DESIGN CRITERIA: (2012 INTERNATIONAL BUILDING CODE WITH GEORGIA AMENDMENTS THROUGH 2018)

1. ROOF SNOW LOAD:
   A. GROUND SNOW LOAD (Pg) = 5 psf
2. DESIGN ROOF AND FLOOR LOADS:
   A. ROOF DEAD LOAD = 20 psf
   B. ROOF LIVE LOAD = 20 psf (REDUCIBLE PER ROOF SLOPE AND TRIBUTARY AREA)
   C. FLOOR DEAD LOAD = SEE PLAN
   D. FLOOR LIVE LOAD = 100 psf (NON-REDUCIBLE)
3. WIND LOADS:
   A. ULTIMATE DESIGN WIND SPEED (Vult) = 115 mph (3 SECOND GUST)
   B. NOMINAL DESIGN WIND SPEED (Vasd) = 89 mph (3 SECOND GUST)
   C. RISK CATEGORY II
   D. EXPOSURE B
   E. INTERNAL PRESSURE COEFFICIENT (GCpi) = ±0.18
   F. REFER TO THE DIAGRAM ON THIS SHEET FOR WIND LOADS TO COMPONENT AND CLADDING ELEMENTS DESIGNED BY OTHERS.
4. SEISMIC DESIGN DATA:
   A. SEISMIC IMPORTANCE (Ie) = 1.00 (RISK CATEGORY II)
   B. MAPPED SPECTRAL RESPONSE ACCELERATIONS S_S = 0.214, S_1 = 0.096
   C. SITE CLASS D
   D. SPECTRAL RESPONSE COEFFICIENTS S_{DS} = 0.171, S_{D1} = 0.109
   E. SEISMIC DESIGN CATEGORY B
   F. SEISMIC-FORCE-RESISTING SYSTEM: LIGHT-FRAMED WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE
   G. SEISMIC RESPONSE COEFFICIENT, Cs = 0.036
   H. RESPONSE MODIFICATION FACTOR, R = 6 1/2
   I. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE
5. DESIGN LOAD COMBINATIONS (ALLOWABLE STRESS DESIGN):
   D + L
   D + (Lr or S or R)
   D + 0.75L + 0.75(Lr or S or R)
   D + (0.6W or 0.7E)
   D + 0.75(0.6W or 0.7E) + 0.75L + 0.75(Lr or S or R)
   0.6D + 0.6W
   0.6D + 0.7E

   REFER TO ASCE 7-10 CHAPTER 12.4 FOR ADDITIONAL SEISMIC LOAD EFFECTS AND COMBINATION REQUIREMENTS.

   INCREASES IN ALLOWABLE STRESSES ARE NOT PERMITTED WITH THESE LOAD COMBINATIONS.

   COMBINATIONS THAT CONTAIN SNOW LOAD (S) SHALL INCLUDE EFFECTS OF SNOW DRIFTING WHERE APPLICABLE.

FOUNDATIONS – GENERAL:

1. CONTINUOUS WALL FOOTINGS & SPREAD FOOTINGS SHALL BEAR ON A SOIL CAPABLE OF SUSTAINING A NET ALLOWABLE BEARING PRESSURE OF 2,000 psf UNDER SERVICE LIVE AND DEAD LOADS.
2. FOOTINGS MAY BE POURED INTO AN EARTH-FORMED TRENCH IF SOIL CONDITIONS PERMIT.
3. ALL BEARING MATERIAL SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY PRIOR TO CONCRETE PLACEMENT. THE INDEPENDENT TESTING AGENCY SHALL BE THE SOLE JUDGE AS TO THE SUITABILITY OF THE BEARING MATERIAL. FOOTING ELEVATIONS SHALL BE ADJUSTED AS REQUIRED.
4. BOTTOM OF EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 1'-0" BELOW THE UNDISTURBED GROUND SURFACE.
5. FOUNDATION CONCRETE SHALL HAVE REACHED A MINIMUM COMPRESSIVE STRENGTH OF 2,000 psi BEFORE BEING LOADED. STRENGTHS SHALL BE VERIFIED BY TESTS.

CONCRETE:

1. UNLESS NOTED OTHERWISE, ALL CONCRETE SHALL BE NORMAL WEIGHT AND HAVE THE FOLLOWING MINIMUM 28-DAY COMPRESSIVE STRENGTHS:

   FOUNDATIONS — 3,000 psi
   INTERIOR SLAB ON GROUND — 3,500 psi
   EXTERIOR SLAB ON GROUND — 3,000 psi

2. REINFORCING SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
4. MINIMUM CONCRETE COVER, UNLESS NOTED OTHERWISE:

   UNFORMED SURFACE IN CONTACT WITH THE GROUND. — 3 IN.

   FORMED SURFACES EXPOSED TO EARTH OR WEATHER.
   #6 BARS AND LARGER — 2 IN.
   #5 BARS AND SMALLER — 1 1/2 IN.

   FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER:
   BEAMS, GIRDERS, AND COLUMNS — 1 1/2 IN.
   SLABS, WALLS, AND JOISTS:
   #11 BARS AND SMALLER — 3/4 IN.
   #14 AND #18 BARS — 1 1/2 IN.

5. LAP SPLICES SHALL BE IN ACCORDANCE WITH THE FOLLOWING TABLE, UNLESS NOTED OTHERWISE. WHERE CLASSES ARE NOT CALLED OUT ON DRAWINGS, USE CLASS "B" SPLICES.

BAR SIZE	TENSION SPLICES (INCHES)				COMPRESSION SPLICES (INCHES)
	TOP BARS		OTHER BARS		ALL BARS
	A	B	A	B	
#3	22	28	17	22	12
#4	29	37	22	29	15
#5	36	47	28	36	19
#6	43	56	33	43	23
#7	53	81	48	63	27
#8	72	93	55	72	30
#9	81	105	62	81	34
#10	91	116	70	91	38
#11	101	131	76	101	42

   COMPRESSION DOWEL EMBEDMENT: .22 BAR DIAMETERS
   LAP WELDED WIRE FABRIC ONE SPACING OF CROSS WIRES PLUS 2".

6. BASE PLATES, ANCHOR BOLTS, SUPPORT ANGLES, ETC., BELOW GRADE SHALL BE COVERED WITH A MINIMUM OF 3" OF CONCRETE.

METAL-PLATE-CONNECTED WOOD TRUSS SYTEM: DELEGATED ENGINEERING

1. THE DESIGN DOCUMENTS INCLUDE A SYSTEM OF CUSTOM ENGINEERED TRUSS COMPONENTS, ASSEMBLIES, AND CONNECTIONS. THE ENTIRE SYTEM, INCLUDING ALL TRUSSES, CONNECTIONS, BRIDGING, TEMPORARY AND PERMANENT BRACING SHALL BE DESIGNED BY A DELEGATED SPECIALTY PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE STRUCTURE IS LOCATED.
2. NO WANES, SKIPS, KNOTS, OR OTHER DEFECTS SHALL OCCUR IN THE PLATE CONTACT AREA OR SCARFED AREA OF WEB MEMBERS. PLATES SHALL BE CENTERED WITH EACH SIDE OF TRUSS.
3. NUMBER OF PANELS AND DIRECTION OF WEB MEMBERS TO SUIT CONDITIONS OR SIMPLE SPAN TRUSS REQUIREMENTS. SEE STRUCTURAL AND ARCHITECTURAL DRAWINGS FOR ANY SPECIAL CONDITIONS/LOCATIONS OF PANEL JOINTS.
   A. DETAILER NOTE THAT ALL WOOD MEMBER SIZES SHOWN ARE NOMINAL, U.N.O.
   B. DESIGN OF METAL PLATE CONNECTED ROOF TRUSSES TO COMPLY WITH NFPA's NATIONAL DESIGN SPECIFICATION FOR THE DESIGN OF LUMBER AND ITS FASTENINGS: ANSI/TPI 1-2002 – NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION.
4. ALTERNATE TRUSS LAYOUTS ARE ACCEPTABLE ONLY AS A CHANGE ORDER, WHICH WILL INCLUDE ENGINEERING CHARGES FOR REDESIGN OF THE STRUCTURE BY THE ENGINEER OF RECORD.
5. TRUSS DESIGN LOADS ARE AS FOLLOWS:

   SLOPED ROOF TRUSSES
   TOP CHORD:
   LIVE LOAD 20 PSF
   DEAD LOAD 10 PSF

   BOTTOM CHORD:
   DEAD LOAD 10 PSF

   PARALLEL CHORD FLOOR TRUSSES
   TOP CHORD:
   LIVE LOAD 100 PSF
   DEAD LOAD 10 PSF

   BOTTOM CHORD:
   DEAD LOAD 6 PSF

   UPLIFT SEE DIAGRAM ON THIS SHEET

   LOAD DURATION FACTORS:

DEAD LOAD	0.90
DEAD LOAD + ROOF LIVE LOAD	1.25
DEAD LOAD + WIND LOAD	1.33

   USE PATTERNED AND PARTIAL SPAN LIVE LOADS WHERE REQUIRED TO PRODUCE MAXIMUM FORCE IN ANY TRUSS MEMBER. APPLY NET WIND UPLIFT ON ROOFS.

   MECHANICAL UNITS AND OTHER SUPERIMPOSED LOADS AS SHOWN ON THE DRAWINGS.

   INDIVIDUAL TRUSSES ARE TO BE DESIGNED FOR SPECIFIC FRAMING CONDITIONS AND CONCENTRATED LOADS RESULTING FROM EQUIPMENT WEIGHTS AND OTHER LOADS AS INDICATED ON THE DRAWINGS.
6. SUBMIT SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY THE SPECIALTY ENGINEER PERFORMING THE DESIGN FOR REVIEW BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO FABRICATION. FABRICATION OF TRUSSES SHALL NOT BEGIN UNTIL THE SHOP DRAWINGS AND CALCULATIONS HAVE BEEN REVIEWED AND RETURNED APPROVED. SHOP DRAWINGS SHALL BE AVAILABLE ON THE JOB SITE DURING TIMES OF INSPECTION AND SHALL BEAR CLEAR INDICATION THAT THEY HAVE BEEN REVIEWED AND APPROVED BY THE PROJECT STRUCTURAL ENGINEER OF RECORD.
7. AN ERECTION DRAWING SHALL BE INCLUDED, IDENTIFYING TRUSS SYSTEM COMPONENTS, AS WELL AS PERMANENT BRACING REQUIRED FOR TRUSS DESIGN.
8. HANDLING, ERECTION, AND BRACING OF WOOD TRUSSES SHALL BE IN ACCORDANCE WITH BCSI 1-03 BUILDING COMPONENT SAFETY SUMMARY SHEETS. TEMPORARY BRACING DESIGN SHALL BE PER BCSI 1-03 OR DSB-89 (TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES) AS REQUIRED. COMMENTARY AND RECOMMENDATIONS BY THE TRUSS PLATE INSTITUTE.
9. WHERE ROOF PLYWOOD IS NOT PERMANENTLY ATTACHED TO TOP CHORD OF TRUSS, PROVIDE CONTINUOUS 2x4 AT 24" O.C. PERPENDICULAR TO TOP CHORD.
10. DELEGATED DESIGN COMPONENTS, TRUSS SHOP DRAWINGS, AND CALCULATIONS SHALL BE SIGNED AND SEALED BY THE ENGINEER PERFORMING THE DESIGN.
11. PLYWOOD ROOF AND WALL SHEATHING ARE DESIGNED AS DIAPHRAGMS AND SHALL COMPLY WITH APPLICABLE PROVISIONS OF THE INTERNATIONAL BUILDING CODE.
12. TRUSS PROFILES SHOWN IN PLANS, ELEVATIONS, OR DETAILS ARE SHOWN SCHEMATICALLY UNLESS A SPECIFIC PROFILE IS NOTED IN PLAN. TRUSS MANUFACTURER IS REPSONSIBLE FOR PROVIDING TRUSS PROFILES AND MEMBER SIZES REQUIRED TO SUPPORT SPECIFIC LOADS. TRUSSES SHOWN IN DETAIL ARE SHOWN SCHEMATICALLY FOR GENERAL INFORMATION, AND SHALL NOT BE INTERPRETED AS THE INTENDED DESIGN UNLESS SPECIFICALLY NOTED.
13. DEFLECTIONS OF WOOD ROOF TRUSSES SHALL BE LIMITED TO L/240 FOR LIVE LOADS AND L/180 FOR COMBINED DEAD AND LIVE LOADS. DEFLECTION OF WOOD FLOOR TRUSSES SHALL BE LIMITED TO L/480 FOR LIVE LOADS AND L/240 FOR COMBINED LIE AND DEAD LOADS.
14. GENERAL CONTRACTOR TO COORDINATE HORIZONTAL AND VERTICAL CHASES, ATTIC AND ACCESS REQUIREMENTS, INCLUDING SIZE AND LOCATION WITH MECHANICAL, ARCHITECTURAL, AND ELECTRICAL DRAWINGS.
15. SUBMIT PERMANENT AND TEMPORARY BRACING PLANS PREPARED BY THE SPECIALTY ENGINEER SPECIFICALLY FOR THIS BUILDING.
16. TRUSS TOP CHORDS SHALL BE SOUTHERN PINE GROUP II SPECIES LUMBER. EXPOSED TO VIEW TRUSSES SHALL BE OF SELECT STRUCTURAL GRADE. ALL OTHER GRADE AND SPECIES SELECTION IS AT THE DISCRETION OF THE SUPPLIER.
17. COORDINATE ALL TRUSS DETAILS WITH ARCHITECTURAL DRAWINGS. FOR CONCEALED TO VIEW TRUSSES, WEB CONFIGURATIONS WHERE SHOWN ARE SUGGESTIONS AND MAY BE MODIFIED BY THE SUPPLIER FOR ECONOMY.
18. PROVIDE SIMPSON "TSS" PLATE, TAR IMPREGNATED FELT PAPER, OR OTHER SUITABLE VAPOR BARRIER BETWEEN TRUSSES AND CONCRETE OR MASONRY BEARING SURFACES. PROVIDE G90 GALVANIZED HURRICANE ANCHORS DESIGNED FOR NET WIND UPLIFT AT ALL BEARINGS.

STRUCTURAL STEEL:

1. STEEL SHALL CONFORM TO THE FOLLOWING GRADES:

   ALL WIDE FLANGES (U.N.O.) — A992 (F_y = 50 ksi)
   ALL CHANNELS, ANGLES, PLATES, ETC. — A36 (F_y = 36 ksi)
   STRUCTURAL TUBE — A500 (F_y = 46 ksi) (GRADE B)
   STEEL PIPE — A53 (F_y = 35 ksi)
   ANCHOR BOLTS — A307, A36, OR F1554
   BOLTS — A325
   WELDING ELECTRODES — E70XX

2. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE (2010), EXCEPT AS MODIFIED IN THESE NOTES AND THE PROJECT SPECIFICATIONS.
3. THE STEEL STRUCTURE IS A NON-SELF-SUPPORTING STEEL FRAME AND IS DEPENDENT UPON DIAPHRAGM ACTION OF THE FLOOR AND/OR ROOF SYSTEMS AND ATTACHMENT TO THE PERIMETER BRACING ELEMENTS FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES. PROVIDE ALL TEMPORARY SUPPORTS REQUIRED FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES UNTIL THESE ELEMENTS ARE COMPLETE AND ARE CAPABLE OF PROVIDING THIS SUPPORT.

SPECIAL INSPECTIONS:

1. THE OWNER OR OWNER'S DESIGNATED REPRESENTATIVE FOR CONSTRUCTION SHALL EMPLOY AN APPROVED SPECIAL INSPECTION AGENCY TO PERFORM INSPECTIONS DURING CONSTRUCTION OF ALL STRUCTURAL COMPONENTS.
2. SPECIAL INSPECTION OF THE CONSTRUCTION IS REQUIRED BY THE STATE OF GEORGIA IN ACCORDANCE WITH SECTION 1704 OF THE GA-IBC AMENDMENTS 2014. CONSTRUCTION SHALL BE INSPECTED IN ACCORDANCE WITH THE SPECIAL INSPECTION PLAN SUBMITTED TO THE LOCAL AUTHORITY HAVING JURISDICTION. THE SPECIAL INSPECTION PLAN IS PART OF THE PERMIT DOCUMENTS.

MISCELLANEOUS:

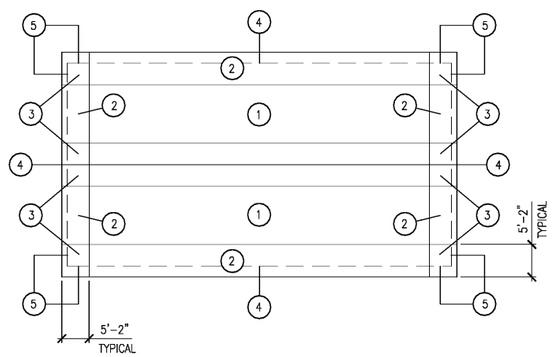
1. THESE GENERAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
2. STRUCTURAL DRAWINGS ARE INTENDED TO BE USED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING SUCH REQUIREMENTS INTO THEIR SHOP DRAWINGS AND WORK.
3. NO OPENINGS SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE PROFESSIONAL OF RECORD.
4. NO CHANGE IN SIZE OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE PROFESSIONAL OF RECORD.
5. OPENINGS 1'-4" AND LESS ON A SIDE ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SUCH OPENINGS.
6. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
7. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.
8. THE CONTRACT DRAWINGS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, SEQUENCES AND SAFETY. THE ENGINEER DOES NOT HAVE CONTROL OR CHARGE OF AND SHALL NOT BE RESPONSIBLE FOR:
   A. CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES.
   B. PRECAUTIONS AND PROGRAMS FOR SAFETY IN CONNECTION WITH THE WORK.
   C. THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
9. DO NOT SCALE THE DRAWINGS, USE DIMENSIONS SHOWN.
10. CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
11. THE CONTRACTOR SHALL INFORM THE PROFESSIONAL OF RECORD IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY OF SUCH DEVIATION BY THE PROFESSIONAL OF RECORD REVIEW OF SHOP DRAWINGS, PRODUCT DATA, ETC., UNLESS THE CONTRACTOR HAS SPECIFICALLY INFORMED THE PROFESSIONAL OF RECORD OF SUCH DEVIATION AT THE TIME OF SUBMISSION, AND THE PROFESSIONAL OF RECORD HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.
12. WHERE REQUIRED, THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE ARCHITECT FOR REVIEW BY THE STRUCTURAL ENGINEER. FABRICATION OF MATERIAL SHALL NOT BEGIN PRIOR TO RETURN OF SHOP DRAWINGS MARKED "NO EXCEPTIONS TAKEN" OR "EXCEPTIONS NOTED". WHERE EXCEPTIONS ARE NOTED, FIELD USE DRAWINGS SHALL BE SUBMITTED SHOWING THE REQUIRED CORRECTIONS. REFER TO THE PROJECT SPECIFICATIONS FOR REQUIRED SHOP DRAWING SUBMITTALS.
13. THE CONTRACTOR SHALL NOT IMPLEMENT REMEDIAL DETAILS NECESSITATED BY MISFABRICATION OR FAULTY CONSTRUCTION WITHOUT WRITTEN AUTHORIZATION FROM THE STRUCTURAL ENGINEER.

WOOD FRAMING:

1. CONFORM TO NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION, PUBLISHED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION, AF&PA NDS AND AF&PA SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC.
2. UNLESS NOTED OTHERWISE, USE SPRUCE-PINE-FIR, 19% MAX. MOISTURE CONTENT, AS FOLLOWS:

• BEAMS, HEADERS	No. 1/No. 2
• LOAD BEARING STUDS & EXTERIOR STUDS	No. 1/No. 2
• JOISTS, PURLINS	No. 1/No. 2
• SUB-PURLINS, PLATES, AND BLOCKING	No. 1/No. 2
• NON-LOAD BEARING INTERIOR STUDS	STUD GRADE

3. ANY WOOD IN CONTACT WITH CONCRETE, MASONRY, SOIL, OR EXPOSED TO THE ELEMENTS SHALL BE PRESSURE TREATED.
4. METALS (PLATES, NAILS, BOLTS, ETC.) IN CONTACT WITH PRESSURE TREATED, FIRE-RETARDANT, OR WOLMANIZED WOOD SHALL HAVE G185 GALVANIZED COATING. ALL OTHER METALS IN CONTACT WITH WOOD SHALL HAVE G90 ZINC GALVANIZED COATING.
5. METAL CONNECTORS SHALL BE FASTENED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS TO DEVELOP THE MAXIMUM PUBLISHED CAPACITY.
6. TIMBER FASTENING SHALL BE PER GOVERNING BUILDING CODE "MINIMUM FASTENING SCHEDULE" UNLESS NOTED AS GREATER ON CONSTRUCTION DRAWINGS. CONNECTORS TO BE SIMPSON OR APPROVED ALTERNATE WITH MINIMUM CAPACITY AS NOTED.
7. TOE NAILS SHALL BE DRIVEN AT AN ANGLE OF 30 DEGREES TO THE PIECE AND BE STARTED AT 1/3 THE NAIL LENGTH FROM THE END OF THE PIECE.
8. ALL BOLTS SHALL BE FURNISHED WITH STANDARD NUT WASHERS.
9. ALL NAILS SHALL BE COMMON WIRE NAILS.
10. GENERAL STRUCTURAL WOOD NOTES:
    • (3) 2x, (2) 2x, AND (2) 2x + 1/2" PLYWOOD PLATE BEAMS SHALL BE SPIKED TOGETHER WITH 12d NAILS AT 12" O.C. TOP AND BOTTOM. (4) 2x AND LARGER BEAMS SHALL BE BOLTED TOGETHER WITH 1/2" DIAMETER BOLTS AT 2'-6" O.C. MAX. TOP AND BOTTOM.
    • ROOF BEAMS WITH SPANS GREATER THAN 5'-0" SHALL BE ANCHORED EACH END WITH 1 1/4" x 16 GA. (MIN.) STRAP WITH (4) 8d NAILS INTO BEAM AND INTO STUD.
    • MULTIPLE STUD OR SOLID COLUMNS SHALL BE CONTINUOUS FROM FRAMING LEVEL, WHERE SHOWN, TO THE FOUNDATION. THIS WILL REQUIRE SOLID BLOCKING WITHIN THE FLOOR FRAMING EQUAL TO THE COLUMN DIMENSION.
    • MULTIPLE STUD PACK COMPONENTS SHALL BE CONNECTED LATERALLY TO ONE ANOTHER IN ACCORDANCE WITH THE TYPICAL DETAIL INCLUDED IN THE CONSTRUCTION DRAWINGS.
11. PROVIDE ONE ROW OF BRIDGING FOR EACH 8'-0" SPAN FOR ROOF JOISTS.
12. WALL SHEATHING NOTES: STRUCTURAL DRAWINGS OR SCHEDULES SHALL BE ATTACHED DIRECTLY TO THE FACE OF FRAMING MEMBERS. SEE ARCHITECTURAL DRAWINGS FOR ALL NON-STRUCTURAL SHEATHING REQUIREMENTS. ADDITIONAL SHEATHING REQUIRED BY ARCHITECTURAL DRAWINGS SHALL BE ATTACHED TO THE OUTSIDE FACE OF THE STRUCTURAL SHEATHING.
13. AT CONTRACTOR'S OPTION, FINGER-JOINTED LUMBER MAY BE USED FOR WALL STUDS ONLY.
14. ALL NON-LOAD BEARING AND NON-SHEAR WALLS MAY BE ANCHORED USING POWER-DRIVEN FASTENERS, 3/4" EMBEDMENT DEPTH.
15. ALL WALL DOUBLE TOP PLATES SHALL BE LAPPED AT CORNERS AND INTERSECTIONS AND FASTENED PER GOVERNING BUILDING CODE "MINIMUM FASTENING SCHEDULE" UNLESS NOTED OTHERWISE. ALL DOUBLE PLATE SPLICE JOINTS SHALL BE OFFSET AT LEAST 24". DOUBLE PLATES TO BE FASTENED TOGETHER PER GOVERNING BUILDING CODE "MINIMUM FASTENING SCHEDULE" UNLESS NOTED OTHERWISE. AT LOAD BEARING WALLS, FASTEN EACH PLATE TO PLATE WITH (2) ROWS OF 12d NAILS AT 8" O.C.
16. JOISTS OR STUDS SHALL NOT BE CUT TO INSTALL PLUMBING OR WIRING UNLESS METAL OR WOOD SIDE PIECES/PLATES ARE PROVIDED TO STRENGTHEN THE MEMBER.
17. WALL FRAMING MAY BE PANELIZED (WITHOUT GYPSUM WALLBOARD OR SHEATHING). SUBMIT PANEL DRAWINGS FOR REVIEW.
18. LET-IN BRACING FOR WALL PANELS IS NOT ALLOWED. SHEATHING FASTENED TO FACE OF WALL AT SHOP MAY SERVE AS PANEL BRACING.



1 S100 COMPONENTS & CLADDING PRESSURE DIAGRAM
SCALE: N.T.S.

	ZONE	TRIBUTARY AREA (SF)	POSITIVE PRESSURE (PSF)	NEGATIVE PRESSURE (PSF)	OVERHANGS (PSF)
ROOF	1	10	16.0	−21.8	
		20	16.0	−20.8	
		50	16.0	−20.4	
		100	16.0	−19.8	
	2	10	16.0	−37.9	−40.3
		20	16.0	−35.4	−40.3
		50	16.0	−31.4	−40.3
		100	16.0	−27.8	−40.3
	3	10	16.0	−56.0	−70.6
		20	16.0	−52.4	−67.5
		50	16.0	−48.0	−58.5
		100	16.0	−44.0	−50.4
WALLS	4	10	23.8	−25.8	
		20	22.8	−24.8	
		50	21.8	−22.8	
		100	20.8	−22.4	
		500	17.8	−19.7	
	5	10	23.8	−31.9	
		20	22.8	−29.9	
		50	21.8	−26.8	
		100	20.8	−25.4	
		500	17.8	−19.8	

NOTE: POSITIVE PRESSURE INDICATES WIND ACTING TOWARD THE RESPECTIVE SURFACE, AND NEGATIVE PRESSURE INDICATES WIND ACTING AWAY FROM RESPECTIVE SURFACE. OVERHANG PRESSURES INCLUDE PRESSURE CONTRIBUTIONS FROM BOTH UPPER AND LOWER SURFACES.

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NEST FEATHERS ADDITION
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MARK	DATE	DESCRIPTION
	10-01-18	SCHEMATIC DESIGN

R N 18130L
ILE NAME
RA N LTB
S EET TITLE
GENERAL N TES
S EET N M ER
S100