

1.0 BUILDING CODES USED FOR DESIGN
DESIGN IS IN COMPLIANCE WITH 2012 INTERNATIONAL BUILDING CODE

1.1 DESIGN LOADS

- A. ROOF LIVE LOAD = 20 PSF
B. ROOF DEAD LOAD = 20 PSF

GROUND SNOW LOAD = 5 PSF, SHAPE COEFFICIENT $C_e = 0.7$,
($W_p = W_g \times C_e$), $C_d = 0.8$

ULTIMATE WIND SPEED = 117 MPH (3 SEC GUST)
BASIS WIND SPEED = 96 MPH (3 SEC GUST)
WIND IMPORTANCE FACTOR, $I_w = 1.00$
WIND EXPOSURE CATEGORY = B
OCCUPANCY CATEGORY = 2
INTERNAL PRESSURE COEFFICIENTS = +/- 0.18

WINDWARD PRESSURE COEFFICIENT = 0.8
LEeward PRESSURE COEFFICIENT = 0.5

COMPONENTS AND CLADDING NOT DESIGNED BY THE ENGINEER OF RECORD SHALL BE DESIGNED USING MIN. PRESSURES IN ACCORDANCE WITH ASCE 7.

SEISMIC OCCUPANCY CATEGORY = 2
SEISMIC IMPORTANCE FACTOR = 1
SEISMIC SITE CLASS = 3
DESIGN SPECTRAL RESPONSE ACCELERATION COEFFICIENT $S_{ds} = 0.30$
DESIGN SPECTRAL RESPONSE ACCELERATION COEFFICIENT $S_{d1} = 0.18$
SEISMIC DESIGN CATEGORY = C

1.2 NEW CONSTRUCTION

- THE CONTRACTOR SHALL FOLLOW WRITTEN DIMENSIONS ONLY. DO NOT SCALE DRAWINGS.
- ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSIDERED TO APPLY AT ANY SIMILAR CONDITION ELSEWHERE ON THE PROJECT, EXCEPT WHERE A DIFFERENT DETAIL OR SECTION IS SHOWN.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. WHERE DISCREPANCIES OCCUR, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ARCHITECT OR ENGINEER PRIOR TO CONSTRUCTION.
- THE STEEL FRAMING MEMBERS SHOWN RELY ON BUILDING COMPONENTS OTHER THAN STRUCTURAL STEEL FOR FINAL STRUCTURAL STABILITY (PREVIOUSLY REFERRED TO AS A NON-Self-SUPPORTING STEEL FRAME BY THE AISC CODE OF STANDARD PRACTICE). THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND PROVISION OF ANY AND ALL TEMPORARY BRACING AND SHORING AGAINST WIND, ERECTION AND ALL CONSTRUCTION LOADS UNTIL ALL ELEMENTS, MEMBERS AND CONNECTIONS (FLOORS, ROOFS, SHEAR WALLS, ETC.), AS SHOWN ON THE CONTRACT DOCUMENTS ARE COMPLETELY INSTALLED. THE STRUCTURAL MEMBERS SHOWN ON THE CONTRACT DOCUMENTS ARE DESIGNED FOR THE ANTICIPATED LOADS THAT THE STRUCTURE WILL BE SUBJECTED TO ONLY AFTER ALL STRUCTURAL ELEMENTS ARE IN PLACE AND FINAL CONNECTIONS ARE COMPLETE.
- THE GENERAL CONTRACTOR SHALL VERIFY ALL OPENING SIZES, PAD SIZES AND LOCATIONS WITH THE RESPECTIVE SUB-CONTRACTORS.

1.3 NEW WORK IN CONNECTION WITH EXISTING CONSTRUCTION

- THE CONTRACTOR SHALL VERIFY BY FIELD CHECK ALL SIZES, DIMENSIONS, ELEVATIONS, LOCATIONS, ETC. OF ELEMENTS OF THE EXISTING CONSTRUCTION WHICH ARE RELATIVE TO THE NEW CONSTRUCTION.
- ALL DIMENSIONS INVOLVING NEW WORK TYING INTO OR GOVERNED BY EXISTING CONSTRUCTION SHALL BE FIELD CHECKED BY THE CONTRACTOR AND FURNISHED TO THE SUB-CONTRACTOR PRIOR TO FABRICATION OF ANY WORK. THE VERIFIED DIMENSIONS SHALL APPEAR AND BE NOTED AS SUCH ON THE FIRST SHOP DRAWING SUBMITTED.
- THE ENGINEER HAS MADE ASSUMPTIONS CONCERNING THE SOUNDNESS OF THE EXISTING BUILDINGS AND THESE ASSUMPTIONS ARE THAT THIS BUILDING WAS DESIGNED AND CONSTRUCTED IN CONFORMANCE WITH PROPER DESIGN AND CONSTRUCTION PRACTICES. THE CONTRACTOR SHALL TAKE EXTRAORDINARY PRECAUTIONS CONCERNING PRESERVATION OF THE BUILDING DURING DEMOLITION AND NEW CONSTRUCTION WORK. FURTHER, HE SHALL AGREE TO ASSUME ALL RESPONSIBILITY FOR THE PRESERVATION OF THIS PROPERTY.
- THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER IMMEDIATELY OF ANY DISCREPANCIES BETWEEN CONSTRUCTION DOCUMENTS AND ACTUAL FIELD CONDITIONS.
- ALL HOLES THROUGH EXISTING CONSTRUCTION SHALL BE CORE DRILLED OR SAW CUT.

1.4 MISCELLANEOUS

- ALL ANCHOR BOLTS FOR MECHANICAL AND ELECTRICAL EQUIPMENT ARE FURNISHED AND LOCATED BY THE RESPECTIVE CONTRACTORS AND SET BY THE GENERAL CONTRACTOR EXCEPT WHERE OTHER CONTRACTORS FURNISH THEIR OWN CONCRETE PADS.
- ALL PIPE SLEEVES ARE FURNISHED BY AND LOCATED BY THE MECHANICAL AND ELECTRICAL CONTRACTORS AND SET BY THE GENERAL CONTRACTOR.
- THE GENERAL CONTRACTOR SHALL VERIFY ALL OPENING SIZES, PAD SIZES AND LOCATIONS WITH THE RESPECTIVE SUB-CONTRACTORS.
- ALL CORE DRILLING SHALL BE DONE BY THE MECHANICAL AND ELECTRICAL CONTRACTORS FOR THEIR OWN WORK UNDER THE SUPERVISION OF THE GENERAL CONTRACTOR. NO REINFORCING SHALL BE CUT, WRENCHED, LOCATED OR REINFORCED BEFORE CORE DRILLING. THERE SHALL NOT BE ANY CORE DRILLING THROUGH BEAMS OR COLUMNS. MAXIMUM CORE HOLE THROUGH SLABS SHALL BE PIPE DIAMETER PLUS 1".

1.5 SHOP DRAWINGS

- SHOP DRAWINGS, UNLESS OTHERWISE NOTED, SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.
- BEFORE SUBMITTAL, THE CONTRACTOR SHALL REVIEW THE SHOP DRAWINGS AND INDICATE ANY CORRECTIONS REQUIRED. THE CONTRACTOR SHALL STAMP AND SIGN THE DRAWINGS INDICATING THEY HAVE BEEN REVIEWED.
- SHOP DRAWINGS SHALL BE FURNISHED FOR ALL STRUCTURAL COMPONENTS. ALL SUBMITTALS TO BE MINIMUM THREE (3) SETS OF PRINTS.
- REVIEW OF SHOP DRAWINGS BY ENGINEER DOES NOT RELIEVE CONTRACTOR FROM COMPLIANCE WITH REQUIREMENTS OF THE DRAWINGS AND SPECIFICATIONS. REVIEW IS FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT OF THE PROJECT AND GENERAL COMPLIANCE WITH THE INFORMATION GIVEN IN THE CONTRACT DOCUMENTS. THE CONTRACTOR IS RESPONSIBLE FOR CONFIRMING AND CORRELATING ALL QUANTITIES AND DIMENSIONS SELECTING FABRICATION PROCESSES AND TECHNIQUES OF CONSTRUCTION, COORDINATING HIS WORK WITH THAT OF OTHER TRADES, AND PERFORMING HIS WORK IN A SAFE AND SATISFACTORY MANNER.

2.0 EXCAVATION AND EARTHWORK

- THE SOILS AND FOUNDATION ENGINEERING REPORT IS FOR INFORMATIONAL PURPOSES ONLY AND SHALL NOT BE CONSIDERED PART OF THE CONTRACT DOCUMENTS.
- THE FOUNDATION DESIGN IS BASED ON THE FOLLOWING NET ALLOWABLE BEARING PRESSURES:
 - SPREAD FOOTINGS = 2,000 PSF
 - WALL FOOTINGS = 2,000 PSF
 - ALL FOOTING EXCAVATIONS SHALL BE INSPECTED, PRIOR TO CONCRETE PLACEMENT, BY A SOILS ENGINEER TO VERIFY SUITABLE BEARING MATERIAL OF CAPACITY AS SPECIFIED.
- NOTIFY THE OWNER'S REPRESENTATIVE WHEN ADDITIONAL EXCAVATION IS REQUIRED TO REACH SUITABLE BEARING MATERIAL.
- THE SOILS ENGINEER SHALL CERTIFY THAT ALL FOUNDATIONS WERE PLACED ON MATERIAL WITH THE BEARING VALUE AS SPECIFIED.
- WITHIN THE EXCAVATION AREA OF THE FOUNDATIONS, ALL VEGETATION, TOPSOIL, PREVIOUSLY PLACED FILL AND UNSUITABLE SOILS SHALL BE REMOVED. ALL FOOTINGS TO BEAR ON VIRGIN SOILS OR PROPERLY PLACED AND COMPACTED ENGINEERED FILL.

2.1 LVA

- 2.2 FOUNDATION / UNDERGROUND MECHANICAL COORDINATION**
 - UNDERGROUND SEWER, WATER, GAS LINES, ETC. CROSSING CONTINUOUS WALL FOUNDATIONS SHALL NOT PASS THROUGH FOOTINGS. WHERE PIPE OCCURS ABOVE TOP OF FOOTING, SLEWE THROUGH WALL. WHERE PIPE OCCURS IN FOOTING DEPTH, DROP TOP OF FOOTING SUCH THAT PIPE PASSES JUST ABOVE FOOTING. IF TOP OF PIPE IS LESS THAN 6" BELOW BOTTOM OF FOOTING, PROVIDE IF COMPRESSIBLE FOAM INSULATION BELOW FOOTING FOR WIDTH OF TRENCH.

3.0 CONCRETE

- ALL CONCRETE WORK INCLUDING FORMING, REINFORCING, MIXING, PLACING AND CURING SHALL BE DONE IN ACCORDANCE WITH THE ACI MANUAL OF CONCRETE PRACTICE INCLUDING BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI 318-11 AND SPECIFICATIONS FOR STRUCTURAL CONCRETE, 301-04.
- ALL CONCRETE SHALL ATTAIN A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF $f_c = 3,000$ PSI FOR FOUNDATIONS, FOOTINGS AND STEM WALLS. $f_c = 4,000$ PSI FOR SLABS, PIERS, COLUMNS AND BEAMS.
- MIX DESIGNS SHALL BE IN ACCORDANCE WITH METHOD 1 OR METHOD 2 OF ACI 301-04.
- ALL CONCRETE EXPOSED TO FREEZING / THAWING SHALL HAVE A MINIMUM AIR CONTENT OF 4.5% TO 7.5%.
- ALL CONCRETE SHALL HAVE A MAXIMUM WATER-CEMENT RATIO OF 0.50.
- CONCRETE SHALL ARRIVE AT THE JOBSITE WITH A SLUMP OF 3" TO 5".
- DEPOSIT CONCRETE AS NEAR AS PRACTICAL TO FINAL POSITION TO AVOID SEGREGATION DUE TO RESHANDLING OR FLOWING.
- CONCRETE SHALL BE VIBRATED INTO FORMS WHILE PLACING WITHOUT OVER-VIBRATING. REINFORCING SHALL HAVE MINIMUM CONCRETE COVER OF - CONCRETE EXPOSED TO AIR = 2"
- SLAB-ON-GRADE SHALL BE PLACED IN CONTINUOUS STRIPS PER ACI RECOMMENDATIONS.
- COORDINATE CONCRETE WORK WITH THAT OF OTHER TRADES TO ALLOW FOR SETTING OF SLEEVES, ACCESSORIES, ETC.
- ALL ANCHOR BOLTS SHALL BE IN PLACE PRIOR TO POURING OF CONCRETE.
- CONCRETE TEST REPORTS SHALL BE MADE AVAILABLE AT JOB SITE FOR REVIEW BY INSPECTOR.
- DESIGN OF CONCRETE STRUCTURAL ELEMENTS INCLUDING WALLS, FORMED SLABS, BEAMS AND COLUMNS IS IN ACCORDANCE WITH ACI 318-11.

3.1 JOINTS IN CONCRETE

- CONSTRUCTION AND/OR CONTROL JOINTS SHALL BE MADE AS DETAILED ON THE DRAWINGS. SAW-CUT CONTROL JOINTS SHALL BE INSTALLED WITHIN 12 HOURS OF CONCRETE PLACEMENT.
- CONSTRUCTION AND/OR CONTROL JOINTS FOR SLAB-ON-GRADE CONSTRUCTION SHALL BE LOCATED ON COLUMN LINES.
- MAXIMUM SPACING OF CONSTRUCTION AND/OR CONTROL JOINTS IN SLAB-ON-GRADE CONSTRUCTION SHALL BE AS SHOWN ON DRAWINGS.
- CONSTRUCTION OR CONTROL JOINTS IN CONCRETE FOUNDATION WALLS SHALL BE SPACED NO FURTHER THAN 20 FEET APART.
- CONSTRUCTION JOINTS FOR ELEVATED SLABS SHALL BE LOCATED AT THE CENTER OF THE SPAN. REINFORCEMENT SHALL BE CONTINUOUS ACROSS ALL CONSTRUCTION JOINTS.
- CONSTRUCTION JOINTS FOR RETAINING WALLS AND/OR CONTROL JOINTS IN RETAINING WALLS SHALL BE 20'-0".
- MAXIMUM SPACING OF EXPANSION JOINTS IN RETAINING WALLS SHALL BE 100'-0".

3.2 CONCRETE REINFORCEMENT

- THE REINFORCING STEEL CONTRACTOR SHALL FABRICATE ALL REINFORCEMENT AND FURNISH ACCESSORIES, CHAIRS, SPACER BARS AND SUPPORTS NECESSARY TO SECURE THE REINFORCEMENT UNLESS SHOWN OTHERWISE ON THE DRAWINGS.
- REINFORCING STEEL SHALL BE ASTM A63-04 GRADE 60.
- HELDED WIRE FABRIC SHALL CONFORM TO ASTM A185-06.
- CONCRETE REINFORCEMENT SHALL BE PLACED ACCORDING TO THE CRSI RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS.
- ALL REINFORCEMENT SPLICES SHALL BE LAPPED PER ACI REQUIREMENTS MINIMUM UNLESS NOTED OTHERWISE. PROVIDE CORNER BARS FOR ALL HORIZONTAL REINFORCEMENT AT CORNERS AND INTERSECTIONS.
- TOP BARS SHALL BE HOOKED AT END SPANS.

3.3 MASONRY

- CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-90. COMPRESSIVE STRENGTH OF MASONRY SHALL BE DETERMINED BY THE UNIT STRENGTH METHOD. NET AREA COMPRESSIVE STRENGTH, F_n , SHALL BE 300 PSI.
- MORTAR AND ITS INGREDIENTS AS DELIVERED TO THE MASON SHALL CONFORM TO ASTM C 210, TYPE S.
- MASONRY GROUT SHALL CONFORM TO ASTM C 411, TYPE S.
- GROUT FOR REINFORCED AND NON-REINFORCED MASONRY SHALL CONFORM TO ASTM C 476. MINIMUM GROUT COMPRESSIVE STRENGTH SHALL BE 2500 PSI.
- MASONRY CONSTRUCTION SHALL CONFORM TO THE LATEST STANDARDS OF THE MASONRY STANDARDS JOINT COMMITTEE (ACI 530-08/ASCE 5-05/ TMS 400-08 AND ACI 530 (ASCE 6-08) TMS 600-08.)
- MASONRY BONDING SHALL BE RUNNING BOND UNLESS NOTED OTHERWISE.

5.0 STRUCTURAL STEEL

- FABRICATION AND ERECTION OF STRUCTURAL STEEL MEMBERS IS TO BE IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES AS INDICATED IN THE NINTH EDITION OF THE MANUAL OF STEEL CONSTRUCTION FOR ALLOWABLE STRESS DESIGN UNLESS NOTED OTHERWISE.
- ALL CONNECTIONS SHALL BE BOLTED OR WELDED AND SHALL BE DESIGNED FOR THE END REACTIONS INDICATED ON PLANS. IF REACTIONS ARE NOT INDICATED, CONNECTIONS SHALL BE DESIGNED TO DEVELOP 80% OF THE ALLOWABLE UNIFORM LOAD TABULATED IN THE NINTH EDITION OF THE AISC MANUAL OF STEEL CONSTRUCTION FOR ALLOWABLE STRESS DESIGN UNLESS NOTED OTHERWISE. NUMBERS OF BOLTS MUST SATISFY MINIMUM REQUIREMENTS AS FOLLOWS -
 - BOLTS PER CONNECTION FOR 6" AND 0" DEEP MEMBERS
 - BOLTS PER CONNECTION FOR 12" AND 14" DEEP MEMBERS
 - BOLTS PER CONNECTION FOR 16" AND 18" DEEP MEMBERS
 - BOLTS PER CONNECTION FOR 21" AND 24" DEEP MEMBERS
 - BOLTS PER CONNECTION FOR 21" AND DEEPER MEMBERS
- ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS - WIDE FLANGES, CHANNELS - ASTM A992 PIPE SHAPES - ASTM A53, GRADE B OR ASTM A501 TUBE SHAPES - ASTM A500, GRADE B
- ALL FILLER METAL USED IN WELDING SHALL BE 10 KSI YIELD, LOW-HYDROGEN. ALL WELDING SHALL BE BY CERTIFIED WELDERS AND SHALL CONFORM TO THE AWS AND AISC MINIMUM REQUIREMENTS FOR WELD SIZE. ALL WELDED JOINTS SHALL CONFORM TO THE PROVISIONS OF AISC D1-104 STRUCTURAL WELDING CODE BY AMERICAN WELDING SOCIETY. ALL WORK SHALL BE PERFORMED BY FULLY QUALIFIED WELDERS IN THE TYPE OF CONSTRUCTION INVOLVED. WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.
- BOLTS SHALL BE HIGH-STRENGTH A-325 BOLTS. CONNECTIONS SHALL CONFORM TO AISC 308-104. THE SPECIFICATIONS FOR STRUCTURAL JOINTS USING A508 OR A-502 BOLTS. CONNECTIONS ARE BEARING TYPE. BOLTS SHALL BE TIGHTENED TO "SNUG-TIGHT" CONDITION.
- DESIGN OF SPECIAL CONNECTIONS BETWEEN STEEL FRAMING COMPONENTS BY OTHER THAN THE PROJECT STRUCTURAL ENGINEER OF-RECORD SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA INCLUDING, BUT NOT LIMITED TO BRACE END CONNECTIONS, MOMENT RESISTING CONNECTIONS, MODIFIED BEAM SEAT CONNECTIONS, AND MEMBER SPLICE CONNECTIONS.
- STEEL COLUMN BASES SHALL BE SIZE AS SHOWN ON PLAN WITH 5/4" DIAMETER ANCHOR BOLTS (A507) AND 1" OF NON-METALLIC, NON-SHRINK GROUT UNDER THE BASE.
- UNLESS NOTED OTHERWISE, STRUCTURAL STEEL SUPPLIER IS TO FURNISH 4 3/8" X 3/4" SHOP WELDED ANGLE FRAMES AT ALL ROOF OPENINGS. VERIFY SIZE AND LOCATION WITH CONTRACTOR BEFORE TRANSMITTING FORCES PERPENDICULAR TO THE PLANE OF THE WALL.
- SPLICE MEMBERS ONLY WHERE INDICATED AND ACCEPTED ON SHOP DRAWINGS.
- DO NOT CORRECT FABRICATION ERRORS BY GAS-CUTTING WITHOUT PERMISSION OF ENGINEER. DO NOT FLAME CUT HOLES OR ENLARGE HOLES BY BURNING.

5.4 GOLD FORMED METAL FRAMING

- ALL GOLD FORMED MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH AMERICAN IRON AND STEEL INSTITUTE (AISI) SPECIFICATION FOR THE DESIGN OF GOLD FORMED STEEL STRUCTURAL MEMBERS.
- ALL GOLD FORMED METAL FRAMING MEMBERS SHALL BE OF THE TYPE, SIZE AND GAUGE AS SHOWN ON THE DRAWINGS.
- ALL STUDS, JOISTS AND ACCESSORIES SHALL BE PRIMED WITH RUST INHIBITIVE PAINT MEETING THE PERFORMANCE REQUIREMENTS OF T1-P-8386, OR SHALL BE FORMED FROM STEEL HAVING A 6-80 GALVANIZED COATING CONFORMING TO ASTM A124.
- ALL PAINTED 12, 14 AND 16 GAUGE MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A955 WITH A MINIMUM YIELD STRENGTH OF 50 KSI.
- ALL PAINTED 18 AND 20 GAUGE MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A661 WITH A MINIMUM YIELD STRENGTH OF 40 KSI. ALL GALVANIZED 18 AND 20 GAUGE MEMBERS SHALL MEET THE REQUIREMENTS OF ASTM A651 WITH A MINIMUM YIELD STRENGTH OF 35 KSI.
- FASTENING OF COMPONENTS SHALL BE WITH SELF-DRILLING SCREWS OR WELDING SCREWS OR WELDS SHALL BE OF SUFFICIENT LENGTH TO INSURE THE STRENGTH OF THE CONNECTION. ALL WELDS SHALL BE TO THE MANUFACTURER'S RECOMMENDATIONS ON ROD TYPE, TEMPER, ELECTROLYTIC TYPING OF COMPONENTS SHALL NOT BE PERMITTED.
- ALL GOLD FORMED METAL STUDS AND JOISTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS REGARDING MINIMUM INSTALLATION STANDARDS FOR BRACING, BRIDGING AND GUSSETING.
- PROVIDE ALL ACCESSORIES INCLUDING, BUT NOT LIMITED TO, TRACKS, CLIPS, WEB STIFFENERS, ANCHORS, FASTENING DEVICES, RESILIENT CLIPS AND OTHER ACCESSORIES REQUIRED FOR A COMPLETE AND PROPER INSTALLATION, AND AS RECOMMENDED BY THE MANUFACTURER FOR THE STEEL MEMBERS USED.
- PRIOR TO FABRICATION OF FRAMING, THE CONTRACTOR SHALL SUBMIT FABRICATION AND ERECTION DRAWINGS TO OBTAIN APPROVAL.

SCHEDULE OF SPECIAL INSPECTION SERVICES					
MATERIAL / ACTIVITY	SERVICE	Y/N	APPLICABLE TO THIS PROJECT		
			EXTENT	AGENT	DATE COMPLETED
1704.2 Inspection of Fabricators	Verify fabrication/quality control procedures	In-plant review	N		
Structural steel fabricator	Submit Certificate of Compliance	Y	Continuous	CMT	
1704.3 Steel Construction	Review material markings and certificates of compliance	Y	Periodic	CMT	
Material verification of structural steel	Field inspection	Y	Periodic	CMT	
Identify markings	Field inspection	Y	Periodic	CMT	
Identify markings	Review submitals	Y	Each submital	DPOR	
Weld field materials	Review certificate of compliance and field verification	Y	Periodic	CMT	
Structural steel welding	Shop and field inspection	N	Continuous		
Complete and verify generation groove welds	N	Continuous			
Must-pass field welds	N	Continuous			
Single-pass field welds > 9/16"	Y	Periodic	CMT		
1704.4 Concrete Construction	Inspection of reinforcing steel	Field inspection	Y	Periodic	CMT
Verify use of approved design mix	Field review	Y	Periodic	DPOR	
Fresh concrete sampling	Field testing	Y	Continuous	CMT	
Inspection of anchors and reinforcing steel installed in hardened concrete	Verify anchor type, anchor dimensions, hole clearing procedure, anchor spacing, edge distance, concrete minimum thickness, anchor embedment and tightening torque	Field inspection	Periodic	CMT	
Inspection of concrete placement for proper application techniques	Field inspection	Y	Continuous	CMT	
Concrete curing operations	Field inspection	Y	Continuous	CMT	
Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports	Y	Periodic	CMT	
Inspection of formwork for shape, size, location and connections	Field inspection	Y	Periodic	CMT	
1704.5 Masonry Construction	Verify proportions of site prepared mortar, grout and prestressing grout for concrete masonry	Field and submital review	Y	Periodic	CMT
Verify construction of masonry	Field inspection	Y	Periodic	CMT	
Verify layout of reinforcing concrete masonry	Field inspection	Y	Periodic	CMT	
Verify anchor and placement of reinforcing steel	Field #2 submital review	Y	Periodic	CMT	
Verify type, size and location of anchors, including certificate of approval of masonry anchors, anchors, ties and accessories	Field inspection	Y	Level 1 - Periodic	CMT	
Verify type, size and location of anchors, including certificate of approval of masonry anchors, anchors, ties and accessories	Field inspection	Y	Level 2 - Continuous	CMT	
Verify type, size and type of reinforcing steel	Field inspection	Y	Periodic	CMT	
Verify layout of reinforcing steel	Field inspection	Y	Continuous	CMT	
Verify protection of masonry during hot/dry weather	Field inspection	Y	Periodic	CMT	
Verify grout space is clean prior to grouting	Field inspection	Y	Level 1 - Periodic	CMT	
Verify grout placement complies with code and construction document provisions	Field inspection	Y	Continuous	CMT	
Observe preparation of grouts	Field inspection	Y	Continuous	CMT	
Verify compliance with required testing and inspection provisions of construction documents and the approved submittals	Field testing and inspection	Y	Periodic	CMT	
1704.7 Bolts	Verify materials below shadow foundations are adequate to achieve the design bearing capacity	Field inspection	Y	Periodic	CMT
Verify excavation are extended to proper depth and have reached proper material	Field inspection	Y	Periodic	CMT	
Perform classification and testing of controlled fill materials	Field inspection	Y	Periodic	CMT	
Verify use of proper materials, grades, and lift thicknesses during placement and compaction of controlled fill	Field inspection	Y	Continuous	CMT	
Prior to placement of controlled fill, observe subgrade and verify that site has been prepared properly	Field inspection	Y	Periodic	CMT	

ISSUANCES

No.	Drawing Issue Description	Date
1	Design Development	08/28/19

Order Plans

PROJECT NUMBER: 20190316 PROJECT NAME: St Vincent de Paul Georgia Council Headquarters
WBRD NUMBER: 608760316 DRAWING NUMBER: 2017_SVOP_EXTERIOR ARCHITECTURE: wjg0202054.rvt 8/27/2019 4:00:11 PM

St Vincent de Paul Georgia Council Headquarters

2050 Chamblee Tucker Rd
Chamblee, GA 30314

St Vincent de Paul Georgia

Structural Notes and Special Inspections

Designer	20190316
Prepared by/Checked	Project No.
Approver	08/28/19
Issue Number	Date
Checker	
Project Address	
Author	
Scale/Annot	

S1.01