THE CONTRACTOR FOR INITIATING, MAINTAINING AND SUPERVISING ALL SAFETY PROCEDURES . THE STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PROCEDURES.

7. THE STRUCTURAL DRAWINGS ARE ONE DISCIPLINE OF THE CONTRACT DOCUMENTS AND DO NOT BY THEMSELVES CONTAIN ALL THE INFORMATION REQUIRED TO PROPERLY COMPLETE THE PROJECT STRUCTURE. THE GENERAL CONTRACTOR SHALL REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS AND COORDINATE THE INFORMATION INDICATED IN THESE CONSTRUCTION DOCUMENTS WITH THE STRUCTURAL DRAWINGS TO PROPERLY CONSTRUCT THE PROJECT.

8. ALL DETAILS, SECTIONS AND NOTES INDICATED ON THE CONSTRUCTION DOCUMENTS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.

9. SPECIALTY ENGINEERED PRODUCTS.

   A. THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SHOP DRAWINGS FOR SPECIALTY ENGINEERED PRODUCTS WHICH SHALL BE SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. INTERIOR SPECIALTY PRODUCTS SHALL BE DESIGNED FOR LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING:

      1. MISCELLANEOUS STEEL
      2. LADDERS
      3. HANDRAILS
      4. CLADDING SYSTEMS
      5. LIGHT GAGE METAL TRUSSES

10. DESIGN LOADS:

   A. DESIGN ROOF LIVE LOAD: 20 PSF
   B. DESIGN FLAT ROOF LIVE LOAD: 50 PSF
   C. DESIGN ROOF DEAD LOAD (TOP CHORD OF TRUSS): 15 PSF
   D. DESIGN ROOF DEAD LOAD (BOTTOM CHORD OF TRUSS): 10 PSF
   E. DESIGN FLAT ROOF DEAD LOAD : 15 PSF

   F. DESIGN WIND LOAD:

      1. ULTIMATE DESIGN WIND SPEED (3 SECOND GUST), V_{ULT} = 154 MPH. NOMINAL DESIGN WIND SPEED (3 SECOND GUST), V_{ASD} = 119.3 MPH.
      2. RISK CATEGORY: II
      3. WIND EXPOSURE CATEGORY: D
      4. COMPONENTS AND CLADDING WIND PRESSURE: SEE WIND LOAD SCHEDULE THIS SHEET.
      5. INTERNAL PRESSURE COEFFICIENTS +/- 0.18
      6. WIND-BORNE DEBRIS REGION (WITHIN HURRICANE-PRONE REGIONS)
         A. WITHIN 1 MILE OF THE COASTAL MEAN HIGH WATER LINE WHERE V_{ULT} IS 130 MPH OR GREATER; OR
         B. IN AREAS WHERE V_{ULT} IS 140 MPH OR GREATER.

11. SHOP DRAWINGS:

   A. THE CONTRACTOR SHALL SUBMIT, AS REQUIRED, PRINTS OF SHOP DRAWINGS FOR ALL FABRICATED MATERIALS TO ARCHITECT FOR REVIEW. REPRODUCTION OF CONTRACT DRAWINGS FOR SHOP DRAWINGS WILL NOT BE PERMITTED.

   B. REVIEW OF SHOP DRAWINGS BY THE ARCHITECT/ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF THOSE SHOP DRAWINGS.

   C. SHOP DRAWINGS REQUIRING A SPECIAL ENGINEERING DESIGN BY THE FABRICATOR SHALL BE STAMPED BY A PROFESSIONAL ENGINEER OF RECORD IN THE STATE OF THE PROJECT LOCATION BEFORE SUBMITTING FOR REVIEW BY THE ARCHITECT/ENGINEER. THESE DRAWINGS SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF OBSERVATIONS.

12. THE FOLLOWING SHOP DRAWINGS SHALL BE SUBMITTED:

   A. CONCRETE MIX DESIGNS
   B. CONCRETE REINFORCING STEEL AND WELDED WIRE FABRIC
   C. STRUCTURAL STEEL
   D. CONCRETE MASONRY UNIT SUBMITTALS AND OTHER MASONRY ACCESSORIES
   E. PRE-ENGINEERED WOOD TRUSSES

MASONRY WALL CONSTRUCTION:

1. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 2000 PSI (f'm = 1500 PSI).

2. MORTAR SHALL BE TYPE M OR S, CONFORMING TO ASTM C270.

3. COARSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAX. AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI W/ SLUMP OF 8-11 INCHES.

4. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS. FILL ALL CELLS CONTAINING REINFORCING WITH COARSE GROUT.

5. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 8'-0". REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL (TYPICAL) UNLESS OTHERWISE NOTED. SEE TYPICAL GROUTING DETAILS FOR ADDITIONAL INFORMATION.

6. REINFORCING STEEL SHALL BE LAPPED MINIMUM 48 BAR DIAMETERS.

7. HORIZONTAL WALL REINFORCING SHALL BE STANDARD LADDER TYPE DUR-O-WAL (9 GA.) HOT DIPPED GALVANIZED AT 16" O.C. (VERTICALLY) UNLESS SHOWN OTHERWISE ON THE DRAWINGS.

8. SPLICED WIRE REINFORCEMENT SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE OF EACH PIECE OF REINFORCEMENT WITHIN THE 6" LAP. USE STANDARD 'T' AND 'L' SHAPED PIECES AT INTERSECTIONS AND CORNERS.

9. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICALS. DOWELS SHALL BE ROUTED INTO A CORE IN VERTICAL ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCEMENT.

10. PROVIDE PRECAST CONCRETE LINTELS OVER ALL OPENINGS UNLESS NOTED OTHERWISE ON DRAWINGS. LINTELS SHALL BE OF SUFFICIENT SIZE AND REINFORCEMENT FOR THE GIVEN SPANS AND LOADING CONDITIONS. SUBMIT SHOP DRAWINGS WITH RATED LOAD CAPACITIES TO THE ARCHITECT FOR REVIEW.

FOUNDATION:

1. FOUNDATION DESIGN IS BASED ON AN ALLOWABLE SOIL BEARING PRESSURE OF 2,500 PSF FOR A CONVENTIONALLY REINFORCED SHALLOW FOUNDATION, AS RECOMMENDED IN THE GEOTECHNICAL REPORT PREPARED BY NUTTING ENGINEERS, ORDER NO. 315.18 DATED SEPTEMBER 2015 AND MARCH 2018.

2. THE SITE SHALL BE PREPARED UNIFORM IN ACCORDANCE WITH CIVIL DRAWINGS, SPECIFICATIONS, SOILS REPORT AND THE ALLOWABLE BEARING PRESSURE.

3. ALL EXCAVATIONS AND BUILDING PADS SHALL BE INSPECTED BY A QUALIFIED GEOTECHNICAL ENGINEER TO VERIFY THE DESIGN ASSUMPTIONS AND REPORT ADVERSE CONDITIONS.

4. WHERE FILL IS REQUIRED, IT SHALL BE PLACED IN ACCORDANCE WITH INSTRUCTIONS OF A QUALIFIED GEOTECHNICAL ENGINEER TO MAINTAIN DESIGN BEARING PRESSURE.

5. FOOTING ELEVATIONS GIVEN ARE FOR THE PURPOSE OF DESIGN. SOIL BELOW FOOTING NOT MEETING DESIGN BEARING PRESSURE SHALL BE EXCAVATED TO A DEPTH OF VERIFIABLE DESIGN PRESSURE AND BACKFILLED PER SOIL REPORT RECOMMENDATIONS TO LEVEL OF FOUNDATION BEARING. THIS SHALL BE APPROVED BY A QUALIFIED GEOTECHNICAL ENGINEER.

6. ALL EXCAVATION SHALL BE KEPT DRY. EXCAVATE TO DEPTHS AND DIMENSIONS INDICATED. TAKE EVERY PRECAUTION TO GUARD AGAINST ANY MOVEMENT OR SETTLEMENT OF ADJACENT STRUCTURES, UTILITIES, PIPING, ETC.

7. PROVIDE ANY BRACING OR SHORING NECESSARY TO AVOID SETTLEMENT OR DISPLACEMENT OF EXISTING FOUNDATION OR STRUCTURES.

8. BACKFILL AGAINST WALLS SHALL BE PLACED EVENLY EACH SIDE UNLESS SHORINGIS PROVIDED BY THE CONTRACTOR. SHORING SHALL BE THE SOLE RESPONSIBILTY OF THE CONTRACTOR AND SHALL REMAIN IN PLACE UNTIL STRUCTURAL ELEMENT BRACING THE WALL ARE IN PLACE AND HAVE REACHED FULL DESIGN STRENGTH.

CONCRETE:

1. CONCRETE MEMBERS TO HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTHS AT 28 DAYS:

   A. FOUNDATIONS 3000 PSI
   B. SLAB-ON-GRADE 3000 PSI
   C. BEAMS 4000 PSI

2. ALL CONCRETE SHALL BE READY MIX AND MEET THE FOLLOWING REQUIREMENTS:

   A. CONCRETE SLUMP SHALL BE 4 INCHES PLUS OR MINUS 1".
   B. CONCRETE SHALL HAVE 2 TO 4 PERCENT AIR ENTRAINMENT.
   C. ALL CONCRETE TO HAVE MAXIMUM WATER/CEMENT RATIO OF 0.54.

3. CONCRETE MIX SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 301 CHAPTER 3, METHOD 1 OR METHOD 3. CONTRACTOR SHALL SUBMIT BACKUP DATA PER CHAPTER 5 SECTION 5.3 OF ACI 318 LATEST EDITION.

4. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-615 GRADE 60. SUBMIT ALL REINFORCING STEEL SUBMIT ALL SHOP DRAWINGS FOR APPROVAL PRIOR TO ANY FABRICATION.

5. CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS.

6. WELDED WIRE FABRIC SHALL CONFORM WITH ASTM A 185. WELDED WIRE FABRIC SHALL BE LAPPED AT LEAST 12 INCHES UNLESS NOTED OTHERWISE.

7. LAP ALL BARS MINIMUM 48 DIAMETERS UNLESS OTHERWISE NOTED ON DRAWINGS.

8. PROVIDE ACI STANDARD HOOKS UNLESS NOTED OTHERWISE ON DRAWINGS.

9. ALL CONCRETE WORK SHALL CONFORM TO ACI 318 LATEST EDITION "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE STRUCTURES", AND ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS."

10. ALL CONCRETE DETAILS SHALL CONFORM TO ACI 315 LATEST EDITION "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" UNLESS NOTED OTHERWISE ON THE DRAWINGS.

11. CONTRACTOR SHALL REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS AND SIZES OF SLEEVES, OPENINGS, EMBEDDED ITEMS, SLAB RECESSES, SLOPES, ECT. THESE ITEMS SHALL BE COORDINATED WITH OTHER TRADES AND INSTALLED PRIOR TO CONCRETE PLACEMENT.

12. CONTRACTOR SHALL VERIFY ANCHOR BOLT SIZES AND LOCATIONS PRIOR TO CONCRETE PLACEMENT.

13. BAR LENGTHS PROVIDED ON DRAWINGS DO NOT INCLUDE HOOK LENGTH. HOOKS SHALL BE PROVIDED AT TOP BARS AT BEAM ENDS AND SLAB EDGES.

14. CONTRACTOR SHALL PROVIDE CHAIRS, BOLSTERS, SPACERS, ECT. AS REQUIRED TO SECURELY SUPPORT REINFORCEMENT. SUPPORT ITEMS ON EXPOSED CONCRETE SHALL BE PLASTIC SUPPORT REINFORCEMENT. TIPPED OR STAINLESS STEEL IN HIGHLY CORROSIVE ENVIRONMENTS, SUPPORT ITEM SHALL BE PLASTIC.

15. THE CONTRACTOR SHALL SEAL ALL HAIRLINE CRACKING IN CONCRETE SLAB ON GRADE WITH A CRACK SUPPRESSION KIT SUCH AS LATICRETE OR APPROVED EQUAL.

16. A COPY OF THE "FIELD REFERENCE MANUAL" ACI SP-15 LATEST EDITION SHALL BE KEPT BY THE CONTRACTOR ON SITE.

17. ONE COPY OF ALL THE CONCRETE TEST RESULTS SHALL BE SUBMITTED BY THE TESTING AGENCY DIRECTLY TO THE ENGINEER OF RECORD.

STRUCTURAL STEEL:

1. CONFORM TO AISC MANUAL OF STEEL CONSTRUCTION, 13TH EDITION.

MATERIALS:
   ASTM A 992, FY=50 KSI IN GENERAL.
   ASTM A 500, GRADE B, FY=46 KSI FOR STRUCTURAL TUBING.
   ASTM A 501, FY=36 KSI FOR PIPES.
   ASTM A 325, TYPE 1, FOR HIGH STRENGTH BOLTS.
   ASTM A 307, GRADE A, ANCHOR BOLTS.
   ASTM A 36, FY=36 KSI FOR PLATES, BARS, RODS, AND ANGLES.
   ASTM A 563, HEAVY HEX NUTS.
   ASTM A 436, HARDENED STEEL WASHERS.

3. THE DESIGN OF CONNECTIONS FOR ANY PORTION OF THE STRUCTURE NOT INDICATED ON THE DRAWINGS SHALL BE DESIGNED BY THE FABRICATOR AS FOLLOWS:

   A. STANDARD CONNECTIONS SHALL BE USED WHERE POSSIBLE.

   B. ALL SHOP CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED. FIELD CONNECTIONS SHALL BE HIGH STRENGTH BOLTED WHERE POSSIBLE.

   C. UNLESS NOTED, BOLTS SHALL BE 3/4" DIAMETER ASTM A-325 TYPE N BEARING CONNECTIONS. BOLTS SHALL BE "SNUG TIGHT."

   D. PROVIDE THE MINIMUM NUMBER OF BOLTS REQUIRED TO DEVELOP THE BEAM SHEAR "V" NOTED ON THE CONTRACT DRAWINGS. IF THE BEAM SHEAR IS NOT NOTED, THE CONNECTIONS SHALL DEVELOP THE BEAM SHEAR V = W/2 WHERE W IS THE TOTAL ALLOWABLE BEAM UNIFORM LOAD BASED ON LATERALLY SUPPORTED SIMPLE SPAN MOMENTS PER TABLES LOCATED IN THE AISC MANUAL OF STEEL CONSTRUCTION, 9th EDITION, BEAMS, PAGES 2-36 THRU 2-140.

4. MEMBERS SUPPORTING DECK AT THE PERIMETER OF THE BUILDING SHALL BE CONTINUOUS EXCEPT AT EXPANSION JOINT. BUTT WELD CONTINUOUS MEMBERS PLACED END TO END.

5. STRUCTURAL STEEL SHALL RECEIVE ONE SHOP COAT OF RUST INHIBITIVE PRIMER UNLESS THE STEEL IS TO RECEIVE SPRAY ON CEMENTITUOUS FIREPROOFING. SEE ARCHITECTURAL DRAWINGS. ALL EXPOSED STEEL SHALL BE HOT DIPPED GALVANIZED OR COATED WITH CORROSION INHIBITED PAINT.

6. STEEL COLUMNS AND BASE PLATES SHALL HAVE MINIMUM 3" CONCRETE COVER PROTECTION.

TERMITE PROTECTION NOTE
(IN ACCODANCE WITH FLORIDA BUILDING CODE 6th EDITITON (2017), CHAPTER 18)

TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL-APPLIED PESTICIDES, BAITING SYSTEMS AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE AS A PREVENTATIVE TREATMENT TO NEW CONSTRUCTION. SEE SECTION 202, REGISTERED TERMITICIDE. UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT, A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES."

1816.1.1
IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, THE INITIAL CHEMICAL SOIL TREATMENT INSIDE THE FOUNDATION PERIMETER SHALL BE DONE AFTER ALL EXCAVATION, BACKFILLING AND COMPACTION IS COMPLETE.

1816.1.2
IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, SOIL AREA DISTURBED AFTER INITIAL CHEMICAL SOIL TREATMENT SHALL BE RETREATED WITH A CHEMICAL SOIL TREATMENT, INCLUDING SPACES BOXED OR FORMED.

1816.1.3
IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, SPACE IN CONCRETE FLOORS BOXED OUT OR FORMED FOR THE SUBSEQUENT INSTALLATION OF PLUMBING TRAPS, DRAINS OR ANY OTHER PURPOSE SHALL BE CREATED BY USING PLASTIC OR METAL PERMANENTLY PLACED FORMS OF SUFFICIENT DEPTH TO ELIMINATE ANY PLANNED SOIL DISTURBANCE AFTER INITIAL CHEMICAL SOIL TREATMENT.

1816.1.4
IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, CHEMICALLY TREATED SOIL SHALL BE PROTECTED WITH A MINIMUM 6 MILLIMETER VAPOR RETARDER TO PROTECT AGAINST RAINFALL DILUTION. IF RAINFALL OCCURS BEFORE VAPOR RETARDER PLACEMENT, RETREATMENT IS REQUIRED. ANY WORK, INCLUDING PLACEMENT OF REINFORCING STEEL, DONE AFTER CHEMICAL TREATMENT UNTIL THE CONCRETE FLOOR IS POURED, SHALL BE DONE IN SUCH MANNER AS TO AVOID PENETRATING OR DISTURBING TREATED SOIL.

1816.1.5
IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, CONCRETE OVERPOUR OR MORTAR ACCUMULATED ALONG THE EXTERIOR FOUNDATION PERIMETER SHALL BE REMOVED PRIOR TO EXTERIOR CHEMICAL SOIL TREATMENT TO ENHANCE VERTICAL PENETRATION OF THE CHEMICALS.

1816.1.6
IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, CHEMICAL SOIL TREATMENTS SHALL ALSO BE APPLIED UNDER ALL EXTERIOR CONCRETE OR GRADE WITHIN 1 FOOT (305 MM) OF THE PRIMARY STRUCTURE SIDEWALLS. ALSO, A VERTICAL CHEMICAL BARRIER SHALL BE APPLIED PROMPTLY AFTER CONSTRUCTION IS COMPLETED, INCLUDING INITIAL LANDSCAPING AND IRRIGATION/SPRINKLER INSTALLATION. ANY SOIL DISTURBED AFTER THE CHEMICAL VERTICAL BARRIER IS APPLIED SHALL BE PROMPTLY RETREATED.

1816.1.7
IF A REGISTERED TERMITICIDE FORMULATED AND REGISTERED AS A BAIT SYSTEM IS USED FOR SUBTERRANEAN TERMITE PREVENTION, SECTIONS 1816.1.1 THROUGH 1816.1.6 DO NOT APPLY; HOWEVER, A SIGNED CONTRACT ASSURING THE INSTALLATION, MAINTENANCE AND MONITORING OF THE BAITING SYSTEM FOR A MINIMUM OF 5 YEARS FROM THE ISSUE OF THE CERTIFICATE OF OCCUPANCY SHALL BE PROVIDED TO THE BUILDING OFFICIAL PRIOR TO THE POURING OF THE SLAB, AND THE SYSTEM MUST BE INSTALLED PRIOR TO FINAL BUILDING APPROVAL. IF THE BAITING SYSTEM DIRECTIONS FOR USE REQUIRE A MONITORING PHASE PRIOR TO INSATLLATION OF THE PESTICIDE ACTIVE INGREDIENT, THE INSALLATION OF THE MONITORING PHASE COMPONENTS SHALL BE DEEMED TO CONSTITUTE INSTALLATION OF THE SYSTEM.

1816.1.8
IF A REGISTERED TERMITICIDE FORMULATED AND REGISTERED AS A WOOD TREATMENT IS USED FOR SUBTERRANEAN TERMITE PREVENTION, SECTIONS 1816.1.1 THROUGH 1816.1.6 DO NOT APPLY. APPLICATION OF THE WOOD TREATMENT TERMITICIDE SHALL BE AS REQUIRED BY LABEL DIRECTIONS FOR USE, AND MUST BE COMPLETED PRIOR TO FINAL BUILDING APPROVAL. CHANGES IN FRAMING OR ADDITIONS TO FRAMING IN AREAS OF THE STRUCTURE REQUIRING TREATMENT THAT OCCUR AFTER THE INITIAL WOOD TREATMENT MUST BE TREATED PRIOR TO FINAL BUILDING APPROVAL.

1816.2
PENETRATION. PROTECTIVE SLEEVES AROUND PIPING PENETRATING CONCRETE SLAB-ON-GRADE FLOOR SHALL NOT BE OF CELLULOSE-CONTAINING MATERIALS. IF SOIL TREATMENT IS USED FOR SUBTERRANEAN TERMITE PROTECTION, THE SLEEVE SHALL HAVE A MAXIMUM WALL THICKNESS OF 0.010 INCH (0.254 MM) AND BE SEALED WITHIN THE SLAB USING A NONCORROSIVE CLAMPING DEVICE TO ELIMINATE THE ANNULAR SPACE BETWEEN THE PIPE AND THE SLEEVE. NO TERMITICIDES SHALL BE APPLIED INSIDE THE SLEEVE.

COMPONENT AND CLADDING ROOF WIND PRESSURES (PSF) (8/12 PITCH)

ROOF ZONES \ EFFECTIVE WIND AREA	V_{ASD} = 119.3 MPH, EXC. D RISK CATEGORY II GCpi = + / - 0.18			
	10	20	50	100
1A	+41.0/-75.7	+38.9/-66.3	+36.1/-53.9	+34.0/-44.5
2A	+41.0/-93.1	+38.9/-84.7	+36.1/-73.7	+34.0/-65.3
3A	+41.0/-96.6	+38.9/-87.2	+36.1/-74.7	+34.0/-65.3

1. EDGE DISTANCE A = 5.3 FEET

COMPONENT AND CLADDING ROOF WIND PRESSURES (PSF) (9/12 PITCH)

ROOF ZONES \ EFFECTIVE WIND AREA	V_{ASD} = 119.3 MPH, EXC. D RISK CATEGORY II GCpi = + / - 0.18			
	10	20	50	100
1B	+41.0/-75.7	+38.9/-66.3	+36.1/-53.9	+34.0/-44.5
2B	+41.0/-93.1	+38.9/-84.7	+36.1/-73.7	+34.0/-65.3
3B	+41.0/-96.6	+38.9/-87.2	+36.1/-74.7	+34.0/-65.3

1. EDGE DISTANCE A = 5.3 FEET

COMPONENT AND CLADDING ROOF WIND PRESSURES (PSF) (10/12 PITCH)

ROOF ZONES \ EFFECTIVE WIND AREA	V_{ASD} = 119.3 MPH, EXC. D RISK CATEGORY II GCpi = + / - 0.18			
	10	20	50	100
1C	+41.0/-75.7	+38.9/-66.3	+36.1/-53.9	+34.0/-44.5
2C	+41.0/-93.1	+38.9/-84.7	+36.1/-73.7	+34.0/-65.3
3C	+41.0/-96.6	+38.9/-87.2	+36.1/-74.7	+34.0/-65.3

1. EDGE DISTANCE A = 5.3 FEET

NOTE:
1. ASD DESIGN METHODOLOGY.

COMPONENT AND CLADDING ROOF WIND PRESSURES (PSF) (FLAT)

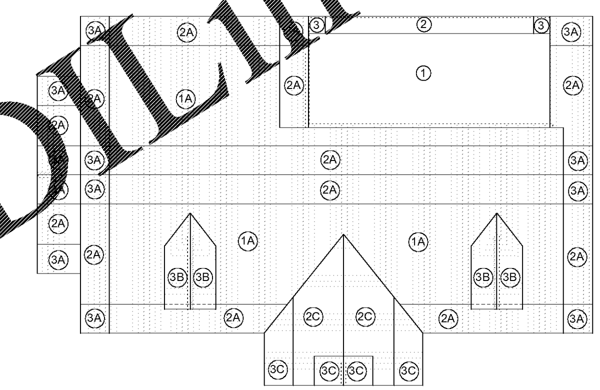
ROOF ZONES \ EFFECTIVE WIND AREA	V_{ASD} = 119.3 MPH, EXC. D RISK CATEGORY II GCpi = + / - 0.18			
	10	20	50	100
1	+15.5/-38.1	+14.5/-37.1	+13.2/-35.8	+12.3/-34.8
2	+15.5/-63.9	+14.5/-57.1	+13.2/-48.1	+12.3/-41.3
3	+15.5/-96.1	+14.5/-79.6	+13.2/-57.8	+12.3/-41.3

1. EDGE DISTANCE A = 3.0 FEET

COMPONENT AND CLADDING WALL WIND PRESSURES (PSF)

ROOF ZONES \ EFFECTIVE WIND AREA	V_{ASD} = 119.3 MPH, EXC. D RISK CATEGORY II GCpi = + / - 0.18			
	10	20	50	100
4	+34.8/-37.[illegible]	+33.3/-36.2	+31.3/-34.2	+29.7/-32.6
5	+34.8/-46.5	+33.3/-43.4	+31.3/-39.3	+29.7/-36.2

1. EDGE DISTANCE A = 5.3 FEET



3 COMPONENT AND CLADDING
NOT TO SCALE

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ISSUE BLOCK

	CLIENT REVIEW	08/30/19
	100%	09/27/19

CHECKED BY: GGB
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DESIGN CRITERIA, GENERAL AND MATERIAL NOTES

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