METAL DECK

1. THE METAL DECK WORK SHALL CONSIST OF FURNISHING EVERYTHING (LABOR, MATERIALS, ACCESSORIES, EQUIPMENT, ETC.) NECESSARY AND INCIDENTAL TO THE EXECUTION AND COMPLETION OF ALL STEEL DECK WORK AS INDICATED AND SPECIFIED ON THE DRAWINGS.
2. SUBMIT PLACEMENT AND DETAILED ("SHOP") DRAWINGS FOR REVIEW. NO STEEL DECK SHALL BE INSTALLED UNTIL THE SHOP DRAWINGS HAVE BEEN REVIEWED AND RETURNED.
3. METAL DECK SHALL CONFORM TO STEEL DECK INSTITUTE'S CURRENT STANDARDS.
4. METAL DECK SHALL BE OF THE CONFIGURATION, DEPTH, AND MINIMUM GAGE AS SHOWN ON THE DRAWINGS.
5. DO NOT HANG OR SUPPORT ANY LOADS FROM METAL ROOF DECK.
6. WHERE POSSIBLE, METAL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF 3 SPANS. TWO SPAN DECK SHALL BE USED ONLY WHERE DECK LAYOUT DOES NOT PERMIT THE USE OF THREE SPANS. SINGLE SPAN DECK IS NOT PERMITTED.
7. ROOF OPENINGS LESS THAN 6" SQUARE OR DIAMETER REQUIRE NO REINFORCEMENT. OPENINGS 6" TO 10" INCLUSIVE SHALL BE REINFORCED WITH A 20 GAUGE GALVANIZED PLATE WELDED TO THE DECK AT EACH CORNER AND 6" MAXIMUM CENTERS WITH A 5/8" DIAMETER PUDDLE WELD OR SHEET METAL SCREWS. SEE DRAWINGS FOR REINFORCEMENT OF OPENINGS LARGER THAT 10".
8. ROOF DECK SHALL BE ATTACHED AS FOLLOWS:
   A. AT SUPPORTS: 5/8" PUDDLE WELDS.
   B. AT SIDELAPS: #10 TEK SCREWS
9. ATTACHMENT SPACING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED ON DRAWINGS:
   A. INTERIOR SUPPORTS: 36/3 PATTERN (ATTACH SIDELAPS AT EACH SUPPORT)
   B. SIDELAPS: 6 PER SPAN
   C. BUILDING PERIMETER/EDGES OF DECK: 6" OC
   D. END ZONES OF BUILDING PER ROOF UPLIFT DIAGRAM (IF PROVIDED; OTHERWISE 10'-0"): 6" OC (36/7 PATTERN)
10. DECK SHALL BE POSITIONED SO THAT A COMPLETE RIB BEARS ON STEEL SUPPORT.

MASONRY

1. ALL MASONRY DESIGN AND CONSTRUCTION SHALL CONFORM TO ACI 530-13, ASCE 5-13, TMS 402-13 AND ACI 530.1-13, ASCE 6-13, TMS 602-13.
2. MASONRY SHALL BE MEDIUM WEIGHT AND HAVE A MINIMUM COMPRESSIVE STRENGTH, f'm, OF 1500 PSI BASED ON NET CROSS SECTIONAL AREA. MORTAR SHALL CONFORM TO ASTM C270 TYPE S OR M. GROUT SHALL CONFORM TO ASTM C476, WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI.
3. REINFORCING BARS SHALL CONFORM TO ASTM A 615 GRADE 60 UNLESS NOTED OTHERWISE.
4. CONTINUOUS WIRE REINFORCING (JOINT REINFORCING) SHALL BE GALVANIZED LADDER TYPE FABRICATED UNITS WITH A SINGLE PAIR OF 9 GAGE SIDE RODS AND 9 GAGE CROSS RODS FABRICATED FROM COLD DRAWN STEEL WIRE COMPLYING WITH ASTM A82. JOINT REINFORCING SHALL BE SPACED AT 16" OC VERTICALLY IN ALL MASONRY WALLS UNLESS NOTED OTHERWISE.
5. VERTICAL CONTROL JOINTS IN MASONRY WALLS ARE NOT INDICATED ON THESE DRAWINGS. HORIZONTAL BOND BEAM AND LINTEL REINFORCING SHALL BE CONTINUOUS ACROSS VERTICAL CONTROL JOINTS. HORIZONTAL JOINT REINFORCING (DUR-O-WALL) SHALL BE TERMINATED ON EITHER SIDE OF VERTICAL CONTROL JOINTS. WALLS SHORTER THAN 15'-0" IN LENGTH SHALL NOT HAVE VERTICAL CONTROL JOINTS.
   A. AT EXTERIOR WALLS, SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF VERTICAL CONTROL JOINTS. JOINTS SHALL BE PLACED AT A SPACING NOT TO EXCEED 30'-0" ON CENTER. JOINTS SHALL NOT BE LOCATED CLOSER THAN 2'-8" TO THE JAMB OF ANY EXTERIOR WALL OPENING (UNO). JOINTS SHALL NOT BE LOCATED FURTHER THAN 15'-0" FROM ANY CORNER, NOR CLOSER THAN 4'-0" FROM ANY CORNER.
   B. AT INTERIOR SHEAR WALLS, JOINTS SHALL BE PLACED AT A SPACING NOT TO EXCEED 30'-0" ON CENTER. JOINTS SHALL NOT BE LOCATED CLOSER THAN 2'-8" TO THE JAMB OF ANY SHEAR WALL OPENING (UNO). JOINTS SHALL NOT BE LOCATED FURTHER THAN 15'-0" FROM ANY CORNER, NOR CLOSER THAN 5'-0" FROM ANY CORNER.
   C. AT INTERIOR NON-SHEAR WALLS, VERTICAL CONTROL JOINTS SHALL BE PLACED AT A SPACING NOT TO EXCEED 30'-0" ON CENTER. JOINTS SHALL BE LOCATED AT WALL JAMBS, WHERE PRACTICAL, AND SHALL STEP 8" HORIZONTALLY AT MASONRY LINTEL LOCATIONS. WHERE WALLS SIT ON TOP OF A CAST SLAB-ON-GRADE, ALIGN WALL CONTROL JOINTS WITH SLAB CONTROL JOINTS. JOINTS SHALL BE LOCATED AT ALL CORNER/TEE INTERSECTIONS WHERE THE LEGS OF EACH CORNER/TEE EXCEED 15'-0" IN LENGTH.
6. CONTROL JOINTS SHALL BE LOCATED IN THE INTERIOR WALLS OF THE BUILDING AT A SPACING NOT EXCEEDING 30 FEET. JOINTS SHALL, AT A MINIMUM, BE LOCATED AT INTERSECTING WALLS.
7. MASONRY PILASTERS SHALL BE LOCATED ADJACENT TO CONTROL OR EXPANSION JOINTS.
8. ALL REINFORCED CELLS AND ALL CELLS BELOW FINISH FLOOR SHALL BE GROUTED SOLID.
9. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL BLOCK CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICAL. DOWELS MAY BE GROUTED INTO A CELL IN VERTICAL ALIGNMENT EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING.
10. REINFORCING STEEL SHALL BE SECURED IN PLACE BEFORE GROUTING STARTS.
11. VERTICAL BARS SHALL BE HELD IN POSITION WITH PRE-MANUFACTURED TIES AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 200 DIAMETERS OF THE REINFORCING NOR 10 FEET.
12. VERTICAL CELLS THAT WILL BE GROUTED SHALL HAVE A VERTICAL ALIGNMENT TO MAINTAIN A CONTINUOUS UNOBSTRUCTED CELL OF NOT LESS THAN 2 1/2" x 3".
13. GROUTING SHALL BE STOPPED 1 1/2" BELOW THE TOP OF A COURSE SO AS TO FORM A KEY AT THE POUR JOINT.
14. GROUTING SHALL BE CONSOLIDATED USING A MECHANICAL VIBRATOR PER REQUIREMENTS STATED IN ACI 530-05.
15. GROUTING OF MASONRY BEAMS OVER OPENINGS SHALL BE DONE IN ONE CONTINUOUS OPERATION.
16. ALL BOLTS INSERTED IN THE WALLS SHALL BE GROUTED SOLIDLY INTO POSITION.
17. WHERE EXPANSION BOLTS OR OTHER ANCHORS ARE EMBEDDED INTO THE SIDE OF MASONRY WALLS, THE CELLS SHALL BE FULLY GROUTED AT LEAST 8" ABOVE AND BELOW EACH BOLT OR ANCHOR.
18. WHERE NOT OTHERWISE SHOWN, MASONRY WALL FOOTINGS SHALL BE 12" THICK AND HAVE A MINIMUM OF 4" PROJECTION ON EACH SIDE OF WALL. REINFORCE WITH (2) #5 BARS CONTINUOUS TOP AND BOTTOM.
19. WALLS SHALL BE GROUTED USING LOW LIFT GROUTING TECHNIQUES.
20. ALL MASONRY WALLS SHALL BE ASSUMED TO BE RUNNING BOND, UNLESS NOTED OTHERWISE IN PLAN OR SECTION.

WOOD FRAMING

1. ALL WOOD DESIGN AND CONSTRUCTION SHALL CONFORM TO THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION (ANSI/AF&PA NDS-2015) AND RELATED SUPPLEMENTS.
2. UNLESS NOTED, USE SPRUCE PINE FIR (E=1400 KSI MIN), 19% MAX MOISTURE CONTENT, AS FOLLOWS:

BEAMS & HEADERS	No 2
LOAD BEARING STUDS	No 2
NON-LOAD BEARING STUDS (EXTERIOR)	No 2
NON-LOAD BEARING STUDS (INTERIOR)	STUD GRADE
JOISTS & PURLINS	No 2
PLATES, BLOCKING & SUB-PURLINS	No 2

3. ALL WOOD IN CONTACT WITH CONCRETE, MASONRY OR SOIL OR PERMANENTLY EXPOSED TO WEATHER SHALL BE PRESSURE TREATED.
4. AT STUD WALL OPENINGS, THE TOTAL NUMBER OF DISPLACED AND/OR CUT STUDS SHALL BE INSTALLED AND ATTACHED TO THE JAMBS, ONE-HALF OF THE TOTAL TO EACH SIDE OF THE OPENING (TOTAL NUMBER INCLUDING JACK AND KING STUDS.)
5. METAL CONNECTORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS SO THAT THE MAXIMUM PUBLISHED CAPACITY IS DEVELOPED.
6. WHERE NO CONNECTION IS INDICATED ON THE DRAWINGS, ATTACHMENT SHALL BE MADE IN ACCORDANCE TO TABLE 2304.9.1 FASTENING SCHEDULE IN THE INTERNATIONAL BUILDING CODE.
7. ALL CONNECTORS SHALL BE G90 GALVANIZED STEEL, EXCEPT CONNECTORS IN CONTACT WITH PRESSURE TREATED, FIRE-RETARDANT OR WOLMANIZED WOOD SHALL BE COATED WITH G185 ZINC COATING.
8. FURNISH BOLTS AND ANCHOR RODS WITH STANDARD NUT WASHER.
9. TOE NAILS SHALL BE DRIVEN AT A 30° ANGLE RELATIVE TO PIECE. START NAIL AT ONE-THIRD NAIL LENGTH FROM THE END OF PIECE.
10. ALL LOAD BEARING STUD WALLS (INTERIOR & EXTERIOR) SHALL HAVE CONTINUOUS HORIZONTAL BLOCKING AT 4'-6" OC (MAX) VERTICALLY PRIOR TO APPLYING ANY LOADS (INCLUDING FRAMING FOR FLOORS ABOVE).
11. WHERE (2) 2x AND (2) 2x W/ 1/2" PLYWOOD PLATE BEAMS ARE DESIGNATED, SPIKE PLATES TOGETHER WITH 12d NAILS @ 12" OC, 1" FROM TOP AND 1" FROM BOTTOM OF PLATE.
12. WHERE (3) 2x AND LARGER BEAMS ARE DESIGNATE, PLATES SHALL BE BOLTED TOGETHER WITH 1/2" DIA BOLTS @ 30" OC, 11/2" FROM TOP AND BOTTOM. BOLTS SHALL HAVE MINIMUM BENDING YIELD (Fyb) OF 45,000 PSI.
13. WHERE STUD PACK WOOD COLUMNS ARE DESIGNATED, SPIKE STUDS TOGETHER WITH 16d NAILS @ 12" OC (VERTICALLY).
14. STUD PACK OR SOLID SAWN WOOD COLUMNS SHALL BE CONTINUOUS FROM LOCATION SHOWN TO THE FOUNDATION. BLOCK FLOOR CAVITY SOLID BELOW WOOD COLUMN (WIDTH EQUAL TO WOOD COLUMN) TO ACHIEVE CONTINUITY.
15. FINGER-JOINTED LUMBER IS PERMISSIBLE AT WALL STUDS ONLY.
16. STRUCTURAL ELEMENTS SHALL NOT BE CUT TO INSTALL PLUMBING OR WIRING UNLESS METAL OR WOOD SIDE PLATES ARE PROVIDED TO STRENGTHEN THE MEMBER. PENETRATIONS IN FLOOR AND WALL SHEATHING IS PERMITTED PROVIDED THAT 2x BLOCKING IS INSTALLED AT OPENING PERIMETER (FOR OPENINGS LARGER THAN 10" IN LENGTH/DIAMETER) AND WALL FRAMING IS NOT INTERRUPTED.
17. DOUBLE TOP PLATES ((2) 2x) AT ALL WALLS SHALL BE LAPPED AT CORNERS AND INTERSECTIONS AND FASTENED IN ACCORDANCE WITH TABLE 2304.9.1 FASTENING SCHEDULE IN THE INTERNATIONAL BUILDING CODE, UNLESS NOTED OTHERWISE. OFFSET DOUBLE PLATE END JOINTS 24" (MIN).
18. WALL SHEATHING NOTED ON STRUCTURAL DRAWINGS SHALL BE ATTACHED DIRECTLY TO THE FACE OF FRAMING MEMBERS. SEE ARCHITECTURAL DRAWINGS FOR ALL NON-STRUCTURAL SHEATHING REQUIREMENTS. WHERE ARCHITECTURAL DRAWINGS REQUIRE ADDITIONAL SHEATHING, SUCH SHEATHING SHALL BE ATTACHED TO THE OUTSIDE FACE OF STRUCTURAL SHEATHING.
19. EXTERIOR WALLS WITH NO STRUCTURAL SHEATHING INDICATED ON STRUCTURAL DRAWINGS SHALL HAVE 1/2" PLYWOOD OR OSB SHEATHING, AND SHALL BE ATTACHED TO INTERMEDIATE SUPPORTS WITH 10d NAILS @ 12" OC AND AT PANEL EDGES WITH 10d NAILS @ 6" OC.
20. ANCHOR SHEAR WALLS TO ANCHOR RODS OR EPOXY ANCHORS PER STRUCTURAL DRAWINGS. ALL NON-SHEAR WALLS, EXTERIOR AND INTERIOR LOAD BEARING SHALL BE ANCHORED TO SLAB USING 1/2" DIA x 7" MIN EMBED ANCHOR BOLTS @ 48" OC (MAX). OTHER WALLS (WALLS NOT ON THICKENED SLABS OR TURNDOWNS) SHALL BE ANCHORED TO SLAB USING POWDER ACTUATED FASTENERS WITH 0.157" DIA AND EMBEDMENT OF 3/4" AT 12" OC (MAX).
21. PROVIDE ONE ROW OF BRIDGING FOR EACH 8'-0" LENGTH OF ROOF FRAMING MEMBERS.

ENGINEERED WOOD PRODUCTS

1. ALL MEMBERS SHALL BE MANUFACTURED IN ACCORDANCE WITH THE FOLLOWING PROVISIONS:
   GLUED LAMINATED TIMBER (GLULAM®): AITC/APA/TC A190.1 – FRAMING APPEARANCE GRADE
   LAMINATED VENEER LUMBER (LVL): ICC ES REPORT NO. ESR-1387
   PARALLEL STRAND LUMBER (PSL): ICC ES REPORT NO. ESR-1387
   LAMINATED STRAND LUMBER (LSL): ICC ES REPORT NO. ESR-1387
2. ALL MEMBERS SHALL CONFORM TO THE FOLLOWING MINIMUM PROPERTIES:

TYPE	SPECIES	COMB. SYMBOL	USE
GLULAM ®	DOUGLAS FIR (DF)	24F-V4	SIMPLE SPAN BEAM
	DOUGLAS FIR (DF)	24F-V4	CANTILEVER SPAN BEAM
GLULAM ®	SOUTHERN PINE (SP)	24F-V3	SIMPLE SPAN BEAM
	SOUTHERN PINE (SP)	24F-V5	CANTILEVER SPAN BEAM

TYPE	USE	GRADE	Fb	Ft	Fc⊥	Fcll	Fv
MICROLLAM LVL	BEAM	2.0E	2,600	1,555	750	2,510	285
PARALLAM PSL	BEAM	2.0E	2,900	2,025	625	2,900	290
	COLUMN	1.8E	2,400	1,755	545	2,500	190
TIMBERSTRAND LSL	BEAM	2.0E	1,700	1,075	710	1,835	425
	COLUMN	1.8E	1,700	1,075	710	1,835	425

3. MEMBERS SHALL NOT BE CUT, NOTCHED, OR DRILLED WITHOUT PRIOR APPROVAL FROM ENGINEER.
4. ALL MEMBERS TO BE GRADE MARKED FOR VISUAL INSPECTION ON SITE.
5. ALL MEMBERS SHALL BE UTILIZED IN DRY SERVICE CONDITIONS, UNLESS NOTED OTHERWISE.
6. WANE SHALL BE LIMITED IN ALL MEMBERS AS TO NOT REQUIRE ANY REDUCTION IN LOAD CAPACITY.

WOOD UPLIFT LOAD RESISTING SYSTEM

1. THESE NOTES ARE INTENDED TO SUPPLEMENT THE STRUCTURAL PLANS, DETAILS AND WALL ELEVATIONS. WHERE CONFLICT EXISTS, PLANS, DETAILS, AND WALL ELEVATIONS SHALL GOVERN. CONNECTIONS BELOW ARE MINIMUM AND DO NOT RELIEVE PRE-ENGINEERED TRUSS ENGINEER FROM CONNECTION DESIGN.
2. ROOF TRUSSES, TRUSS GIRDERS, JOISTS AND BEAM TO WALL CONNECTIONS:
   A. TYPICAL TRUSSES AND JOISTS: (1) SIMPSON H5 AT EACH END
   B. 2 SPAN TRUSSES AND JOISTS: (1) SIMPSON H5 AT EACH END AND AT INTERMEDIATE SUPPORTS (ONE EACH SIDE)
   C. BEAMS OR TRUSS GIRDERS: (1) SIMPSON MTS20 EACH END
3. TOP PLATES TO FLOOR STUDS: (1) SIMPSON H6 @ 32" OC
4. GROUND FLOOR STUDS TO MUD SILL PLATE: (2) SIMPSON H4 AT 2'-0" OC FOR NON LOAD BEARING WALLS AND 4'-0" FOR LOAD BEARING WALLS. (SEE WALL SCHEDULE).

PRE-ENGINEERED WOOD FLOOR TRUSSES

1. TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION (ANSI/AF&PA NDS - 2005) & RELATED SUPPLEMENTS.
2. WOOD TRUSSES SHALL BE FULLY DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH THE LOAD CONDITIONS AND DESIGN PARAMETERS INDICATED ON THE STRUCTURAL DRAWINGS. THE MANUFACTURER'S ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN, ADEQUACY AND SUFFICIENCY OF ALL WOOD TRUSSES.
3. MANUFACTURER SHALL SUBMIT DESIGN CALCULATIONS AND SHOP DRAWINGS BEARING SEAL AND SIGNATURE OF MANUFACTURER'S ENGINEER WITH ENGINEER'S SEAL FOR PROJECT STATE. SHOP DRAWINGS SHALL INCLUDE LAYOUT AND DETAIL NECESSARY FOR DETERMINING FIT AND PLACEMENT OF TRUSS IN THE BUILDING, INCLUDING SUPPORT ATTACHMENTS AND CONNECTIONS. CONFORM TO INTERNATIONAL BUILDING CODE SECTION 2303.4 FOR SUBMITTALS.
4. UNLESS NOTED OTHERWISE, WOOD TRUSSES SHALL BE DESIGNED AS SIMPLY SUPPORTED UNIFORMLY LOADED TRUSSES WITH THE TOP CHORD BRACED AGAINST LATERAL BUCKLING.
5. TRUSS MANUFACTURER SHALL DETERMINE AND PROVIDE ALL NECESSARY END BLOCKING AND/OR BRIDGING BETWEEN TRUSSES.
6. GENERAL CONTRACTOR SHALL COORDINATE ALL MECHANICAL, ELECTRICAL AND PLUMBING COMPONENTS WITH WOOD TRUSS MANUFACTURER. OPENINGS AND ADDITIONAL LOADS REQUIRED FOR PLACEMENT OF THESE COMPONENTS SHALL BE INCORPORATED INTO DESIGN AND COORDINATED ON SHOP DRAWINGS. NO NOTCHES, CUTS OR OTHER MODIFICATIONS MAY BE MADE TO WOOD TRUSS IN FIELD WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD AND THE MANUFACTURER'S ENGINEER.
7. MINIMUM TRUSS CHORD MEMBERS SHALL BE 2x4.
8. WOOD FLOOR TRUSSES SHALL BE DESIGNED FOR A LIVE LOAD DEFLECTION OF SPAN / 480 AND A DEAD PLUS LIVE LOAD DEFLECTION OF SPAN / 360.
9. WHERE CONCENTRATED LOADS OR OTHER NON-TYPICAL LOADS ARE INDICATED, GENERAL CONTRACTOR SHALL INSURE LOAD IS APPLIED AT TRUSS PANEL POINT, OR ACCEPTABLE DISTANCE FROM PANEL POINT TO BE DETERMINED BY TRUSS DESIGNER. WHERE THIS DISTANCE IS EXCEEDED, GENERAL CONTRACTOR SHALL REINFORCE TRUSS PER DIRECTION AND APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD AND THE MANUFACTURER'S ENGINEER.

PRE-ENGINEERED WOOD ROOF TRUSSES

1. TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION (ANSI/AF&PA NDS - 2005) & RELATED SUPPLEMENTS.
2. WOOD TRUSSES SHALL BE FULLY DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH THE LOAD CONDITIONS AND DESIGN PARAMETERS INDICATED ON THE STRUCTURAL DRAWINGS. THE MANUFACTURER'S ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN, ADEQUACY AND SUFFICIENCY OF ALL WOOD TRUSSES.
3. MANUFACTURER SHALL SUBMIT DESIGN CALCULATIONS AND SHOP DRAWINGS BEARING SEAL AND SIGNATURE OF MANUFACTURER'S ENGINEER WITH ENGINEER'S SEAL FOR PROJECT STATE. SHOP DRAWINGS SHALL INCLUDE LAYOUT AND DETAIL NECESSARY FOR DETERMINING FIT AND PLACEMENT OF TRUSS IN THE BUILDING, INCLUDING SUPPORT ATTACHMENTS AND CONNECTIONS. CONFORM TO INTERNATIONAL BUILDING CODE SECTION 2303.4 FOR SUBMITTALS.
4. UNLESS NOTED OTHERWISE, WOOD TRUSSES SHALL BE DESIGNED AS SIMPLY SUPPORTED UNIFORMLY LOADED TRUSSES WITH THE TOP CHORD BRACED AGAINST LATERAL BUCKLING.
5. TRUSS MANUFACTURER SHALL DETERMINE AND PROVIDE ALL NECESSARY END BLOCKING AND/OR BRIDGING BETWEEN TRUSSES.
6. GENERAL CONTRACTOR SHALL COORDINATE ALL MECHANICAL, ELECTRICAL AND PLUMBING COMPONENTS WITH WOOD TRUSS MANUFACTURER. OPENINGS AND ADDITIONAL LOADS REQUIRED FOR PLACEMENT OF THESE COMPONENTS SHALL BE INCORPORATED INTO DESIGN AND COORDINATED ON SHOP DRAWINGS. NO NOTCHES, CUTS OR OTHER MODIFICATIONS MAY BE MADE TO WOOD TRUSS IN FIELD WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD AND THE MANUFACTURER'S ENGINEER.
7. MINIMUM TRUSS CHORD MEMBERS SHALL BE 2x4.
8. WOOD ROOF TRUSSES SHALL BE DESIGNED FOR A LIVE LOAD DEFLECTION OF SPAN / 360 AND A DEAD PLUS LIVE LOAD DEFLECTION OF SPAN / 240.
9. TRUSSES SHALL BE DESIGNED FOR A TOP CHORD DEAD LOAD OF 15 PSF AND A ROOF LIVE LOAD AS INDICATED ON THE PLAN. TRUSSES SHALL BE DESIGNED FOR A BOTTOM CHORD DEAD LOAD OF 10 PSF AND NO LIVE LOAD.
10. WHERE CONCENTRATED LOADS OR OTHER NON-TYPICAL LOADS ARE INDICATED, GENERAL CONTRACTOR SHALL INSURE LOAD IS APPLIED AT TRUSS PANEL POINT, OR ACCEPTABLE DISTANCE FROM PANEL POINT TO BE DETERMINED BY TRUSS DESIGNER. WHERE THIS DISTANCE IS EXCEEDED, GENERAL CONTRACTOR SHALL REINFORCE TRUSS PER DIRECTION AND APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD AND THE MANUFACTURER'S ENGINEER.

OSB/PLYWOOD FLOOR DECKING

1. USE 3/4" APA - CDX TONGUE AND GROOVE OSB/PLYWOOD DECKING (RATED 48/24).
2. CONTINUOUS PANEL JOINTS ARE NOT PERMITTED. LONG SIDE OF PANEL SHALL BE PERPENDICULAR TO THE SUPPORT WITH THE SHORT SIDE OF THE PANEL ALIGNED WITH THE CENTER OF THE TWO ADJACENT PANELS.
3. ATTACHMENT OF PANEL TO WOOD FRAMING MEMBERS SHALL BE 10d NAILS AT THE FOLLOWING SPACING, UNLESS NOTED OTHERWISE:
   4" AT FLOOR PERIMETER
   6" AT PANEL EDGES
   12" AT INTERMEDIATE SUPPORTS
   GLUE DECKING TO SUPPORTING FLOOR FRAMING.
4. ATTACHMENT OF PANELS TO LIGHT GAUGE METAL FRAMING MEMBERS SHALL BE NO. 6, S-12 SCREWS AT THE FOLLOWING SPACING, UNLESS NOTED OTHERWISE:
   4" AT FLOOR PERIMETER
   6" AT PANEL EDGES
   12" AT INTERMEDIATE SUPPORTS

OSB/PLYWOOD ROOF DECKING

1. DECKING SHALL BE 5/8" APA-CDX RATED OSB/PLYWOOD SHEATHING 32/16 (EXPOSURE 1).
2. ORIENT LONG SIDE OF PANEL PERPENDICULAR TO SUPPORT. END JOINT SHALL BE ALIGNED WITH THE MIDPOINT OF THE TWO ADJACENT PANELS. NO CONTINUOUS PANEL JOINTS ARE PERMITTED. PANELS SHALL BE CONTINUOUS OVER TWO OR MORE SPANS (NO SINGLE SPAN CONDITIONS).
3. ATTACHMENT OF PANEL TO LIGHT GAUGE METAL FRAMING MEMBERS SHALL BE NO. 6, S-12 SCREWS AT THE FOLLOWING SPACINGS, UNLESS NOTED OTHERWISE:
   4" AT ROOF PERIMETER
   6" AT PANEL EDGES
   12" AT INTERMEDIATE SUPPORTS
4. ATTACHMENT OF PANEL TO WOOD FRAMING MEMBERS SHALL BE 8d NAILS AT THE FOLLOWING SPACINGS, UNLESS NOTED OTHERWISE:
   4" AT ROOF PERIMETER
   6" AT PANEL EDGES
   12" AT INTERMEDIATE SUPPORTS
5. EDGE SUPPORTS SHALL BE PROVIDED AS RECOMMENDED BY THE AMERICAN PLYWOOD ASSOCIATION (APA) BY USE OF PANEL CLIPS OR WOOD BLOCKING BETWEEN JOISTS OR TRUSSES. PANEL END JOINTS SHALL OCCUR OVER FRAMING AND SHALL BE BLOCKED AT PERIMETER OF ROOF AND AT DIRECTIONAL CHANGES.

ABBREVIATIONS

ACI	AMERICAN CONCRETE INSTITUTE
ADDL	ADDITIONAL
AFF	ABOVE FINISHED FLOOR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ALT	ALTERNATE
ARCH	ARCHITECT
ASD	ALLOWABLE STRESS DESIGN
ASTM	AMERICAN SOCIETY OF TESTING AND MATERIALS
BLDG	BUILDING
BOT	BOTTOM
BTWN	BETWEEN
C	CHANNEL
C/C	CENTER-TO-CENTER
CJ	CONTROL JOINT
CL	CENTERLINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONNX	CONNECTION
CONT	CONTINUOUS
CTR	CENTER
DBE	DECK BEARING ELEVATION
DBL	DOUBLE
DIA	DIAMETER
DL	DEAD LOAD
DWL	DOWEL
(E)	EXISTING
EA	EACH
EF	EACH FACE
EJ	EXPANSION JOINT
ELEV	ELEVATION
ENG	ENGINEER
EOR	ENGINEER OF RECORD
EOS	EDGE OF SLAB
EQ	EQUAL
EW	EACH WAY
EXP	EXPANSION
EXT	EXTERIOR
FDN	FOUNDATION
FF	FINISH FLOOR
FLR	FLOOR
FT	FEET
FTG	FOOTING
FV	FIELD VERIFY
GA	GAUGE, GAGE
GALV	GALVANIZED
GB	GRADE BEAM
GC	GENERAL CONTRACTOR
GT	GIRDER TRUSS
GYP	GYPSUM
HORIZ	HORIZONTAL
HSS	HOLLOW STRUCTURAL SECTION
IN	INCH(ES)
INT	INTERIOR
JBE	JOIST BEARING ELEVATION
K	KIPS
K-FT	KIP-FT
KLF	KIPS/FT
KSF	KIPS/SQUARE FT
KSI	KIPS/SQUARE IN
LFH	LONG FACE HORIZONTAL
LFV	LONG FACE VERTICAL
LL	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LOCN	LOCATIONS
LRFD	LOAD RESISTANCE FACTORED DESIGN
LSH	LONG SIDE HORIZONTAL
LSV	LONG SIDE VERTICAL
LW	LONG WAY
MANUF	MANUFACTURER
MAX	MAXIMUM
MECH	MECHANICAL
MEP	MECHANICAL, ELECTRICAL, & PLUMBING
MIN	MINIMUM
MISC	MISCELLANEOUS
(N)	NEW
NS	NEAR SIDE
NTS	NOT TO SCALE
OC	ON CENTER
OH	OPPOSITE HAND
PAF	POWDER ACTUATED FASTENERS
PC	PILE CAP
PLF	POUNDS/FT
PREFAB	PRE-FABRICATED
PSF	POUNDS/SQUARE FT
PSI	POUNDS/SQUARE IN
PT	POST TENSIONED
P.T.	PRESSURE TREATED
QTY	QUANTITY
REF	REFERENCE
REINF	REINFORCEMENT
REV	REVISION
RTU	ROOF TOP UNIT
SF	SQUARE FT
SIM	SIMILAR
SLH	SHORT LEG HORIZONTAL
SLV	SHORT LEG VERTICAL
SPEC	SPECIFICATION
STD	STANDARD
STL	STEEL
SW	SHORT WAY
T&G	TONGUE AND GROOVE
T/S	TEMERATURE AND SHRINKAGE
TEMP	TEMPORARY
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W/	WITH
W/O	WITHOUT
WWF	WELDED WIRE FABRIC

WWW.LDInc.com
Order Plans Only

PHILLIPS

KEYPLAN

CONSULTANT

DEL VALLE + MCNEIL, LLC
Structural Engineers
www.dvmstructural.com
404.369.0058
Project No. 19-201027
GA COA #: PEF007235

SEAL

ISSUE & REVISION RECORD

#	DATE	DESCRIPTION

This document is © Phillips Partnership, PC and is considered an Instrument of Service and is solely for use with respect to the project as indicated below. All rights reserved. This document may not be reproduced without the prior written consent of Phillips Partnership, PC.

PROJECT

Fairfield
BY MARRIOTT
INN & SUITES

DAWSONVILLE, GEORGIA

PROTOTYPE VERSION 2016
REV 7 12-21-18

4 STORY, 92 UNITS
w/ INDOOR POOL

FAIRFIELD
DAWSON FOREST ROAD
DAWSONVILLE, GA
30534

CLIENT

RIMROCK COMPANIES
10000 RIVERSIDE AVE
SUITE 32201
JACKSONVILLE, FL 32204

PHILLIPS JOB NUMBER 1972101
ISSUE DATE 08/23/2019
DRAWN BY/CHECKED BY GH/WT/TDV
DRAWING TITLE GENERAL NOTES

SHEET NUMBER

S-0.2

5901 PEACHTREE DUNWOODY RD.
BUILDING A, SUITE 450
ATLANTA, GEORGIA 30328
PHILLIPSPART.COM 770-394-1618