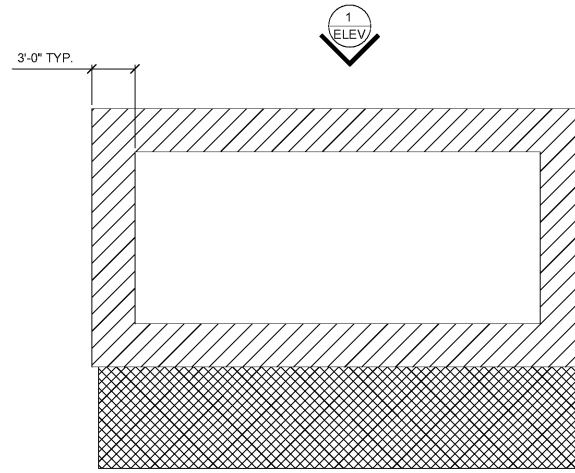


CONCRETE MASONRY

- MINIMUM 28-DAY COMPRESSIVE STRENGTH OF CONCRETE MASONRY SHALL BE F'M = 1500 PSI.
- MORTAR SHALL COMPLY WITH THE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY AND SHALL BE OF THE FOLLOWING TYPE:
 - WALLS BELOW GRADE TYPE M
 - BEARING WALLS TYPE M OR S
- CONCRETE MASONRY UNITS SHALL BE GROUTED WITH 2500 PSI COARSE GROUT AS SHOWN IN THE STRUCTURAL DOCUMENTS. GROUT FOR REINFORCED AND NONREINFORCED MASONRY SHALL CONFORM TO ASTM C476.
- PROVIDE HORIZONTAL JOINT REINFORCEMENT WITH NO. 9 GAGE LONGITUDINAL WIRES AT 16" C/C VERTICALLY, UNLESS NOTED OTHERWISE. PROVIDE SPECIAL ACCESSORIES FOR CORNERS, INTERSECTIONS, ETC.
- PROVIDE OPEN BOTTOM BEAM BLOCK UNITS WITH 3" DEEP MINIMUM WEB OPENINGS AT HORIZONTAL REINFORCEMENT LOCATIONS. A MINIMUM CLEAR SPACE OF ONE BAR DIAMETER SHALL BE PROVIDED BETWEEN THE REINFORCING BARS AND THE FACE OF MASONRY UNITS.
- PROVIDE CONTROL JOINTS IN ALL CONCRETE MASONRY WALLS AT LOCATIONS APPROVED BY THE DESIGN PROFESSIONAL AT A MAXIMUM SPACING OF 3 TIMES THE WALL HEIGHT OR 40'-0", WHICHEVER IS LESS.
- SUBMIT WRITTEN CONSTRUCTION PROCEDURES PRIOR TO THE START OF MASONRY CONSTRUCTION.
- MINIMUM VERTICAL WALL REINFORCEMENT SHALL BE #5 @ 32" C/C, UNLESS NOTED OTHERWISE.

STRUCTURAL STEEL

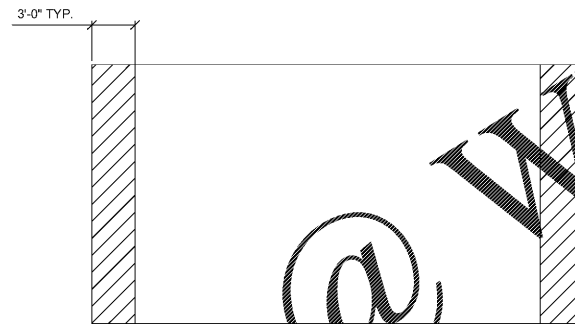
- STRUCTURAL STEEL SHALL CONFORM TO ASTM A992, UNLESS NOTED OTHERWISE.
 - STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM A500, GRADE B.
 - STRUCTURAL CHANNELS, MISCELLANEOUS PLATES AND CONNECTION MATERIAL SHALL CONFORM TO ASTM A36, UNLESS NOTED OTHERWISE.
- BOLTS AND ANCHORS:
 - BOLTED CONNECTIONS SHALL BE TYPE N (BEARING TYPE WITH THREADS INCLUDED IN SHEAR PLANE) WITH MINIMUM 3/4" DIAMETER (UNO) A325 BOLTS. SUBMIT PROPOSED BOLT TIGHTENING PROCEDURE FOR REVIEW.
 - ANCHOR BOLTS SHALL BE HEADED BOLTS CONFORMING TO ASTM F1554 AND SHALL BE HEADED RODS OR THREADED RODS WITH HEAVY HEXAGONAL NUT WELDED TO THE BOTTOM OF THE THREADED ROD, GRADE A36, UNLESS NOTED OTHERWISE.
 - EXPANSION ANCHORS SHALL HAVE BEEN EVALUATED BY THE ICC EVALUATION SERVICES, INC (ICC-ES) WITH A PUBLISHED EVALUATION REPORT. ANCHORS INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 193 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. ALL ANCHORS SHALL BE APPROVED FOR RESISTING WIND AND SEISMIC LOADS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE EQUAL TO 4.5 TIMES THE ANCHOR DIAMETER, UNLESS NOTED OTHERWISE.
 - ADHESIVE ANCHORS SHALL CONSIST OF AN ALL-THREAD STEEL ANCHOR WITH ADHESIVE CONFORMING TO ASTM C881-02, TYPE IV, GRADE 3, CLASS A, B, & C EXCEPT GEL TIMES AND EPOXY CONTENT. ADHESIVE SHALL CONSIST OF A TWO COMPONENT ADHESIVE SYSTEM CONTAINED IN SIDE BY SIDE PACKAGING CONNECTED TO A MIXING NOZZLE WHICH THOROUGHLY MIXES THE COMPONENTS AS IT IS INJECTED INTO THE HOLE. ADHESIVE SHALL HAVE PASSED ICC EVALUATION SERVICES, INC (ICC-ES) ACCEPTANCE CRITERIA 308 FOR LONG TERM CREEP. ANCHORS INSTALLED IN CONCRETE THAT MAY BECOME CRACKED UNDER SERVICE LOADS SHALL BE EVALUATED BY ICC-ES ACCEPTANCE CRITERIA 308 AND BE SPECIFICALLY APPROVED FOR USE IN CRACKED CONCRETE. CONTACT DESIGN PROFESSIONAL FOR DETERMINATION OF CRACKED OR UNCRACKED CONCRETE CONDITION UNLESS CONDITION IS NOTED ON THE DRAWINGS. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE EQUAL TO 4.5 TIMES THE ANCHOR DIAMETER, UNLESS NOTED OTHERWISE.
- STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO BOTH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
- SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. CONNECTIONS SHALL BE DETAILED BASED ON THE DESIGN INFORMATION PROVIDED IN THE CONTRACT DOCUMENTS. CONNECTIONS SHALL BE DESIGNED FOR THE SERVICE LOAD REACTION VALUES SHOWN ON THE STRUCTURAL DRAWINGS. FOR STEEL MEMBERS WHOSE REACTIONS ARE NOT SHOWN, THE DESIGN REACTION SHALL BE OBTAINED FROM THE TABLES ENTITLED "MAXIMUM TOTAL UNIFORM LOAD" IN PART 3 OF THE AISC "MANUAL OF STEEL CONSTRUCTION", FOURTEENTH (14TH) EDITION. THE DESIGN REACTION IS EQUAL TO HALF THE TABULATED VALUE FOR NONCOMPOSITE BEAMS. DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT WRITTEN PERMISSION FROM THE DESIGN PROFESSIONAL. REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH CONNECTIONS. SINGLE ANGLE CONNECTIONS ARE NOT ACCEPTABLE.
- USE PRE-QUALIFIED WELDED JOINTS IN ACCORDANCE WITH AISC AND THE STRUCTURAL WELDING CODE OF THE AMERICAN WELDING SOCIETY. "NON-PRE-QUALIFIED JOINTS" SHALL BE QUALIFIED PRIOR TO FABRICATION.
- STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE GALVANIZED.



**ROOF PLAN COMPONENTS AND CLADDING
ULTIMATE WIND PRESSURE DIAGRAM
- RESTROOM**

= -27 PSF/+26 PSF = -48 PSF/+26 PSF
 = -32 PSF/+26 PSF

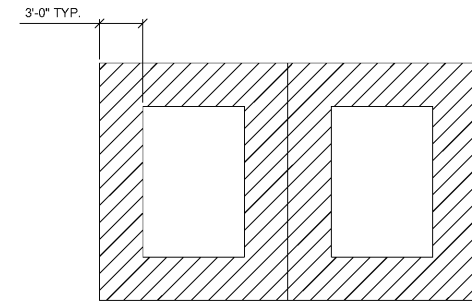
NOTE: WIND PRESSURE BASED ON 50 SQUARE FOOT AREA.
NEGATIVE INDICATES PRESSURE AWAY FROM SURFACE



**WALL ELEVATION COMPONENTS AND CLADDING
ULTIMATE WIND PRESSURE DIAGRAM
- RESTROOM**

= -30 PSF/+27 PSF = -34 PSF/+27 PSF

NOTE: WIND PRESSURE BASED ON 50 SQUARE FOOT AREA.
NEGATIVE INDICATES PRESSURE AWAY FROM SURFACE



**ROOF PLAN COMPONENTS AND CLADDING
ULTIMATE WIND PRESSURE DIAGRAM
- STAGE**

= -34 PSF/+40 PSF = -22 PSF/+40 PSF

NOTE: WIND PRESSURE BASED ON 36 SQUARE FOOT AREA.
NEGATIVE INDICATES PRESSURE AWAY FROM SURFACE

Order Plans @ www.LDILine.com

**LORD
AECK
SARGENT**

© 2018 Lord Aeck Sargent
LordAeckSargent.com
Unless otherwise agreed to in writing, this document is the sole property of Lord Aeck Sargent and is to be returned upon demand. The information herein is confidential and may not be used nor divulged without the written permission of Lord Aeck Sargent.

REVISION



GENERAL NOTES

SHEET TITLE
SCALE
AS SHOWN

JOB NAME
GWINNETT COUNTY
ELISHA WINN HOUSE
LOCATION
Dacula, Georgia 30019

ISSUE DATE
2/12/2019
JOB NO.
10958-00
DWG NO.

S002



Foresite Group, Inc. | www.fg-inc.net
3740 Davinci Court, Suite 100 | 770.368.1399
Peachtree Corners, GA 30092 | 770.368.1944