1. MINIMUM DESIGN LOADS

2. ALL DESIGN LOADS ARE IN ACCORDANCE WITH THE 2015 INTERNATIONAL BUILDING CODE OF ALABAMA.
3. DEAD LOADS
   - 3.1. ROOF — 15 PSF
4. LIVE LOADS
   - 4.1. MAIN FLOORS @ PUBLIC AREAS — 100 PSF
   - 4.2. ROOF — 20 PSF
5. SNOW LOAD
   - 5.1. GROUND SNOW LOAD — 10 PSF
6. WIND LOADS
   - 6.1 BASIC WIND SPEED -- 3 SECOND GUST (ULT) — 115 MPH
   - 6.2. IMPORTANCE FACTOR — 1.0
   - 6.3. WIND EXPOSURE CATEGORY — B
   - 6.4. INTERNAL PRESSURE COEFFICIENT, GCpi — ±0.18
   - 6.5. ULTIMATE DESIGN WIND PRESSURE FOR COMPONENTS & CLADDING (PSF):
     - 6.5.1. ROOF — +8.3/-22.4 PSF
     - 6.5.2. WALL — +21.3/-23.3 PSF
7. SEISMIC LOADS
   - 7.1. SEISMIC IMPORTANCE FACTOR — 1.0
   - 7.2. MAPPED SPECTORAL RESPONSE Ss — 0.263 g
   - 7.3. MAPPED SPECTORAL RESPONSE S1 — 0.113 g
   - 7.4. SOIL SITE CLASS PER GEOTECH REPORT — D
   - 7.5. SPECTRAL RESPONSE COEFFICIENT SDs — 0.278 g
   - 7.6. SPECTRAL RESPONSE COEFFICIENT SD1 — 0.177 g
   - 7.7. SEISMIC DESIGN CATEGORY — C
   - 7.8. BASIC SEISMIC FORCE RESISTING SYSTEM — ORDINARY CMU SHEAR WALL/STEEL MOMENT FRAME
     - 7.8.1. SEISMIC BASE SHEAR — 9.4 KIPS
     - 7.8.2. SEISMIC RESPONSE COEFFICIENT, Cs — 0.089
     - 7.8.3 RESPONSE MODIFICATION FACTOR, R — 2.0
     - 7.8.4. ANALYSIS PROCEDURE USED — EQUIV. LATERAL FORCE PROCEDURE

GENERAL STRUCTURAL NOTES

1. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS OF ALL OTHER DISCIPLINES AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, CHASES, HANGERS, INSERTS, ANCHORS, HOLES AND OTHER ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS DURING THE WORK. THE ENGINEER WILL NOT ADVISE ON NOR ISSUE DIRECTION AS TO SAFETY PRECAUTIONS AND PROGRAMS.
3. THE STRUCTURAL DRAWINGS HERE IN REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL ALL STRUCTURAL WORK AND CONNECTIONS HAVE BEEN COMPLETED. THE INVESTIGATION, DESIGN, SAFETY, ADEQUACY AND INSPECTION OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
4. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR THE METHODS, TECHNIQUES, AND SEQUENCES OF THE PROCEDURES TO PERFORM THE WORK. THE SUPERVISION OF THE WORK IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
5. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO APPROVAL BY THE ENGINEER.
6. ALL STRUCTURAL SYSTEMS WHICH ARE TO BE COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH THE SUPPLIER'S INSTRUCTIONS AND REQUIREMENTS.
7. LOADING APPLIED TO THE STRUCTURE DURING THE PROCESS OF CONSTRUCTION SHALL NOT EXCEED THE SAFE LOAD-CARRYING CAPACITY OF THE STRUCTURAL MEMBERS. THE LIVE LOADING USED IN THE DESIGN OF THIS STRUCTURE ARE INDICATED IN THE "DESIGN CRITERIA NOTES". DO NOT APPLY ANY CONSTRUCTION LOADS UNTIL STRUCTURAL FRAMING IS PROPERLY CONNECTED TOGETHER AND UNTIL ALL TEMPORARY BRACING IS IN PLACE.
8. ALL ASTM AND OTHER REFERENCES ARE PER THE LATEST EDITIONS OF THESE STANDARDS, UNLESS NOTED OTHERWISE.
9. SHOP DRAWINGS AND OTHER ITEMS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE GENERAL CONTRACTOR BEFORE SUBMITTAL. THE ENGINEER'S REVIEW IS TO BE FOR CONFORMANCE WITH THE DESIGN CONCEPT AND GENERAL COMPLIANCE WITH THE RELEVANT CONTRACT DOCUMENTS. THE ENGINEER'S REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW, CHECK AND COORDINATE THE SHOP DRAWINGS PRIOR TO SUBMISSIONS. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR THE ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, DIMENSIONS, ETC.

11. PROVIDE ADEQUATE AND PROPER FLASHING WHEREVER REQUIRED AGAINST WATER INTRUSION.
12. THE DESIGNS HEREIN BELONG TO THE STRUCTURAL ENGINEER OF RECORD. A LICENSE TO CONSTRUCT THIS BUILDING FROM THESE PLANS AT A SINGLE SITE IS GRANTED TO THE CONTRACTED CLIENT. LICENSEE LIMITS LIABILITY OF THE STRUCTURAL ENGINEER OF RECORD TO THE TOTAL FEES PAID FOR WORK HEREIN. LICENSE IS NON-TRANSFERABLE. ANY BREACH OF THIS LICENSE SHALL ENTITLE THE STRUCTURAL ENGINEER OF RECORD TO PURSUE ANY AND ALL REMEDIES, AT LAW OR EQUITY, INCLUDING WITHOUT LIMITATION, INJUNCTIVE RELIEF TO PREVENT OR CEASE SUCH BREACH.
13. IT IS THE RESPONSIBILITY OF PURCHASER OF PLANS TO ENSURE THE FOLLOWING BEFORE CONSTRUCTION:
    - 13.1. CONTRACTOR MUST VERIFY ALL DIMENSIONS PRIOR TO PROCEEDING WITH CONSTRUCTION.
    - 13.2. CONTRACTOR MUST VERIFY COMPLIANCE WITH ALL LOCAL BUILDING CODES IN THE AREA THE PROJECT IS TO BE BUILT.
    - 13.3. ENGINEERING CONSULTANTS MUST INCORPORATE ACTUAL SITE CONDITIONS.
    - 13.4. ANY MODIFICATIONS TO THESE DOCUMENTS MUST BE MADE BY THE STRUCTURAL ENGINEER OF RECORD.
    - 13.5. PLANS INDICATE LOCATION ONLY. SITE CONDITIONS MUST BE VERIFIED BY OTHERS AND ACTUAL SITE CONDITION MUST BE INCORPORATED INTO ENGINEERING ASPECTS.
14. FOR ANY REASON, IF ANY PART OF THIS STRUCTURE (i.e. FLOORS, CEILINGS, ...etc.) IS DESIGNED BY OTHER PARTIES, THE STRUCTURAL ENGINEER OF RECORD CLAIMS NO RESPONSIBILITY FOR, BUT NOT LIMITED TO, THE LATERAL RESISTANCE, STABILITY OF THE STRUCTURE, PROPER TRANSFER OF DESIGN LOADS, ANCHORAGE, HOLD DOWN, AND ANY OTHER ATTACHMENTS OR CONNECTION METHODS.
15. COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF EACH BUILDING COMPONENT NOT DESIGNED BY THE DESIGN TEAM-OF-RECORD AND NOT SPECIFIED ON THE PROJECT CONSTRUCTION DOCUMENTS SHALL BE SEALED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA AND SHALL BE AVAILABLE AT THE JOB SITE DURING THE TIMES OF INSPECTION."
16. DESIGN OF SPECIAL CONNECTIONS BETWEEN STEEL FRAMING COMPONENTS BY OTHER THAN STRUCTURAL ENGINEER OF RECORD SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA, INCLUDING BUT NOT LIMITED TO BRACE END CONNECTIONS, MOMENT RESISTING CONNECTIONS, MODIFIED BEAM SEAT CONNECTIONS, AND MEMBER SPLICE CONNECTIONS.

FOUNDATION NOTES

1. ALL FOOTINGS SHALL BEAR ON UNDISTURBED, FIRM, NATURAL SOIL OR ENGINEERED SOIL CAPABLE OF SUPPORTING A MINIMUM DESIGN BEARING PRESSURE OF 2,000 PSF UNLESS DATA TO SUBSTANTIATE THE USE OF A HIGHER VALUE ARE SUBMITTED AND APPROVED. ALL FOUNDATION EXCAVATIONS SHALL BE EVALUATED BY THE GEOTECHNICAL ENGINEER / TESTING AGENCY PRIOR TO POURING FOUNDATION CONCRETE.
2. ALL FOUNDATION CONCRETE SHALL OBTAIN A 28 DAY COMPRESSIVE STRENGTH OF 3,000 PSI.
3. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60.
4. UNLESS NOTED OTHERWISE, MINIMUM CONCRETE COVER SHALL BY PROVIDED IN ACCORDANCE WITH ACI-20[illegible] SECTION 7.7.1.
5. REINFORCING STEEL, INCLUDING HOOKS AND BENDS, SHALL BE DETAILED IN ACCORDANCE WITH LATEST EDITION OF THE ACI 318. ALL REINFORCED STEEL INDICATED AS BEING CONTINUOUS ("CONT.") SHALL HAVE MINIMUM LAP OF "B" TYPE (ACI 318-11) AT SPLICES UNLESS NOTED OTHERWISE.
6. NO UNBALANCED BACK FILLING SHALL BE DONE AGAINST FOUNDATION WALLS UNLESS WALLS ARE SECURELY BRACED AGAINST OVERTURNING EITHER BY TEMPORARY BRACING OR BY PERMANENT CONSTRUCTION.
7. PRIOR TO COMMENCING ANY FOUNDATION WORK, COORDINATE WORK WITH ANY EXISTING UTILITIES. FOUNDATIONS SHALL BE LOWERED WHERE REQUIRED TO AVOID UTILITIES.
8. PROVIDE CONSTRUCTION JOINTS IN ALL CONCRETE WORK AS REQUIRED BY THE ACI CODE OR AS SHOWN ON THE INDIVIDUAL DETAILS.
9. PROVIDE PROPER AND ADEQUATE DRAINAGE BEHIND ANY TYPE OF RETAINING AND/OR BASEMENT WALLS AS THE SITE CONDITIONS REQUIRE IN THE FIELD.
10. ALL FOOTINGS AND FOUNDATIONS SHALL BE PLACED BELOW THE "FROST - DEPTH" OF THE GEOGRAPHIC AREA OF THE PROJECT.
11. IN THE PRESENCE OF THE GROUND WATER TABLE ABOVE ANY FOOTING OR FOUNDATION, THE GENERAL CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF RECORD FOR ANY DESIGN REVISION.
12. ALL STEEL EXPOSED TO WATER, MOISTURE, AND / OR CORROSIVES SHALL BE COVERED WITH APPROPRIATE PROTECTIVE APPROVED COATING MATERIALS.

STRUCTURAL STEEL NOTES

1. ALL STRUCTURAL STEEL SHALL CONFORM TO THE THIRTEENTH EDITION OF THE "MANUAL OF STEEL CONSTRUCTION" OF THE AISC.
2. UNLESS NOTED OTHERWISE, ALL MATERIALS SHALL BE IN ACCORDANCE WITH THE FOLLOWING ASTM SPECIFICATIONS:

MEMBER	ASTM	MIN. STRENGTH
STRUCTURAL TUBING	A-500 (GRADE B)	46 KSI
ROLLED SHAPES	A-992	50 KSI
PLATES	A-36	36 KSI
CONNECTION BOLTS	A-325 (MIN. TYPE N)	92 KSI
ANCHOR BOLTS	F1554	36 KSI
THREADED RODS	A-36	36 KSI
NONSHRINK GROUT	C-1107	8,000 PSI

3. UNLESS NOTED OTHERWISE, ALL CONNECTIONS SHALL BE SHEAR TYPE CONNECTIONS EXCEPT AS NOTED OTHERWISE AND DESIGNED BY THE FABRICATOR FOR THE FACTORED SHEAR FORCES INDICATED ON PLAN IN ACCORDANCE WITH THE AISC SPECIFICATIONS FOR LOAD AND RESISTANCE FACTOR DESIGN. MINIMUM BOLT DIAMETER SHALL BE 3/4". UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE SHEAR/BEARING TYPE BOLTS AND BE "SNUG-TIGHT".
4. ALL WELDING SHALL BE IN ACCORDANCE WITH AWS D1.1 USING E70XX ELECTRODES. UNLESS NOTED OTHERWISE, PROVIDE CONTINUOUS MINIMUM SIZED FILLET WELDS PER AWS REQUIREMENTS. ALL FILLER MATERIAL SHALL HAVE A MINIMUM YIELD STRENGTH OF 58 KSI. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.
5. HOLES IN STEEL SHALL BE DRILLED OR PUNCHED. ALL SLOTTED HOLES SHALL BE PROVIDED WITH SMOOTH EDGES. BURNING OF HOLES AND TORCH CUTTING AT THE SITE IS NOT PERMITTED.
6. UNLESS NOTED OTHERWISE, ALL STRUCTURAL STEEL PERMANENTLY EXPOSED TO VIEW SHALL BE SHOP PAINTED WITH ONE COAT OF SSPC 15-68, TYPE 1 (RED OXIDE) PAINT.
7. THE STRUCTURAL STEEL ERECTOR SHALL PROVIDE ALL TEMPORARY GUYING AND BRACING (SEE 'GENERAL STRUCTURAL NOTES').
8. COLUMNS, ANCHOR BOLTS, BASE PLATES, ETC., HAVE BEEN DESIGNED FOR THE FINAL COMPLETED CONDITION AND HAVE NOT BEEN INVESTIGATED FOR POTENTIAL LOADING ENCOUNTERED DURING STEEL ERECTION AND CONSTRUCTION. ANY INVESTIGATION OF THE COLUMNS, ANCHOR BOLTS, BASE PLATES, ETC. FOR ADEQUACY DURING THE STEEL ERECTION AND CONSTRUCTION PROCESS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
9. PROTECTIVE COATINGS DAMAGED DURING THE TRANSPORTING, ERECTING, AND FIELD WELDING PROCESSES SHALL BE REPAIRED IN THE FIELD TO MATCH THE SHOP APPLIED COATING.
10. UNLESS NOTED OTHERWISE, ALL BEAM CONNECTIONS SHALL BE STANDARD FRAMED OR SEATED CONNECTIONS AS SHOWN IN THE AISC MANUAL OF STEEL CONSTRUCTION. UNLESS GREATER REACTIONS ARE INDICATED ON THE DRAWINGS, THE CONNECTIONS SHALL DEVELOP AT LEAST ONE HALF OF THE TOTAL UNIFORM LOAD CAPACITY TABULATED IN THE TABLES OF THE MANUAL FOR THE GIVEN SIZE AND SPAN OF THE BEAM IN QUESTION. IN NO CASE SHALL THE LENGTH OF THE FRAME CONNECTIONS BE LESS THAN ONE HALF OF THE "T" DISTANCE OF THE BEAM WEB.
11. PROVIDE STIFFENER PLATES ON EACH SIDE OF THE WEB OF BEAM OR GIRDER AT POINTS OF CONCENTRATED LOADS OR SEATED BEAM BEARING LOCATIONS. MINIMUM STIFFENER THICKNESS SHALL BE 1/2" UNLESS NOTED OTHERWISE.
12. ALL STEEL COMPONENTS IN CONTACT WITH EACH OTHER TO BE WELDED WITH THE LARGER OF ¼" WELD OR MIN. SIZED WELDS PER AISC REQUIREMENTS. WELD ALL AROUND EDGES AND PERIMETERS OF ALL AFFECTED MEMBERS, UNLESS NOTED OTHERWISE ON THE INDIVIDUAL DETAILS.
13. ALL STEEL EXPOSED TO WATER, MOISTURE, AND / OR CORROSIVES SHALL BE COVERED WITH APPROPRIATE PROTECTIVE APPROVED COATING MATERIALS.
14. ALL SHOP DRAWINGS SUBMITTED FOR APPROVAL (IF INCLUDED IN THE CONTRACT) NEED TO BE SEALED, SIGNED, AND DATED BY A REGISTERED ENGINEER IN THE STATE THE PROJECT IS TO BE BUILT.

MASONRY NOTES

1. MASONRY CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 350) PUBLISHED BE THE AMERICAN CONCRETE INSTITUTE.
2. HOLLOW LOAD-BEARING MASONRY UNITS SHALL CONFORM TO ASTM REQUIREMENTS. THE MINIMUM PRISM COMPRESSIVE STRENGTH (f 'm) SHALL BE 1,550 PSI AT AN AGE OF 28 DAYS, AS DETERMINED BY THE UNIT STRENGTH METHOD OF ACI 530.
3. FILL ALL BOND BEAMS AND REINFORCED CELLS SOLIDLY WITH GROUT. GROUT SHALL CONFORM TO ASTM REQUIREMENTS AND SHALL OBTAIN A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2,500 PSI
4. REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ASTM REQUIREMENTS, GRADE 60. SHOP FABRICATE REINFORCING BARS WHICH ARE SHOWN TO BE HOOKED OR BENT. PROVIDE A MINIMUM LAP OF 48 TIMES THE BAR DIAMETERS AT ALL SPLICES, UNLESS NOTED OTHERWISE.
5. THE USE OF MASONRY-CEMENT MORTAR IS STRICTLY PROHIBITED. MORTAR SHALL CONFORM TO ASTM REQUIREMENTS: ALL MORTAR SHALL MEET THE "PROPORTION SPECIFICATION" OF ASTM REQUIREMENTS AND BE MADE WITH PORTLAND CEMENT LIME (NON AIR-ENTRAINED).
6. UNLESS NOTED OTHERWISE, ALL WALLS SHALL BE LAID IN RUNNING BOND. BOND CORNERS AND INTERSECTIONS OF LOAD BEARING WALLS.
7. VERTICAL REINFORCEMENT OF AT LEAST (1) #4 BAR SHALL BE PROVIDED AT CORNERS, WITHIN 16" OF EACH SIDE OPENINGS, WITHIN 8" OF THE ENDS OF WALLS, AND AT A MAXIMUM SPACING OF 10' ON CENTER. PROVIDE BARS AT ALL WALL CORNERS, INTERSECTIONS, AND OPENING EDGES.
8. PROVIDE REBAR DOWELS FROM FOUNDATIONS TO MATCH VERTICAL REINFORCING SIZE AND SPACING. DOWELS SHALL HAVE STANDARD 90 DEGREE HOOKS AND LAP WITH THE FIRST LIFT OF REINFORCING.
9. PROVIDE HORIZONTAL BOND WITH CONTINUOUS REINFORCING AS INDICATED. BOND BEAM REINFORCEMENT SHALL BE AT LEAST (1) #4 BAR SPACED NO MORE THAN 10' O.C. DISCONTINUE ALL HORIZONTAL REINFORCING AT CONTROL JOINTS EXCEPT FOR THE BOND BEAMS AT BEARING ELEVATIONS. INTERMEDIATE BOND BEAMS SHALL BE PROVIDED AS REQUIRED.
10. PROVIDE STANDARD 9 GAUGE HORIZONTAL JOINT REINFORCING AT 16 INCHES ON CENTER IN ALL WALLS. PROVIDE TRUSS TYPE JOINT REINFORCING FOR ALL CONCRETE MASONRY. COORDINATE BRICK TIE-BACK REQUIREMENTS WITH THE ARCHITECTURAL DRAWINGS. UNLESS NOTED OTHERWISE, STOP HORIZONTAL JOINT REINFORCING AT CONTROL JOINTS.
11. PROVIDE BOND BEAM LINTELS AND BRICK LINTEL ANGLES ABOVE ALL WALL OPENINGS PER TYPICAL DETAILS. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF WINDOW AND DOOR OPENINGS.
12. PROVIDE STEEL JOIST AND BEAM BEARING PLATES AND OTHER ACCESSORIES AS INDICATED. PROVIDE THREE COURSES OF SOLIDLY GROUTED CMU BELOW ALL BEAM BEARINGS OVER THE WIDTH OF 2'-8", CENTERED ON THE WALL, UNLESS NOTED OTHERWISE.
13. PROVIDE CMU CONTROL JOINTS AS INDICATED, WITH ADDITIONAL JOINTS SO THAT THE SPACING BETWEEN JOINTS DOES NOT EXCEED A SPACING OF 3 x THE WALL HEIGHT (35 FEET MAXIMUM). WHERE BEAMS OR LINTELS BEAR AT CMU CONTROL JOINTS OFFSET AND LAP THE VERTICAL REINFORCING AS INDICATED.
14. THE MASONRY CONTRACTOR SHALL PROVIDE ALL REQUIRED TEMPORARY WALL BRACING DURING CONSTRUCTION (SEE GENERAL STRUCTURAL NOTES).
15. PROVIDE CONSTRUCTION JOINTS IN ALL MASONRY WORK AS REQUIRED BY THE ACI CODE OR AS SHOWN ON THE INDIVIDUAL DETAILS.
16. ALL STEEL EXPOSED TO WATER, MOISTURE, AND / OR CORROSIVES SHALL BE COVERED WITH APPROPRIATE PROTECTIVE APPROVED COATING MATERIALS.
17. BRICK VENEER ANCHORED TO STUD WALLS SHALL UTILIZE WIRE ANCHORS AT 16" O.C. VERTICALLY AND HORIZONTALLY. WIRE ANCHORS SHALL BE AT LEAST WIRE SIZE W1.7 AND HAVE ENDS BEND TO FORM AN EXTENSION FROM THE BEND AT LEAST 2" LONG. WIRE ANCHORS SHALL BE WITHOUT DRIPS. WIRE ANCHORS SHALL BE PLACED AS FOLLOWS: A) WITH SOLID UNITS, EMBED ANCHORS IN THE MORTAR JOINT AND EXTEND INTO THE VENEER A MINIMUM OF 1½" WITH AT LEAST ⅝" MORTAR COVER TO THE OUTSIDE FACE. B) WITH HOLLOW UNITS, EMBED ANCHORS IN MORTAR OR GROUT AND EXTEND INTO THE VENEER A MINIMUM OF 1½" WITH AT LEAST ⅝" MORTAR OR GROUT COVER TO THE OUTSIDE FACE.

CAST-IN-PLACE CONCRETE NOTES

1. DESIGN OF CONCRETE STRUCTURAL ELEMENTS INCLUDING WALLS, FORMED SLABS, BEAMS, AND COLUMNS IS IN ACCORDANCE W/ ACI 318-11 (BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE)
2. CONCRETE MIXES SHALL BE DESIGNED PER ACI 301 USING PORTLAND CEMENT, AGGREGATES AND ADMIXTURES CONFORMING TO ASTM REQUIREMENTS. CONCRETE SHALL BE READY-MIXED IN ACCORDANCE WITH ASTM REQUIREMENTS.
3. CONCRETE SHALL CONFORM TO THE FOLLOWING COMPRESSIVE STRENGTH, SLUMP AND WATER/CEMENT RATIO REQUIREMENTS:

CONCRETE	MIN. f'c (28 DAYS)	SLUMP*	W/C RATIO
COLUMNS	4,000 PSI	2" TO 4"	.46
ELEVATED SLABS	4,000 PSI	2" TO 4"	.46
CONCRETE NOT NOTED	3,000 PSI	2" TO 4"	.50
FOUNDATION	3,000 PSI	2" TO 4"	.50
SLABS-ON-GRADE	3,000 PSI	2" TO 4"	.50

4. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE."
5. ALL REINFORCING STEEL SHALL CONFORM TO ASTM REQUIREMENTS GRADE 60. ALL WELDING OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH AWS REQUIREMENTS. EPOXY COATED REINFORCING SHALL CONFORM TO ASTM REQUIREMENTS.
6. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 GRADE 60
7. ALL REINFORCING STEEL SHALL BE SET AND TIED IN PLACE PRIOR TO POURING OF CONCRETE, EXCEPT THAT VERTICAL DOWELS FOR MASONRY WALL REINFORCING MAY BE "FLOATED" IN PLACE. DO NOT FIELD BEND BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE UNLESS SPECIFICALLY INDICATED OR APPROVED BY THE ENGINEER.
8. REINFORCING STEEL, INCLUDING HOOKS AND BENDS, SHALL BE DETAILED IN ACCORDANCE WITH LATEST EDITION OF THE ACI 318. ALL REINFORCED STEEL INDICATED AS BEING CONTINUOUS ("CONT.") SHALL HAVE MINIMUM LAP OF "B" TYPE (ACI 318-11) AT SPLICES UNLESS NOTED OTHERWISE.
9. UNLESS NOTED OTHERWISE, MINIMUM CONCRETE COVER SHALL BE PROVIDED IN ACCORDANCE WITH ACI 318-11.
10. BAR SUPPORTS SHALL BE PROVIDED FOR ALL REINFORCING STEEL TO INSURE MINIMUM SUPPORT AND HOLDING BARS SHALL BE PER CONCRETE COVER. BAR SUPPORTS SHALL BE PLASTIC TIPPED OR STAINLESS STEEL.
11. UNLESS OTHERWISE NOTED ON THE INDIVIDUAL DETAILS, ALL CONCRETE WALLS (OTHER THAN RETAINING WALLS AND BASEMENT WALLS) SHALL HAVE MINIMUM REINFORCEMENT AS FOLLOWS:

WALL THICKNESS	HORIZONTAL	VERTICAL	LOCATION
4" TO 6"	#4 @ 16" O.C.	#4 @ 16" O.C.	CENTERED
8"	#4 @ 12" O.C.	#4 @ 12" O.C.	CENTERED
10"	#4 @ 16" O.C.	#4 @ 16" O.C.	EACH FACE
12"	#4 @ 12" O.C.	#4 @ 12" O.C.	EACH FACE

12. THE CONTRACTOR SHALL PROVIDE DOCUMENTATION THAT ALL MATERIALS CONFORM TO THE QUALITY STANDARDS SPECIFIED IN THE APPLICABLE BUILDING CODE.
13. IN ORDER TO AVOID CONCRETE SHRINKAGE OR CRACKING, PLACE CONCRETE SLABS IN ALTERNATING LANE PATTERN. THE MAXIMUM LENGTH OF SLAB CAST IN ANY ONE CONTINUOUS POUR SHALL BE LIMITED TO 80 FEET
14. FORM WORK SHALL REMAIN IN PLACE UNTIL CONCRETE HAS OBTAINED AT LEAST 90% OF ITS 28 DAY COMPRESSIVE STRENGTH. THE CONTRACTOR SHALL PROVIDE ALL SHORING AND RE-SHORING.
15. PROVIDE CONSTRUCTION JOINTS IN ALL CONCRETE WORK AS REQUIRED BY THE ACI CODE OR AS SHOWN ON THE INDIVIDUAL DETAILS.
16. ALL STEEL EXPOSED TO WATER, MOISTURE, AND / OR CORROSIVES SHALL BE COVERED WITH APPROPRIATE PROTECTIVE APPROVED COATING MATERIALS.
17. RESULTS FOR ALL CONCRETE COMPRESSIVE STRENGTH TESTS SHALL BE AVAILABLE ON THE JOB SITE FOR REVIEW BY THE INSPECTOR

WOOD FRAMING NOTES

1. ALL WOOD FRAMING MATERIAL SHALL BE SURFACED DRY AND USED AT 19% MAXIMUM MOISTURE CONTENT.
2. ALL STUD AND WALL FRAMING SHALL BE EITHER NO. 2 GRADE SOUTHERN YELLOW PINE (SYP) OR NO. 2 GRADE SPRUCE-PINE-FIR (SPF).
3. ALL JOIST, RAFTER, AND MISCELLANEOUS FRAMING SHALL BE NO. 2 GRADE SOUTHERN YELLOW PINE. PROVIDE FULL-DEPTH (OR METAL) BRIDGING AT MID-SPAN AND AT A MAXIMUM SPACING OF 8'-0" O.C. IN BETWEEN
4. ALL FRAMING EXPOSED TO THE WEATHER OR IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE-TREATED IN ACCORDANCE WITH THE AMERICAN WOOD PRESERVER'S ASSOCIATION SPECIFICATIONS. WHERE POSSIBLE, ALL CUTS AND HOLES SHOULD BE COMPLETED BEFORE TREATMENT. CUTS AND HOLES DUE TO ON-SITE FABRICATION SHALL BE BRUSHED WITH 2 COATS OF COPPER NAPHTHENATE SOLUTION CONTAINING A MINIMUM OF 2% METALLIC COPPER IN SOLUTION (PER AWPA STD. M4).
5. THE CONTRACTOR SHALL CAREFULLY SELECT LUMBER TO BE USED IN LOAD BEARING APPLICATIONS. THE LENGTH OF SPLIT ON THE WIDE FACE OF 2" NOMINAL LOAD BEARING FRAMING SHALL BE LIMITED TO THE SPLIT ON WIDE FACE OF 3" (NOMINAL) AND THICKER LUMBER SHALL BE LIMITED TO 1/2 OF THE NARROW FACE DIMENSION.
6. ALL NAILING NOT OTHERWISE INDICATED SHALL BE IN ACCORDANCE WITH SCHEDULE 2304.9.1 OF THE 2012 IBC. NAILING SHALL NOT BE OVERDRIVEN.
7. PROVIDE DOUBLE JOISTS UNDER ALL PARTITIONS THAT RUN PARALLEL WITH JOISTS AND UNDER ALL CONCENTRATED LOADS FROM FRAMING ABOVE.
8. PROVIDE HEADER BEAMS OF THE SAME SIZE AS JOISTS OR RAFTERS TO FRAME AROUND OPENINGS IN THE PLYWOOD DECK UNLESS NOTED OTHERWISE.
9. STRUCTURAL STEEL PLATE CONNECTORS SHALL CONFORM TO ASTM A-36 SPECIFICATIONS AND BE 1/4" THICK UNLESS OTHERWISE INDICATED. PROVIDE WASHERS 3/4" DIAMETER UNLESS OTHERWISE INDICATED. PROVIDE WASHERS FOR ALL BOLT HEADS AND NUTS IN CONTRACT WITH WOOD SURFACES.
10. BOLT HOLES SHALL BE CAREFULLY CENTERED AND DRILLED NOT MORE THAN 1/16" LARGER THAN THE BOLT DIAMETER. BOLTED CONNECTIONS SHALL BE SNUG - TIGHT BUT NOT TO THE EXTENT OF CRUSHING WOOD UNDER WASHERS
11. PRE-FABRICATED STRUCTURAL COMPOSITE LUMBER (LVL, PSL, LSL) HEADERS, BEAMS, & COLUMNS SHALL BE MANUFACTURED BY "LEVEL BY WEYERHAEUSER" (BOISE, IDAHO TEL: 888-453-8358) OR APPROVED EQUAL. DO NOT CUT OR NOTCH MATERIAL WITHOUT THE MANUFACTURER'S APPROVAL. ALL PSL COLUMNS SHALL HAVE THE FOLLOWING MINIMUM DESIGN STRESS PROPERTIES:
    - MODULUS OF ELASTICITY (E) = 1,800,000 PSI
    - FLEXURAL STRESS (F_b) = 2,400 PSI
    - SHEAR STRESS (F_v) = 190 PSI
12. PREFABRICATED METAL JOIST HANGERS, HURRICANE CLIPS, HOLD-DOWN ANCHORS, AND OTHER ACCESSORIES SHALL BE AS MANUFACTURED BY "SIMPSON STRONG-TIE COMPANY", TEL (800-999-5099), OR APPROVED EQUAL. INSTALL ALL ACCESSORIES PER THE MANUFACTURER'S REQUIREMENTS. ALL STEEL SHALL HAVE A MINIMUM THICKNESS OF 0.04 INCHES (PER ASTM A-446, GRADE A) AND BE GALVANIZED (COATING G60).
13. ALL HARDWARE AND FASTENERS USED FOR PRESSURE TREATED WOOD, TIMBER, AND LUMBER SHALL BE MADE FROM APPROVED CORROSIVE-RESISTANT MATERIALS.
14. ALL EXTERIOR WALLS TO BE CONSIDERED SHEAR WALLS, THEREFORE, ALL EXTERIOR WALLS TO BE FULLY SHEATHED AND FULLY BLOCKED USING 2" NOMINAL BLOCKING MINIMUM AT ALL EDGES. ALL COLUMNS TO BE BRACED AT THE TOP AND BOTTOM. ALL CONTINUOUS COLUMNS TO BE BRACED AT EACH FLOOR LEVEL, UNLESS NOTED OTHERWISE.
15. ALL WOOD COLUMNS TO BE BRACED AT THE TOP AND THE BOTTOM. THEREFORE, ON ALL WOOD COLUMNS, USE THE APPLICABLE SIMPSON ABU POST BASE AND CC/ECC OR PC/EPC POST CAP.

SCHEDULE OF SPECIAL INSPECTION SERVICES

MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT: Y/N	EXTENT
1704.2 INSPECTION OF FABRICATIONS			
VERIFY FABRICATION/QUALITY CONTROL PROCEDURES.	IN-PLANT REVIEW	Y	PERIODIC
1704.3 STEEL CONSTRUCTION			
MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS, AND WASHERS	REVIEW MATERIAL MARKINGS AND CERTIFICATES OF COMPLIANCE	Y	PERIODIC
INSPECTION OF HIGH-STRENGTH BOLTING:	FIELD INSPECTION		
A. BEARING-TYPE CONNECTIONS		Y	PERIODIC
B. PRE-TENSIONED OR SLIP CRITICAL			
1) TURN-OF-NUT WITH MATCHING MARKINGS		N	PERIODIC
2) DIRECT TENSION INDICATOR		N	PERIODIC
3) TWIST-OFF BOLT		N	PERIODIC
4) TURN-OF-NUT WITHOUT MATCHING MARKINGS		N	CONTINUOUS
5) CALIBRATED WRENCH		N	CONTINUOUS
MATERIAL VERIFICATION OF STRUCTURAL STEEL:			
A. IDENTIFICATION MARKINGS	FIELD INSPECTION	Y	PERIODIC
B. CERTIFIED MILL TESTS	REVIEW SUBMITTALS	Y	EACH SUBMITTAL
WELD FILLER MATERIALS.	REVIEW CERTIFICATE OF COMPLIANCE AND FIELD VERIFICATION	Y	PERIODIC AND EACH SUBMITTAL
STRUCTURAL STEEL WELDING:	SHOP AND FIELD INSPECTION		
A. COMPLETE AND PARTIAL PENETRATION GROOVE WELDS		N	CONTINUOUS
B. MULTI-PASS FILLET WELDS		Y	CONTINUOUS
C. SINGLE-PASS FILLET WELDS > 5/16		Y	CONTINUOUS
D. SINGLE-PASS FILLET WELDS < 5/16		Y	PERIODIC
E. FLOOR AND DECK WELDS		N	PERIODIC
REINFORCING STEEL WELDING	SHOP AND FIELD INSPECTION		
A. VERIFICATION OF WELDABILITY OF STEEL OTHER THAN ASTM A 706		N	PERIODIC
B. REINFORCING STEEL-RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL CONCRETE SHEAR WALLS, AND SHEAR REINFORCEMENT		N	CONTINUOUS
C. SHEAR REINFORCEMENT		N	CONTINUOUS
D. OTHER REINFORCING STEEL		N	PERIODIC
INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS.	FIELD INSPECTION		
A. DETAILS SUCH AS BRACING AND STIFFENING		N	PERIODIC
B. MEMBER LOCATIONS		N	PERIODIC
C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION		N	PERIODIC
1704.4 CONCRETE CONSTRUCTION			
INSPECTION OF REINFORCING STEEL INSTALLATION.	FIELD INSPECTION	Y	PERIODIC
INSPECTION OF ANCHORS AND REINFORCING STEEL INSTALLED IN HARDENED CONCRETE: VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, HOLE DIMENSIONS, HOLE CLEANING PROCEDURES, ANCHOR SPACING, EDGE DISTANCES, CONCRETE MINIMUM THICKNESS, ANCHOR EMBEDMENT AND TIGHTENING TORQUE	FIELD INSPECTION	Y	PERIODIC
VERIFICATION OF REQUIRED DESIGN MIX.	REVIEW SUBMITTALS	Y	PERIODIC
FRESH CONCRETE SAMPLING	FIELD TESTING	Y	CONTINUOUS
INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	FIELD REVIEW	[illegible]	CONTINUOUS
CONCRETE CURING OPERATIONS	FIELD REVIEW	[illegible]	PERIODIC
EVALUATION OF CONCRETE STRENGTH.	FIELD TESTING AND REVIEW OF LABORATORY REPORTS	Y	PERIODIC
VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS	REVIEW FIELD TESTING AND LABORATORY REPORTS	N	PERIODIC
INSPECTION OF FORMWORK FOR SHAPE, LINES, LOCATION AND DIMENSIONS	FIELD INSPECTION	Y	PERIODIC
1704.5 MASONRY CONSTRUCTION			
VERIFY PROPORTIONS OF SITE PREPARED MORTAR, GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	FIELD AND SUBMITTAL REVIEW	[illegible]	PERIODIC
VERIFY CONSTRUCTION OF MORTAR JOINTS	FIELD INSPECTION	Y	PERIODIC
VERIFY LOCATION OF REINFORCEMENT AND CONNECTORS, PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGES.	FIELD INSPECTION	Y	PERIODIC
VERIFY PRESTRESSING TECHNIQUE	FIELD INSPECTION	N	PERIODIC
VERIFY SIZE AND LOCATION OF STRUCTURAL MASONRY ELEMENTS	FIELD AND SUBMITTAL REVIEW	Y	PERIODIC
VERIFY TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	FIELD INSPECTION	Y	LVL 1 - PERIODIC
		N	LVL 2 - CONTINUOUS
VERIFY SIZE, GRADE, AND TYPE OF REINFORCEMENT	FIELD INSPECTION	Y	PERIODIC
VERIFY WELDING OF REINFORCING BARS.	FIELD INSPECTION	N	CONTINUOUS
VERIFY PROTECTION OF MASONRY DURING HOT/COLD WEATHER.	FIELD INSPECTION	Y	PERIODIC
VERIFY GROUT SPACE IS CLEAN PRIOR TO GROUTING.	FIELD INSPECTION	Y	LVL 1 - PERIODIC
		N	LVL 2 - CONTINUOUS
VERIFY GROUT PLACEMENT COMPLIES WITH CODE AND CONSTRUCTION DOCUMENT PROVISIONS.	FIELD INSPECTION	Y	CONTINUOUS
TESTING OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS REQUIRED BY CONSTRUCTION DOCUMENTS	FIELD TESTING	Y	PERIODIC
OBSERVE PREPARATION OF PRISMS REQUIRED BY CONSTRUCTION DOCUMENTS	FIELD INSPECTION	Y	CONTINUOUS
VERIFY COMPLIANCE WITH REQUIRED TESTING AND INSPECTION PROVISIONS OF CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS.	FIELD TESTING & INSPECTION	Y	PERIODIC
VERIFY GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES.	FIELD INSPECTION	N	PERIODIC
VERIFY PROPER GROUTING OF PRESTRESSING TENDONS.	FIELD INSPECTION	N	CONTINUOUS
VERIFY APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	FIELD INSPECTION	N	LVL 1 - PERIODIC
		N	LVL 2 - CONTINUOUS
1704.6 WOOD CONSTRUCTION			
INSPECTION OF THE FABRICATION PROCESS OF WOOD STRUCTURAL ELEMENTS AND ASSEMBLIES IN ACCORDANCE WITH SECTION 1704.2	IN-PLANT REVIEW	Y	PERIODIC
FOR HIGH LOAD DIAPHRAGMS, VERIFICATION OF GRADE AND THICKNESS OF STRUCTURAL PANEL SHEATHING.	FIELD INSPECTION	N	PERIODIC
FOR HIGH LOAD DIAPHRAGMS, VERIFY NOMINAL SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES, NAIL OR STAPLE DIAMETER AND LENGTH, NUMBER OF FASTENER LINES, AND THAT SPACING BETWEEN FASTENERS IN EACH LINE AND AT EDGE MARGINS AGREES WITH APPROVED BUILDING PLANS.	FIELD INSPECTION	N	PERIODIC
1704.7 SOILS			
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	FIELD INSPECTION	Y	PERIODIC
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	FIELD INSPECTION	Y	PERIODIC
PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.	FIELD INSPECTION	Y	PERIODIC
VERIFY THAT PREPARATION COMPLIES WITH APPROVED SOILS REPORT.	FIELD INSPECTION	Y	CONTINUOUS
VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.	FIELD INSPECTION	Y	CONTINUOUS
PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	FIELD INSPECTION	Y	PERIODIC
VERIFY DENSITY OF COMPACTED FILL COMPLIES WITH APPROVED SOILS REPORT.	REVIEW FIELD TESTING	Y	PERIODIC
1707.3 STRUCTURAL WOOD SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE			
INSPECTION OF FIELD GLUING OPERATIONS OF ELEMENTS OF THE SEISMIC-FORCE-RESISTING SYSTEM	FIELD INSPECTION	N	CONTINUOUS
INSPECTION OF NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC-FORCE-RESISTING SYSTEM.	SHOP AND FIELD INSPECTION	N	PERIODIC

NOTES: 1. THE INSPECTION AND TESTING AGENT(S) SHALL BE ENGAGED BY THE OWNER OR THE OWNER'S AGENT, AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL PRIOR TO COMMENCING WORK. THE QUALIFICATIONS OF THE SPECIAL INSPECTOR(S) AND/OR TESTING AGENCIES MAY BE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL AND/OR THE DESIGN PROFESSIONAL.

FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

CONNECTION	FASTENING [a,m]	LOCATION
1. JOIST TO SILL OR GIRDER	3 - 8d COMMON (2½" x 0.131") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOE NAIL
2. BRIDGING TO JOIST	2 - 8d COMMON (2½" x 0.131") 2 - 3" x 0.131" NAILS 2 - 3" 14 GAGE STAPLES	TOE NAIL EACH END
3. 1"x6" SUBFLOOR OR LESS TO EACH JOIST	2 - 8d COMMON (2½" x 0.131")	FACE NAIL
4. WIDER THAN 1"x6" SUBFLOOR TO EACH JOIST	3 - 8d COMMON (2½" x 0.131")	FACE NAIL
5. 2" SUBFLOOR TO JOIST OR GIRDER	2 - 16d COMMON (3½" x 0.162")	BLIND & FACE NAIL
6. SOLE PLATE TO JOIST OR BLOCKING	16d (3½" x 0.135") @ 16" O.C. 3" x 0.131" NAIL @ 8" O.C. 3" 14 GAGE STAPLES @ 12" O.C.	TYPICAL FACE NAIL
SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL	3 - 16d (3½" x 0.135") @ 16" O.C. 4 - 3" x 0.131" NAILS @ 16" O.C. 4 - 3" 14 GAGE STAPLES @ 16" O.C.	BRACED WALL PANELS
7. TOP PLATE TO STUD	2 - 16d COMMON (3½" x 0.131") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	END NAIL
8. STUD TO SOLE PLATE	4 - 8d COMMON (2½" x 0.131") 4 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOE NAIL
	2 - 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	END NAIL
9. DOUBLE STUDS	16d (3½" x 0.135") @ 24" O.C. 3" x 0.131" NAIL @ 8" O.C. 3" 14 GAGE STAPLE @ 8" O.C.	FACE NAIL
10. DOUBLE TOP PLATES	16d (3½" x 0.135") @ 16" O.C. 3" x 0.131" NAIL @ 12" O.C. 3" 14 GAGE STAPLE @ 12" O.C.	TYPICAL FACE NAIL
DOUBLE TOP PLATES	8 - 16d COMMON (3½" x 0.162") 12 - 3" x 0.131" NAILS 12 - 3" 14 GAGE STAPLES	LAP SPLICE
11. BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	3 - 8d COMMON (2½" x 0.131") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOE NAIL
12. RIM JOIST TO TOP PLATE	8d (2½" x 0.131") @ 6" O.C. 3" x 0.131" NAIL @ 6" O.C. 3" 14 GAGE STAPLE @ 6" O.C.	TOE NAIL
13. TOP PLATES, LAPS AND INTERSECTIONS	2 - 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL
14. CONTINUOUS HEADER, TWO PIECES	16d COMMON (3½" x 0.162")	16" O.C. ALONG EDGE
15. CEILING JOISTS TO PLATE	3 - 8d COMMON (2½" x 0.131") 5 - 3" x 0.131" NAILS 5 - 3" 14 GAGE STAPLES	TOE NAIL
16. CONTINUOUS HEADER TO STUD	4 - 8d COMMON (2½" x 0.131")	TOE NAIL
17. CEILING JOISTS, LAPS OVER PARTITIONS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	3 - 16d COMMON (3½" x 0.162") MIN. TABLE 2308.10.4.1 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
18. CEILING JOISTS TO PARALLEL RAFTERS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	3 - 16d COMMON (3½" x 0.162") MIN. TABLE 2308.10.4.1 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
19. RAFTER TO PLATE (SEE SECTION 2308.10.1, TABLE 2308.10.1)	3 - 8d COMMON (2½" x 0.131") MIN. 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOE NAIL
20. 1" DIAGONAL BRACE TO EACH STUD & PLATE	2 - 8d COMMON (2½" x 0.131") 2 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL
21. 1"x8" SHEATHING TO EACH BEARING	3 - 8d COMMON (2½" x 0.131")	FACE NAIL
22. WIDER THAN 1"x8" SHEATHING TO EACH BEARING	3 - 8d COMMON (2½" x 0.131")	FACE NAIL
23. BUILT-UP CORNER STUDS	16d COMMON (3½" x 0.162") 3" x 0.131" NAILS 3" 14 GAGE STAPLES	24" O.C. 16" O.C. 16" O.C.
24. BUILT-UP GIRDER AND BEAMS	20d COMMON (4" x 0.192") @ 32" O.C. 3" x 0.131" NAIL @ 24" O.C. 3" 14 GAGE STAPLE @ 24" O.C.	FACE NAIL AT TOP & BOTTOM, STAGGERED ON OPPOSITE SIDES
	2 - 20d COMMON (4" x 0.192") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL AT ENDS AND AT EACH SPLICE
25. 2" PLANKS	16d COMMON (3½" x 0.162")	AT EACH BEARING
26. COLLAR TIE TO RAFTER	3 - 10d COMMON (3" x 0.148") 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
27. JACK RAFTER TO HIP	3 - 10d COMMON (3" x 0.148") 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	TOE NAIL
	2 - 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL
28. ROOF RAFTER TO 2-BY RIDGE BEAM	2 - 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	TOE NAIL
	2 - 16d COMMON (3½" x 0.162") 3 - 3" x 0.131" NAILS 3 - 3" 14 GAGE STAPLES	FACE NAIL
29. JOIST TO BAND JOIST	3 - 16d COMMON (3½" x 0.162") 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL
30. LEDGER STRIP	3 - 16d COMMON (3½" x 0.162") 4 - 3" x 0.131" NAILS 4 - 3" 14 GAGE STAPLES	FACE NAIL AT EACH JOIST
31. WOOD STRUCTURAL PANELS AND PARTICLEBOARD SUBFLOOR, ROOF & WALL SHEATHING (TO FRAMING)	½" AND LESS — 6d [c,l]; 2⅜" x 0.113" NAIL [n]; 1¾" 16 GAGE [o] 19/32" TO ¾" — 8d [d] OR 6d [e]; 2⅜" x 0.113" NAIL [p]; 2" 16 GAGE [p] ⅞" TO 1" — 8d [c] 1⅛" TO 1¼" — 10d [d] OR 8d [d]	- - -
SINGLE FLOOR (COMBINATION SUBFLOOR - UNDERLAYMENT TO FRAMING)	¾" AND LESS — 6d [e] ⅞" TO 1" — 8d [e] 1⅛" TO 1¼" — 10d [d] OR 8d [e]	
32. PANEL SIDING (TO FRAMING)	½" OR LESS — 6d [f] ⅝" — 8d [f]	- - -
33. FIBERBOARD SHEATHING [g]	½" — No. 11 GAGE ROOFING NAIL [h]; 6d COMMON NAIL (2" x 0.113") 25/32" — No. 11 GAGE ROOFING NAIL [h]; 8d COMMON NAIL (2½" x 0.113"); No. 16 GAGE STAPLE [i]	- - -
34. INTERIOR PANELING	¼" — 4d [j] ⅜" — 6d [k]	- - -

FOR SI: 1 INCH = 25.4 mm.
- a. COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED.
- b. NAILS SPACED AT 6 INCHES ON CENTER AT EDGE, 12 INCHES AT INTERMEDIATE SUPPORTS EXCEPT 6 INCHES AT SUPPORTS WHERE SPANS ARE 48 INCHES OR MORE. FOR NAILING OF WOOD STRUCTURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO SECTION 2305. NAILS FOR WALL SHEATHING ARE PERMITTED TO BE COMMON, BOX OR CASING.
- c. COMMON OR DEFORMED SHANK (6d - 2" x 0.113"; 8d - 2½" x 0.131"; 10d - 3" x 0.148").
- d. COMMON (6d - 2" x 0.113"; 8d - 2½" x 0.131"; 10d - 3" x 0.148").
- e. DEFORMED SHANK (6d - 2" x 0.113"; 8d - 2½" x 0.131"; 10d - 3" x 0.148").
- f. CORROSION-RESISTANT SIDING (6d - 1⅞" x 0.106"; 8d - 2⅜" x 0.128") OR CASING (6d - 2" x 0.099"; 8d - 2½" x 0.113") NAIL.
- g. FASTENERS SPACED 3 INCHES ON CENTER AT EXTERIOR EDGES AND 6 INCHES ON CENTER AT INTERMEDIATE SUPPORTS, WHEN USED AS STRUCTURAL SHEATHING. SPACING SHALL BE 6 INCHES ON CENTER ON THE EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS FOR NONSTRUCTURAL APPLICATIONS.
- h. CORROSION-RESISTANT ROOFING NAILS WITH 7/16-INCH DIAMETER HEAD AND 1½-INCH LENGTH FOR ½-INCH SHEATHING AND 1¾-INCH LENGTH FOR 25/32-INCH SHATHING.
- i. CORROSION-RESISTANT STAPLES WITH NOMINAL 7/16-INCH CROWN AND 1⅛-INCH LENGTH FOR ½ -INCH SHEATHING AND 1½-INCH LENGTH FOR 25/32-INCH SHEATHING. PANEL SUPPORTS AT 16 INCHES (20 INCHES IF STRENGTH AXIS IN THE LONG DIRECTION OF THE PANEL, UNLESS OTHERWISE MARKED.)
- j. CASING (1½" x 0.080") OR FINISH (1½" x 0.072") NAILS SPACED AT 6 INCHES ON PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS.
- k. PANEL SUPPORT AT 24 INCHES. CASING OR FINISH NAILS SPACED AT 6 INCHES ON PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS.
- l. FOR ROOF SHEATHING APPLICATIONS, 8d NAILS (2½" x 0.113") ARE THE MINIMUM REQUIRED FOR WOOD STRUCTURAL PANELS.
- m. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16-INCH.
- n. FOR ROOF SHEATHING APPLICATIONS, FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS.
- o. FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS FOR SUBFLOOR AND WALL SHEATHING AND 3 INCHES ON CENTER AT EDGES, 6 INCHES AT INTERMEDIATE SUPPORTS FOR ROOF SHEATHING.
- p. FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS.

☒ RELEASED FOR CONSTRUCTION

KOBLASZ & KENNISON
ENGINEERING, PC
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404|860|2600

HUDDLE HOUSE
EVO 2000 RESTAURANT

US HWY 78 JASPER, AL
WALKER CO. 35501

SUBMITTALS:

DATE	DESCRIPTION
07-12-2019	STRUCTURAL DESIGN ISSUED

SHEET TITLE:

STRUCTURAL DESIGN NOTES

JOB NUMBER: 19102
ENGINEER: LHK
CHECKED BY: LHK

STAMP:

SHEET NUMBER: S-0