

### GENERAL CONDITIONS AND STATEMENTS

- 1. THESE NOTES SHALL APPLY UNLESS INDICATED OTHERWISE BY DRAWINGS OR SPECIFICATIONS. IN THE EVENT THAT CONFLICTS OCCUR BETWEEN THESE NOTES, DRAWINGS OR SPECIFICATIONS NOTIFY THE STRUCTURAL ENGINEER FOR RESOLUTION PRIOR TO PROCEEDING WITH THE WORK.
2. STRUCTURAL DRAWINGS INDICATE TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY. SHOP DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH THE SPECIFIED STANDARDS AND THE SPECIFIC REQUIREMENTS OF THIS PROJECT.
3. SUBMIT SHOP DRAWINGS ON ALL STRUCTURAL MATERIALS FOR APPROVAL BEFORE FABRICATION. CONTRACTOR SHALL REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMISSION.
4. THE STRUCTURE INDICATED BY THE DRAWINGS AND SPECIFICATIONS IS STRUCTURALLY STABLE ONLY IN ITS COMPLETED FORM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE MEANS, METHODS, SEQUENCES AND OPERATIONS OF CONSTRUCTION AND SHALL PROVIDE TEMPORARY BRACING AS REQUIRED TO MAINTAIN THE STABILITY OF THE STRUCTURE DURING CONSTRUCTION.
5. ALL DETAILS, SECTIONS, AND NOTES INDICATED ON THE DRAWINGS SHALL APPLY AT ALL LOCATIONS WHERE CONDITIONS ARE SIMILAR TO THOSE INDICATED BY THE DETAIL, SECTION, OR NOTE.
6. CENTERLINES OF COLUMNS AND FOUNDATIONS SHALL COINCIDE WITH GRID LINE INTERSECTIONS UNLESS NOTED OTHERWISE.
7. CENTERLINES OF FLOOR AND ROOF FRAMING MEMBERS SHALL COINCIDE WITH GRID LINES UNLESS NOTED OTHERWISE.
8. EQUALLY SPACE FLOOR AND ROOF FRAMING MEMBERS BETWEEN GRID LINES UNLESS NOTED OTHERWISE.
9. USE ONLY DIMENSIONS INDICATED ON THE DRAWINGS. DO NOT SCALE THE DRAWINGS OR USE ANY DIMENSIONS TAKEN FROM ELECTRONIC DATA FILES.
10. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE THE STRUCTURAL WORK WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS AND ALL OTHER RELEVANT TRADES. IN CASE OF CONFLICT BETWEEN STRUCTURAL WORK AND DRAWINGS RELATED TO OTHER TRADES THE CONTRACTOR SHALL MAKE IN THEIR BID ALLOWANCE FOR THE MORE SEVERE REQUIREMENTS. CONFLICTS BETWEEN THE STRUCTURAL WORK AND THE DRAWINGS OF OTHER TRADES SHALL NOT BE A REASON FOR ANY ADDITIONAL COST OR DELAY IN EXECUTION OF THE WORK.
11. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES BETWEEN THE STRUCTURAL DOCUMENTS AND ANY OTHER DOCUMENTS OR EXISTING CONDITIONS FOR RESOLUTION PRIOR TO PROCEEDING WITH THE WORK.

### ABBREVIATIONS

Table with 5 columns of abbreviations and their corresponding full names. Includes terms like ADDL (Additional), ADH (Adhesive), AESS (Architecturally Exposed), etc.

### DESIGN CRITERIA

#### DESIGN CODES

- 1. BUILDING CODE: GEORGIA BUILDING CODE BASED ON THE 2012 INTERNATIONAL BUILDING CODE
2. DESIGN LOADS: ASCE 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
3. STEEL: AISC 360-10 SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS
4. CONCRETE: ACI 318-11 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
5. COLD FORMED STEEL: AISI S100-07 NORTH AMERICAN SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS

#### DESIGN LOADS

- 1. BUILDING OCCUPANCY GROUP: II
2. SNOW LOAD: 20 PSF
3. LIVE LOAD: ROOF, 20 PSF
4. SEISMIC LOAD: le 1.0, D 0.23, Sds 0.153, Sdt 0.153, C, STEEL ORDINARY CANTILEVER COLUMNS, R=1 1/4, EQUIVALENT LATERAL FORCE
5. WIND LOAD: WIND SPEED EXPOSURE 115, Kd, Gz, Kzt, GCp1 +/- 0.18, WIND BASE SHEAR NS 1.67 KIPS, WIND BASE SHEAR EW 1.0 KIPS, COMPONENTS AND CLADDING PRESSURES IN ACCORDANCE WITH ASCE 7

### EXISTING CONDITIONS

- 1. EXISTING DRAWINGS BY HANES GIBSON & ASSOCIATES DAED 06.18.2018 WERE AVAILABLE FOR THIS PROJECT. ALL EXISTING DIMENSIONS, CONNECTION DETAILS, MEMBER SIZES, FRAMING CONFIGURATION, ETC. HAVE BEEN ASSUMED. THE GENERAL CONTRACTOR IS TO FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO STARTING CONSTRUCTION OR PREPARING SHOP DRAWINGS. NOTIFY THE ARCHITECT AND ENGINEER OF ANY AREA WHERE THE EXISTING CONDITIONS DO NOT MATCH THE CONDITIONS ASSUMED ON THESE DRAWINGS.
2. WELDABILITY OF EXISTING STEEL TO BE VERIFIED, AND APPROPRIATE ELECTRODE TO BE CHOSEN. CONTRACTOR TO COORDINATE WITH SPECIAL INSPECTOR. EXCEPT THAT LOCATIONS OF ANY SAMPLES OF EXISTING STEEL TO BE APPROVED BY ENGINEER.
3. FOR WELDING AT EXISTING STEEL, TEMPERATURE OF EXISTING STEEL NOT TO EXCEED 650° F WITHOUT ACCOUNTING FOR REDUCED STRENGTH OF SECTION. ONE THIRD OF FLANGE AREA LESS PERMITTED TO EXCEED 650° F WITHOUT SHORING. IF HIGHER TEMPERATURES PRODUCED, WELD RATE TO BE SLOWER OR SHORING INSTALLED AS REQUIRED.

### SOIL AND SUBSURFACE CONDITIONS

- 1. SOIL BEARING CAPACITY SHALL BE VERIFIED BY A GEORGIA GEOTECHNICAL ENGINEER.
2. THE FOUNDATIONS HAVE BEEN DESIGNED BASED ON THE FOLLOWING DESIGN VALUES ASSUMED: SPREAD FOOTING BEARING PRESSURE ON SOIL 3,000 PSF
3. SEE GEOTECHNICAL REPORT FOR ADDITIONAL REQUIREMENTS AND INFORMATION.
4. ALL FILL MATERIALS SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER. ALL FILL WITHIN 10'-0" OF THE BUILDING FOUNDATION PERIMETER SHALL BE COMPACTED TO 95% OF STANDARD PROCTOR. THE TOP 12" BELOW FLOOR SLABS AND FOOTINGS SHALL BE COMPACTED TO 98% OF STANDARD PROCTOR.
5. FOOTING BEARING ELEVATIONS SHALL BE ADJUSTED AT TIME OF EXCAVATION TO ACHIEVE THE REQUIRED BEARING CAPACITY IF SO REQUIRED.
6. PROVIDE POSITIVE DRAINAGE AWAY FROM BUILDING FOUNDATIONS BOTH DURING CONSTRUCTION AND PERMANENTLY.
7. MAINTAIN STABILITY OF EXCAVATIONS UNTIL PROPERLY BACKFILLED. KEEP EXCAVATIONS FREE OF LOOSE MATERIAL. DEWATER EXCAVATIONS AND REMOVE ANY WET MATERIAL PRIOR TO PLACING CONCRETE.
8. PLACE A 3" THICKNESS 'MUDMAT' OF CONCRETE IN THE BOTTOM OF FOOTINGS THAT WILL BE EXPOSED TO RAIN OR LEFT OPEN OVER NIGHT.
9. HEAVY EQUIPMENT USED FOR PLACING OR COMPACTING BACKFILL SHALL NOT BE OPERATED WITHIN A DISTANCE EQUAL TO THE HEIGHT OF THE BACKFILL ABOVE THE TOP OF FOOTING. (1 HORIZONTAL TO 1 VERTICAL). HAND OPERATED COMPACTION EQUIPMENT SHALL BE USED FOR COMPACTION OPERATIONS IN THIS AREA.
10. GRADE SHALL BE SUCH THAT THE THICKNESS OF ANY FOUNDATION OR SLAB ON GRADE IS NOT REDUCED BY MORE THAN 5% OF THAT INDICATED.
11. EXCAVATION BRACING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. EXCAVATION BRACING SHALL BE DESIGNED FOR LATERAL LOADING RESULTING FROM AN EQUIVALENT FLUID PRESSURE OF 80 PCF AND A SURFACE SURCHARGE OF 250 PSF.

### CAST IN PLACE STRUCTURAL CONCRETE

- 1. SUBMIT MIX DESIGNS FOR EACH TYPE OF CONCRETE SPECIFIED.
2. SUBMIT DATA FOR ALL ADMIXTURES, CURING COMPOUNDS AND HARDENERS THAT ARE INTENDED FOR USE.
3. CONCRETE SHALL HAVE THE MINIMUM 28 DAY COMPRESSIVE STRENGTH AND WEIGHTS:
LOCATION BAR SIZE 28 DAY STRENGTH UNIT WEIGHT
FOUNDATIONS AND SLAB ON GRADE 3,500 PSI 145 PCF
4. CONCRETE WORK SHALL CONFORM TO ACI 318.
5. REINFORCING BARS SHALL CONFORM TO ASTM A615 GRADE 60.
6. REINFORCING BARS TO BE WELDED SHALL CONFORM TO ASTM A706 GRADE 60.
7. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A92 AND A185. PROVIDE MATERIAL IN SHEETS. LAP ALL WELDED WIRE FABRIC ONE FULL SQUARE PLUS 2" AT ALL SHEET EDGES.
8. SLAB ON GRADE DOWELS SHALL BE SMOOTH RODS CONFORMING TO ASTM A36 WITH ENDS SMOOTH CUT.
9. REINFORCING BAR SUPPORT DEVICES SHALL CONFORM TO CRSI MANUAL OF STANDARD PRACTICE.
10. CONCRETE CLEAR COVER ON EMBEDDED REINFORCING SHALL BE AS FOLLOWS:
LOCATION BAR SIZE MINIMUM CLEAR COVER
FOOTINGS 3" BOTTOM AND SIDES, 2" TOP
CONCRETE EXPOSED TO EARTH OR WEATHER #5 AND SMALLER 1 1/2" #6 THROUGH #18 2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER #1 AND SMALLER 3/4"
PRIMARY REINFORCING, STIRRUPS AND TIES T/C TOP CHORD EXTENSION
11. ALL CONTINUOUS BARS SHALL HAVE A CLASS B TENSION LAP SPlice AT ALL SPLICES UNO. PROVIDE CORNER BARS AT ALL CONTINUOUS BARS AT ALL FOUNDATION AND WALL CORNERS AND INTERSECTIONS. LAP CORNER BARS 48 BAR DIAMETER EACH END.
12. PROVIDE (2) #5 x4'-0" LONG TOP DIAGONAL BARS AT ALL REINFRANT CORNERS IN ALL SLAB ON GRADE AND ELEVATED SLABS.
13. PROVIDE DOWELS TO FOOTINGS TO MATCH ALL WALL, PIER AND COLUMN VERTICAL REINFORCING. EMBED DOWELS IN FOOTING WITH HOOK TO WITHIN 3" OF BOTTOM OF FOOTING. EXTEND DOWELS ABOVE FINISH FLOOR FOR 48 BAR DIAMETER LAP SPlice WITH VERTICAL REINFORCING UNO.
14. CONSTRUCTION OR CONTRACTION JOINTS SHALL BE INSTALLED IN SLABS ON GRADE AT A SPACING NOT TO EXCEED 12'-0" OC EACH DIRECTION UNO ON FOUNDATION PLAN. ASPECT RATIO OF SLAB AREAS BETWEEN JOINTS. RATIO OF LONG SIDE TO SHORT SIDE) SHALL NOT EXCEED 1.5. SAW CUT JOINTS SHALL BE MADE AS SOON AS SLABS WILL SUPPORT MEN AND EQUIPMENT. EMBEDDED EDGE ANGLES SHALL BE DISCONTINUED AT SPLICING LOCATIONS.
15. CONSTRUCTION AND CONTRACTION JOINTS IN WALLS SHALL BE LOCATED NOT MORE THAN 10' OC MAXIMUM AND 25'-0" MAXIMUM FROM WALL CORNERS. ALIGN JOINTS IN WALLS WITH JOINTS IN SLABS. LOCATIONS OF ALL JOINTS IN WALLS ARE CONNECTED TO WALLS.
16. CONFORM TO ACI 306 FOR COLD WEATHER CONCRETE AND ACI 308 FOR HOT WEATHER CONCRETE WORK WHEN ANY COMBINATION OF TEMPERATURE, HUMIDITY OR WIND SPEED RESULTS IN CONDITIONS THAT WOULD IMPAIR THE QUALITY OF CONCRETE. CONCRETE IS TO BE REJECTED IF TEMPERATURE AT PLACEMENT IS 90 DEGREES F OR ABOVE.
17. CHAMFER ALL EXPOSED CONCRETE EDGES 3/4" UNO. SEE ARCHITECTURAL DRAWINGS FOR DETAILS.
18. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL EMBEDDED ITEMS IN CONCRETE WORK. COORDINATE WITH THE FOLLOWING: CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS, PRECAST SHOP DRAWINGS, MECHANICAL, ELECTRICAL AND PLUMBING EQUIPMENT AND FIXTURE REQUIREMENTS

### STRUCTURAL STEEL

- 1. STRUCTURAL STEEL CONSTRUCTION DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".
2. STRUCTURAL STEEL MEMBERS SHALL CONFORM TO THE FOLLOWING STANDARDS:
WIDE FLANGE SHAPES ASTM A992
ANGLE, CHANNELS AND PLATES ASTM A36
ANCHOR RODS <= 3/4"Ø PIPE ASTM A53
RECTANGULAR HSS ASTM A500 GRADE C, 50 ksi
HEADED STUDS ASTM A108, GRADE 1015-1020
3. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED.
4. SPLICING OF STRUCTURAL STEEL MEMBERS IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF THE ENGINEER FOR THE LOCATION AND TYPE OF SPLICE.
5. CAMBER BEAMS WHERE INDICATED. WHERE NO CAMBER IS INDICATED, BEAMS SHALL BE FABRICATED SO THAT AFTER ERECTION, ANY NATURAL CAMBER IS UPWARD.
6. ALL COPES, HOLES, OPENINGS AND MODIFICATIONS REQUIRED IN STRUCTURAL STEEL MEMBERS FOR ERECTION OR THE WORK OF OTHER TRADES SHALL BE INDICATED ON THE SHOP DRAWINGS AT TIME OF SUBMITTAL FOR REVIEW. FIELD MODIFICATION OF STRUCTURAL STEEL IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.
7. SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL STEEL AND SHALL INDICATE COMPLETE CONNECTION INFORMATION, BOTH SHOP AND FIELD.
8. PROVIDE A SHOP COAT OF FABRICATOR'S STANDARD RUST INHIBITIVE PRIMER TO ALL STEEL UNO.
9. SEE ARCHITECTURAL DRAWINGS FOR FIRE PROTECTIVE MATERIAL APPLIED TO STRUCTURAL STEEL. DO NOT PRIME STEEL WHICH IS TO RECEIVE SPRAY APPLIED FIRE PROTECTIVE MATERIAL. DO PRIME STEEL WHICH IS TO RECEIVE INTUMESCENT FIRE PROTECTIVE COATING.
10. FILL SOLID WITH NON-SHRINK GROUT UNDER ALL BASE AND BEARING PLATES.
11. CONNECTION NOTES:
A. STRUCTURAL STEEL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR TO SUPPORT MEMBER REACTION INDICATED. REACTIONS INDICATED ARE "SERVICE LOAD FORCES (ASD)"" OR ""FACTORED LOAD FORCES (LFRD)"" FOR APPLICABLE LOAD COMBINATIONS. WHERE NO REACTION IS INDICATED PROVIDE A CONNECTION DESIGNED TO SUPPORT A VERTICAL SHEAR REACTION OF 80% OF THE MAXIMUM TOTAL UNIFORM LOAD FOR THE APPROPRIATE BEAM. REACTION AND SPAN AS DETERMINED PER THE MAXIMUM TOTAL UNIFORM LOAD TABLES IN THE AISC MANUAL.
B. CONNECTION MATERIALS SHALL CONFORM TO THE FOLLOWING STANDARDS AND MATERIAL PROPERTIES:
ANGLES ASTM A36
WELDED JOINTS ASTM A992
PLATES ASTM A36
BOLTS ASTM A325 OR ASTM A490
NUTS ASTM A363
WASHERS ASTM A36
WELDING ELECTRODES E70XX
C. STANDARD SHEAR CONNECTIONS SHALL BE DETAILED AS DOUBLE ANGLE OR SINGLE PLATE CONNECTIONS IN ACCORDANCE WITH THE CONNECTION TYPES IN THE AISC "STEEL CONSTRUCTION MANUAL". BOLTED CONNECTIONS SHALL BE DETAILED USING HIGH STRENGTH BOLTS INSTALLED IN SUGGESTED TIGHTENED JOINTS UNO.
D. BRACED FRAME CONNECTIONS, MOMENT CONNECTIONS AND COLLECTOR ELEMENT CONNECTIONS SHALL BE DESIGNED BY A PROJECT STRUCTURAL ENGINEER. CALCULATIONS FOR THESE CONNECTIONS SHALL BE SUBMITTED AND APPROVED BY THE ENGINEER RESPONSIBLE FOR THE DESIGN.
E. PROVIDE STRENGTHENERS, CONTINUITY PLATES, DOUBLER PLATES OR OTHER ADDITIONAL MEMBER LOCAL STRENGTHENING MEASURES REQUIRED FOR THE CONNECTION DESIGN.
F. BOLTED CONNECTIONS SHALL BE MADE WITH A MINIMUM OF (2) 3/4"Ø BOLTS AND HAVE A MINIMUM SHEAR CAPACITY OF 10 KIPS.
G. BOLTED CONNECTIONS SHALL CONFORM TO THE PROVISIONS OF THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS.
H. WELDED CONNECTIONS SHALL BE MADE WITH CONTINUOUS FILLET WELDS UNO. MINIMUM WELD SIZE SHALL BE 1/4" OR AS REQUIRED BY THE DESIGN, WHICHEVER IS LARGER. MINIMUM WELD LENGTH SHALL BE 2".
I. ALL WELDS SHALL BE MADE BY CERTIFIED WELDERS.
J. BOLTED CONNECTIONS OF MOMENT CONNECTIONS, TENSION CONNECTIONS, BRACED FRAME CONNECTIONS, LIMITED FRAME CONNECTIONS, COLLECTOR ELEMENT CONNECTIONS AND AS INDICATED SHALL BE SLIP-CRITICAL.
13. WHEN NOT SPECIFIED, PROVIDE LOOSE LINTEL PER SCHEDULE.
A. USE SCHEDULE AT OPENINGS IN MASONRY WALLS AND MASONRY VENEER WHERE LINTELS ARE NOT INDICATED ON PLANS.
B. PROVIDE 8" MINIMUM BEARING AT EACH END FOR STEEL ANGLES.
C. ALL LINTELS TO BE HOT DIPPED GALVANIZED.

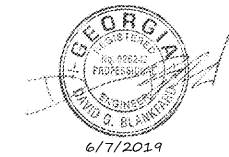
### LIGHT GAGE METAL FRAMING

- 1. LIGHT GAGE METAL FRAMING INDICATED ON THE DRAWINGS INDICATES TYPICAL CONDITIONS AND MINIMUM REQUIREMENTS.
2. LIGHT GAGE METAL FRAMING SHALL BE DESIGNED BY A PROJECT STATE STRUCTURAL ENGINEER. DESIGN CALCULATIONS AND SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER OF RECORD FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE LAYOUT OF ALL LIGHT GAGE METAL FRAMING INCLUDING ARRANGEMENT, DIMENSIONS, MATERIALS, STRESS VALUES, CONNECTORS, ANCHORAGE, AND RELATION TO ADJACENT WORK.
3. LIGHT GAGE METAL FRAMING DESIGN AND CONSTRUCTION SHALL CONFORM TO THE AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.
4. MINIMUM GAGE OF MEMBERS PROVIDING LATERAL SUPPORT FOR MASONRY VENEER SHALL BE 18 GAGE (43 MILS).
5. LIMIT LATERAL DEFLECTION OF STUDS PROVIDING LATERAL SUPPORT FOR MASONRY VENEER TO H600.
6. MINIMUM YIELD STRENGTH (Fy) FOR LIGHT GAGE METAL FRAMING MEMBERS SHALL BE 33,000 PSI FOR 18 GAGE (43 MILS) AND THINNER. MINIMUM YIELD STRENGTH (Fy) FOR MEMBERS SHALL BE 50,000 PSI FOR 16 GAGE (54 MILS) AND THICKER.
7. ALL LIGHT GAGE METAL STUDS, TRUSSES, TRACK, BRIDGING AND ACCESSORIES SHALL BE FORMED FROM STEEL HAVING A G-60 GALVANIZED COATING CONFORMING TO ASTM A653 AND C955.
8. A MINIMUM OF 10" LENGTH OF UN-PUNCHED STEEL IS REQUIRED AT ENDS OF STUDS AND AT ALL BEARING POINTS AND CONCENTRATED LOADS (NO PUNCHING HOLES OF ANY SIZE IS PERMITTED IN THESE 10 INCHES); NO CUTTING OF THE STUD FLANGES IS PERMITTED.
9. SPLICES IN LOAD BEARING STUDS ARE NOT PERMITTED.
10. LOAD BEARING STUDS SHALL HAVE FULL BEARING AGAINST THE INSIDE TRACK WEB TOP AND BOTTOM. STUD ENDS SHALL BE CUT SQUARE.
11. LATERAL BRIDGING SHALL BE USED TO PROVIDE LATERAL STABILITY OF LOAD BEARING STUDS. BRIDGING SHALL BE (2) 1 1/2" - 18 GA (43 MILS) FLAT STRAP (ONE EACH SIDE OF WALL). FASTEN BRIDGING TO EACH STUD WITH (1) #10 SCREW. PROVIDE TRACK BLOCKING BETWEEN STUDS IN LINE WITH BRIDGING SPACED AT 10'-0" MAXIMUM ALONG LENGTH OF ALL BRIDGING LINES AND EACH SIDE OF WALL OPENINGS.
12. BRIDGING IS TO BE SPACED AT 4'-0" OC VERTICALLY.
13. MINIMUM TRACK FASTENING AT FOUNDATION SHALL BE 0.177Ø POWDER ACTUATED FASTENERS (PAF) SPACED AT 8" OC WITH 1 1/2" MINIMUM PENETRATION INTO CONCRETE.
14. CUTTING OF LOAD BEARING METAL STUDS, TRACK, BRIDGING OR BRACING IS NOT PERMITTED WITHOUT SPECIFIC APPROVAL FROM THE ENGINEER OF RECORD.
15. ATTACH ALL EXTERIOR SHEATHING AND INTERIOR SHEATHING AT BEARING WALLS TO METAL STUDS WITH #6 SCREWS SPACED AT 6" OC AT ALL PANEL EDGES AND PANEL INTERIOR. REFER TO ARCHITECTURAL DRAWINGS FOR NON-LOAD BEARING WALLS AND ALL WALL DIMENSIONS.

### METAL ROOF DECK

- 1. THE DESIGN, MANUFACTURE AND ERECTION OF STEEL ROOF DECK AND ITS ANCHORAGE SHALL BE IN ACCORDANCE WITH THE ANSIS/DI "STANDARD FOR STEEL ROOF DECK".
2. PROVIDE ROOF DECK OF TYPE, DEPTH AND MINIMUM THICKNESS INDICATED.
3. ROOF DECK SHALL BE INSTALLED IN LENGTHS TO PROVIDE 3 CONTINUOUS SPANS MINIMUM.
4. INSTALL ROOF DECK WITH A MINIMUM END BEARING LENGTH OF 1'-1 1/2".
5. ROOF DECK SHALL BE FASTENED TO SUPPORTS AS INDICATED ON THE DRAWINGS. FASTEN TO SUPPORTS AT DECK PERIMETER WITH A MINIMUM OF 5/8" DIAMETER WELDS SPACED AT 6" OC.

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CONTENT: GENERAL NOTES

S001

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