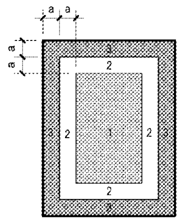


FLAT ROOF 7-27 DEG.

a = 6' - 0" MIN.

(115 MPH) ALLOWABLE (ASD) WIND PRESSURES FOR COMPONENTS & CLADDING (ASCE 7-10)						
EFFECTIVE AREA (SQ FT)	ZONE 1		ZONE 2		ZONE 3	
	PRESSURE	SUCTION	PRESSURE	SUCTION	PRESSURE	SUCTION
10	25 PSF	-25 PSF	37 PSF	-37 PSF	49 PSF	-58 PSF
20	25 PSF	-25 PSF	37 PSF	-37 PSF	49 PSF	-58 PSF
50	25 PSF	-25 PSF	37 PSF	-37 PSF	37 PSF	-37 PSF
100	25 PSF	-25 PSF	37 PSF	-37 PSF	37 PSF	-37 PSF
A ≥ 380	25 PSF	-25 PSF	25 PSF	-25 PSF	25 PSF	-25 PSF

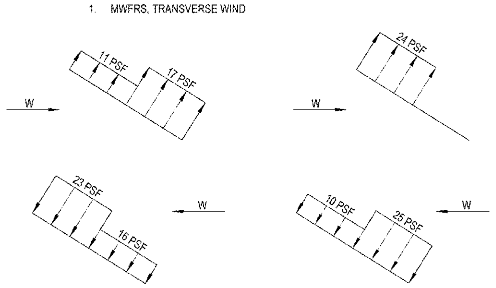
NOTES:
1. 'a' DENOTES EDGE STRIP = 6'-0"
2. POSITIVE & NEGATIVE SIGNS SIGNIFY PRESSURES ACTING TOWARDS AND AWAY FROM THE BUILDING SURFACES, RESPECTIVELY .
3. WIND PRESSURES INDICATED ABOVE ARE ALLOWABLE STRESS DESIGN WIND PRESSURES. DIVIDE THE PRESSURES BY A FACTOR OF 0.6 TO OBTAIN STRENGTH DESIGN WIND PRESSURES

COMPONENTS & CLADDING PRESSURES

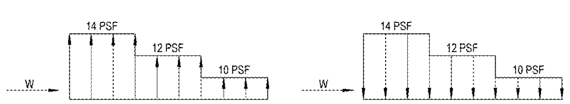
NON-BLDG STRUCTURE WIND LOAD

A. MONOSLOPE OPEN ROOF

1. MWFRS, TRANSVERSE WIND



2. MWFRS, LONGITUDINAL WIND



WIND PRESSURES & VELOCITIES GIVEN HEREIN ARE NOMINAL (V nom)

3. ROOF WL PRESSURE (MWFRS & C&C) ARE THE COMBINED EFFECT OF WIND IN BOTH SURFACES (TOP & BOTTOM)

B. 2D TRUSSES, 28 PSF ON SOLID AREA, EACH TRUSS SIMULTANEOUSLY

C. SIGN, 28 PSF. CASES A,B, AND C CONSIDERED

D. COLUMNS. SQUARE. 28 PSF ALL COLUMNS SIMULTANEOUSLY IN THE WIND DIRECTION BEING CONSIDERED

GENERAL

A. ALL WORK SHALL BE IN ACCORDANCE WITH THE 2012 VUSBC (2012 IBC), AS ADOPTED AND SUPPLEMENTED BY LOCAL REGULATION.

B. THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ARCHITECT / ENGINEER OF DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH WORK.

C. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE COMPLETE DESIGN OF THE STRUCTURE. THEY DO NOT INDICATE THE MEANS AND METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKERS, OR THEIR PERSONS DURING CONSTRUCTION.

D. OBSERVATION VISITS TO THE SITE BY EOR OR REPRESENTATIVES OF THE EOR MAY BE MADE DURING CONSTRUCTION. ANY SUPPORT SERVICES PERFORMED HEREIN SHALL BE DISTINGUISHED FROM INSPECTION AND / OR TESTING SERVICES PERFORMED BY OTHERS, AND ARE NOT TO BE CONSTRUED AS SUPERVISION AND / OR MANAGEMENT OF CONSTRUCTION.

E. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL MEMBERS AS REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT / ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS WITHIN THE STRUCTURE.

F. CONSTRUCTION MATERIALS SHALL NOT BE STACKED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED IN THE GENERAL NOTES. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT THE SUBCONTRACTORS ARE INFORMED AND DO NOT VIOLATE THIS IMPORTANT REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOORS OR ROOFS.

G. DRAWINGS ARE NOT TO BE SCALED.

H. FOR ACTUAL ELEVATION ON FIRST FLOOR (REF. ELEV. 0'-0" HEREIN), SEE SITE PLAN.

I. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO THE DETAILS PRESENTED, SIMILAR DETAILS SHALL BE USED SUBJECT TO THE REVIEW OF ENGINEER OF RECORD.

J. SUBMIT WRITTEN REQUEST TO THE ARCHITECT FOR APPROVAL OF ANY PROPOSED CHANGE TO THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. SPLICING, CUTTING, NOTCHING OR OTHER ALTERATIONS TO STRUCTURAL MEMBERS ARE NOT PERMITTED WITHOUT WRITTEN AUTHORIZATION OF THE ENGINEER. ANY UNAUTHORIZED DEVIATION FROM THE CONTRACT DOCUMENTS. AND CORRECTION THEREOF, IS THE RESPONSIBILITY OF THE CONTRACTOR. SUBSEQUENT DOCUMENTATION / REQUESTS TO BUILDING ENGINEER OF RECORD FROM GENERAL CONTRACTOR SHALL INCLUDE EVALUATION OF DEVIATIONS BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF PROJECT.

K. THE MOST STRINGENT REQUIREMENTS APPLY IN CASE OF CONFLICT BETWEEN SPECIFICATIONS, STANDARDS, CODES AND DRAWINGS.

L. DESIGN DATA

ROOF LIVE LOADS	20 PSF
DEAD LOADS (D)	
ROOF:	15 PSF, INCLUDING STRUCTURE
SIGN:	20 PSF, INCLUDING STRUCTURE
COLUMNS & TRUSSES	STEEL SELF WEIGHT (490PCF)
UPLIFT LOAD COMBINATION (D)	
ROOF:	10 PSF, INCLUDING STRUCTURE
SIGN:	10 PSF, INCLUDING STRUCTURE
COLUMNS & TRUSSES	STEEL SELF WEIGHT (490PCF)
SNOW LOADING:	
GROUND SNOW LOAD, PG:	30 PSF (1)
FLAT-ROOF SNOW LOAD. PF	21 PSF
WIND DESIGN	
VELOCITY:	115 MPH (ASCE 7-05)
IMPORTANCE:	1.00
BLDG. CATEGORY:	II, OPEN
WIND EXPOSURE:	C
INTERNAL PRESSURE COEFF:	+/- 0.00
SEISMIC DESIGN:	
IMPORTANCE:	1.0
DESIGN CATEGORY:	B
SITE CLASS:	D
BASIC S-R SYSTEM:	INVERTED PENDULUM
R (RESPONSE MOD.):	2
Cs (RESPONSE COEF.):	0.038
Ss:	0.143g
S1:	0.045g
Sds:	0.076g
Sd1:	0.024g
ANALYSIS PROCEDURE:	EQUIVALENT LATERAL FORCE
DESIGN BASE SHEAR:	DOES NOT GOVERN THIS DESIGN

SUBMITTALS

A. SHOP DRAWING REVIEW IS FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT. CORRECTIONS OR COMMENTS MADE ON THIS REVIEW DO NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR ERRORS AND OMISSIONS, AND FROM COMPLIANCE WITH THE PLANS AND SPECIFICATIONS. CORRECTIONS OR COMMENTS DO NOT AUTHORIZE AN INCREASE IN THE CONSTRUCTION BUDGET.

B. APPROVAL OF SHOP DRAWINGS DOES NOT INDICATE ACCEPTANCE OF DEVIATIONS FROM CONTRACT DOCUMENTS OR PREVIOUS SHOP DRAWING REVIEW, UNLESS SPECIFICALLY NOTED THEREIN BY ENGINEER OF RECORD.

C. ANY CHANGES TO THE DESIGN CONCEPT SHOWN IN CONTRACT DOCUMENTS SHALL BE SUBMITTED IN WRITING AND APPROVED BY THE ARCHITECT AND ENGINEER PRIOR TO SUBMITTING SHOP DRAWINGS. ALL SUCH CHANGES SHALL BE "BUBBLED" ON THE SHOP DRAWINGS AND REFERENCED TO THE PROPER RFI.

D. SUBMITTALS SHALL CONFORM TO THE REQUIREMENTS OF THE CONTRACT DRAWINGS (REFERENCE ITEM C ABOVE FOR EXCEPTION). NON-CONFORMING OR NON-REVIEWED SUBMITTALS WILL BE RETURNED WITHOUT REVIEW.

1. SHOP DRAWINGS SHALL BE "APPROVED", SIGNED AND DATED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL TO ENGINEER AND ARCHITECT OF RECORD.
2. SHOP DRAWINGS SHALL NOT CONTAIN REPRODUCTIONS OF THE CONTRACT DRAWINGS.
3. SUBMITTAL REQUIREMENTS:

SHOP DRAWINGS / DATA TO BE SUBMITTED FOR APPROVAL

CONCRETE MIX DESIGNS
REINFORCING STEEL
STRUCTURAL STEEL

SHOP DRAWINGS / DATA TO BE SUBMITTED FOR APPROVAL.
SUBMITTALS SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT

PRE-ENGINEERED METAL PANEL DECKING
PRE-ENGINEERED LAM PANEL DECKING

E. PROVIDE COPIES OF MANUFACTURERS LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION ON THE PROJECT.

FOUNDATIONS / SITE WORK

A. FOUNDATION DESIGN IS BASED UPON THE FOLLOWING SOILS REPORT:

COMPANY NAME: ECS MID-ATLANTIC, LLC
DATE: MAY 9, 2017
ECS PROJECT NO: 29:2077

B. ALLOWABLE SOIL PRESSURE IS TO BE 3000 PSF.

C. ANY FILL REQUIRED TO BACKFILL EXCAVATED AREA OR ACHIEVE FINISHED GRADE IN STRUCTURAL AREAS SHALL BE AS INDICATED BY GEOTECHNICAL ENGINEER. THE FILL SHALL BE PLACED IN LEVEL LIFTS NOT EXCEED 12" LOOSE THICKNESS AND COMPACTED TO A MINIMUM OF 95% OF THE SOIL'S MODIFIED PROCTOR MAXIMUM DRY DENSITY AS DETERMINED BY ASTM SPECIFICATION D-1557.

D. IN-PLACE DENSITY TESTS SHALL BE PERFORMED BY AN EXPERIENCED ENGINEERING TECHNICIAN. TESTS SHALL BE PERFORMED FOR EACH 2,000 SQUARE FEET, IN EVERY COLUMN FOOTING LOCATION AND EACH 50'-0" ALONG WALL FOOTINGS. COPIES OF THE TEST REPORTS SHALL BE FURNISHED TO THE STRUCTURAL ENGINEER.

E. REMOVE FREE WATER FROM EXCAVATIONS BEFORE PLACING CONCRETE.

F. CAUTION SHOULD BE USED WHEN OPERATING VIBRATORY COMPACTING EQUIPMENT NEAR THE EXISTING STRUCTURE TO AVOID THE RISK OF DAMAGE TO THE STRUCTURE.

REINFORCED CONCRETE

A. CONCRETE WORK SHALL CONFORM TO ACI-08 SPECIFICATIONS, CODES AND STANDARDS.

B. CAST-IN-PLACE CONCRETE, UNLESS OTHERWISE NOTED:

PORTLAND CEMENT PER ASTM C150
PROPORTIONED FOR MAX. ALLOWABLE UNIT SHRINKAGE OF 0.03%
25% MAXIMUM FLYASH BY WEIGHT

FOOTINGS & PEDESTAL
4000 PSI 28-DAY COMPRESSIVE STRENGTH, NORMAL WEIGHT.
MAXIMUM AGGREGATE SIZE OF 1", 0.55 MAX. W.C, 3"-4" SLUMP

C. CONCRETE COVER OVER REINFORCEMENT:

FOOTINGS & PEDESTAL
3"

D. REINFORCING, UNLESS OTHERWISE NOTED:

1. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.
2. WHERE CONTINUOUS REINFORCING IS SPECIFIED, SUCH REINFORCING MAY BE SPLICED WHERE APPROVED IN WRITING BY THE ENGINEER OF RECORD.
3. AT CHANGES IN DIRECTION OF CONCRETE WALLS, BEAMS, AND FOOTINGS, PROVIDE CORNER BARS OF SAME SIZE, QUANTITY AND SPACING AS HORIZONTAL STEEL.
4. LAP SPLICES SHALL BE 48 BAR DIAMETERS. WIRED TOGETHER.

E. THE OWNER SHALL EMPLOY A QUALIFIED TESTING LABORATORY TO PREPARE ONE SET OF FOUR TEST CYLINDERS PER DAY OR ONE SET FOR EACH 50 CUBIC YARDS POURED, MAXIMUM. LABORATORY REPORT SHALL BE FURNISHED TO THE STRUCTURAL ENGINEER SHOWING STRENGTH OF CONCRETE AT 7 (1 CYLINDER) AND 28 (2 CYLINDERS) DAYS. FINAL CYLINDER SHALL BE HELD IN RESERVE.

F. THERE SHALL BE NO HORIZONTAL JOINTS IN ANY CONCRETE POURS UNLESS SHOWN ON THE STRUCTURAL DRAWINGS. THE ENGINEER OF RECORD SHALL APPROVE ALL DEVIATIONS OR ADDITIONAL JOINTS IN WRITING.

STRUCTURAL STEEL

A. ALL STRUCTURAL STEEL WORK SHALL CONFORM TO AISC SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS-ALLOWABLE STRESS DESIGN. 13TH ED.

B. MATERIAL SHALL CONFORM TO THE FOLLOWING, EXCEPT AS NOTED:

STRUCTURAL STEEL PLATES	ASTM 572, GRADE 50
THREADED ROD ANCHOR BOLTS	ASTM F1554 GRADE 105
TUBE SECTIONS	ASTM A500, GRADE B
STRUCTURAL ANGLE & CHANNELS	ASTM A36

C. WELDING, UNLESS OTHERWISE NOTED:

1. ALL SHOP AND FIELD WELDING SHALL CONFORM TO AWS STRUCTURAL WELDING CODE, ANSI/AWS D1.1
2. ALL WELDING IN THE SHOP AND IN THE FIELD SHALL BE PERFORMED BY CERTIFIED WELDERS ONLY.
3. WELDING ELECTRODES SHALL BE E70XX LOW HYDROGEN.

D. CONNECTIONS, UNLESS OTHER WISE NOTED:

1. TIGHTEN BOLTS BY THE "SNUG-TIGHT" METHOD.
2. FIELD CONNECTIONS SHALL BE MADE WITH 3/4" DIA. MIN. HIGH-STRENGTH BEARING TYPE BOLTS (A325N) WITH THREADS ASSUMED TO BE INCLUDED IN SHEAR PLANES.

E. SHOP DRAWINGS (SEE SUBMITTALS) SHALL INCLUDE COMPLETE DETAILING OF STRUCTURAL STEEL MEMBERS AND CONNECTIONS AS REQUIRED TO FABRICATE AND ERECT ALL STRUCTURAL STEEL FRAMING.

F. GROUT UNDER BEARING PLATES SHALL BE NON-METALLIC, NON-SHRINK TYPE WITH A COMPRESSIVE STRENGTH OF AT LEAST 6000 PSI WHEN BEARING ON 3000 PSI CONCRETE, AND 8000 PSI WHEN BEARING ON 4000 PSI CONCRETE.

G. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED OR OTHERWISE PROTECTED.

H. ALL STEEL EXPOSED TO SOIL SHALL BE ENCASED IN CONCRETE OR OTHERWISE PROTECTED.

STRUCTURAL INSPECTION

A. THE OWNER SHALL EMPLOY A QUALIFIED INDEPENDENT INSPECTION AGENCY. THIS AGENCY (REFERRED TO AS INSPECTOR HEREIN) SHALL BE RESPONSIBLE FOR VERIFYING THAT PROJECT STRUCTURAL WORK IS ACCOMPLISHED IN CONFORMANCE WITH THE CONTRACT DOCUMENTS.

B. DURATION AND FREQUENCY OF JOB VISITS SHALL BE SUFFICIENT FOR THE INSPECTOR TO STATE AT THE COMPLETION OF THE PROJECT THAT THE STRUCTURAL WORK IS ACCOMPLISHED, AND ITS RELATED ELEMENTS HAVE BEEN ERECTED IN ACCORDANCE WITH THE CONTACT DOCUMENTS. SPECIFIC SYSTEMS TO BE INSPECTED INCLUDE:

FOUNDATION REINFORCING & CONCRETE
STRUCTURAL STEEL
PRE-ENGINEERED METAL PANEL DECKING
PRE-ENGINEERED LAM PANEL DECKING

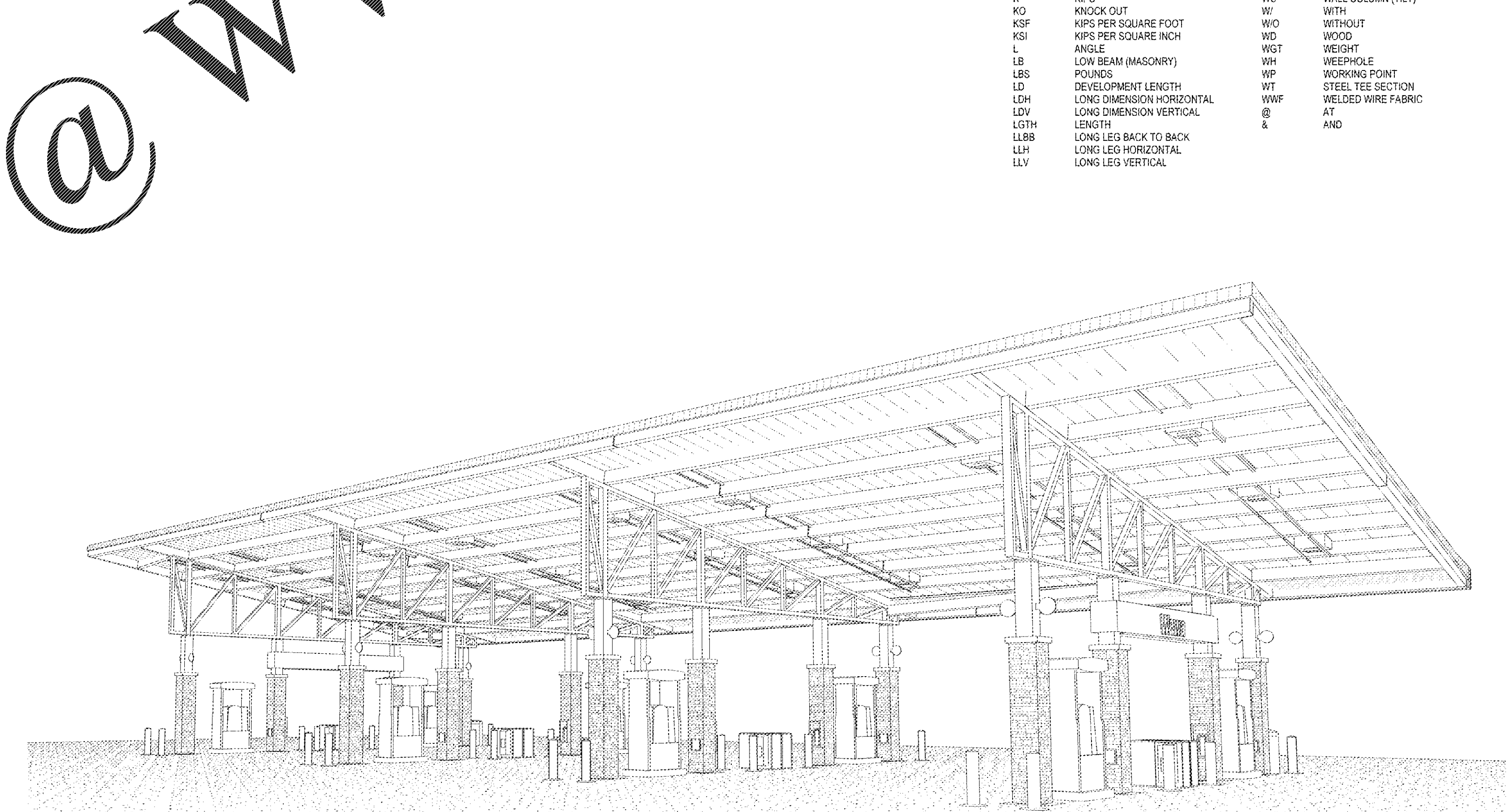
C. THE FOREGOING LIST IS NOT INTENDED TO BE ALL INCLUSIVE. THE INSPECTOR SHALL USE HIS PROFESSIONAL JUDGEMENT AND HIS KNOWLEDGE OF THE JOB SITE CONDITIONS AND THE OFFICIAL CONTRACT DOCUMENTS. THE INSPECTOR WILL NOT REPLACE THE QUALITY CONTROL PERSONNEL OF THE CONTRACTOR.

D. THE INSPECTOR DOES NOT RELIEVE THE CONTRACTORS CONTRACTUAL OR STATUTORY OBLIGATIONS. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY FOR ANY DEVIATIONS FROM THE OFFICIAL CONTRACT DOCUMENTS. THE INSPECTOR WILL NOT REPLACE THE QUALITY CONTROL PERSONNEL OF THE CONTRACTOR.

E. ALL INSPECTION REPORTS SHALL BE FORWARD BY THE INSPECTOR TO THE ENGINEER AND ARCHITECT OF RECORD.

LIST OF STRUCTURAL ABBREVIATIONS

AB	ANCHOR BOLTS	MATL	MATERIAL
ADDL	ADDITIONAL	MAX	MAXIMUM
ALT	ALTERNATE	MB	MASONRY BEAM
ANCH	ANCHOR	MC	MOMENT CONNECTION
APPROX	APPROXIMATE	MC	MASONRY COLUMN
ARCH	ARCHITECT / ARCHITECTURAL	MCJ	MASONRY CONTROL JOINT
BB	BOND BEAM	MECH	MECHANICAL
BLDG	BUILDING	MEZZ	MEZZANINE
BM	BEAM	MFR	MANUFACTURE / MANUFACTURERS
BO	BOTTOM OF	MIN	MINIMUM
BOT / BTM	BOTTOM	MISC	MISCELLANEOUS
BP	BASE PLATE / BEARING PLATE	MTL	METAL
BRG	BEARING	NIC	NOT IN CONTRACT
BTW	BETWEEN	NOM	NOMINAL
CANT	CANTILEVER	NTS	NOT TO SCALE
CB	CONCRETE BEAM	OC	ON CENTER
CC	CONCRETE COLUMN	OPNG	OPENING
CIP	CAST IN PLACE	OPP	OPPOSITE
CJ	CONSTRUCTION JOINT	PB	PARAPET BEAM (MASONRY)
CL	CENTERLINE	PCC	PRECAST CONCRETE
CLR	CLEAR / CLEARANCE	PL	PLATE
COL	COLUMN	PLF	POUNDS PER LINEAR FOOT
CONC	CONCRETE	PLY	PLYWOOD
CONNX	CONNECTION	PRE-ENG	PRE-ENGINEERED
CONSTR	CONSTRUCTION	PEMB	PRE-ENGINEERED METAL BUILDING
CONT	CONTINUOUS	PREFAB	PRE-FABRICATED
CORR	CORRUGATED	PRELIM	PRELIMINARY
CMU	CONCRETE MASONRY UNIT	PROJ	PROJECTION
DET	DETAIL	PSF	POUNDS PER SQUARE FOOT
DIA/DIAM	DIAMETER	PSI	POUNDS PER SQUARE INCH
DIM	DIMENSION	PT	PRESSURE TREATED
DIST	DISTANCE	PT	POST TENSION(ING) (ED)
DN	DOWN	PW	PANEL WIDTH
DR	DRAIN	RAD	RADIUS
DWG	DRAWING	RB	ROOF BEAM (MASONRY)
DWL	DOWEL	RCP	REINFORCED CONCRETE PIPE
EA	EACH	RD	ROOF DRAIN
EE	EACH END	REF	REFERENCE
EF	EACH FACE	REINF	REINFORCING
EJ	EXPANSION JOINT	REQ'D	REQUIRED
EL/ELEV	ELEVATION	REV	REVISION
ELECT	ELECTRICAL	RFI	REQUEST FOR INFORMATION
EMB	EMBEDMENT	RTU	ROOF TOP UNIT
ENGR	ENGINEER	RW	RETAINING WALL
EOR	ENGINEER OF RECORD	SECT	SECTION
EQ	EQUAL	SCH / SCHED	SCHEDULE
EQUIP	EQUIPMENT	SIM	SIMILAR
EQUIV	EQUILAVENT	SP / SPC	SPACE(S)
ES	EACH SIDE	SPECS	SPECIFICATIONS
EXIST	EXISTING	SQ	SQUARE
EXP	EXPANSION	SS	STAINLESS STEEL
EXT	EXTERIOR	STD	STANDARD
EW	EACH WAY	ST / STL	STEEL
FAB	FABRICATE	STRUC	STRUCTURAL
FBC	FLORIDA BUILDING CODE	SYM	SYMMETRICAL
FD	FLOOR DRAIN	T & B	TOP AND BOTTOM
FDN	FOUNDATION	T & G	TONGUE AND GROOVE
FF	FINISHED FLOOR	TE	THICKENED EDGE
FIN	FINISH(ED)	THK	THICK
FL/FLR	FLOOR	THRD	THREAD(ED)
FTG	FOOTING	TB	TIE BEAM
GA	GAGE / GAUGE	TOC	TOP OF CONCRETE
GALV / GV	GALVANIZED	TOM	TOP OF MASONRY
GC	GENERAL CONTRACTOR	TOS	TOP OF STEEL
HAS	HEADED ANCHOR STUD	TRIB	TRIBUTARY
HB	HIGH BEAM (MASONRY)	TYP	TYPICAL
HORIZ	HORIZONTAL	TS	TUBE STEEL
HSB	HIGH STRENGTH BOLTS	UNO	UNLESS NOTED OTHERWISE
HSS	HOLLOW STEEL SECTION	UON	UNLESS OTHERWISE NOTED
HT	HEIGHT	VERT	VERTICAL
INT	INTERIOR	VIF	VERIFY IN FIELD
JST	JOIST	W	WIDE FLANGED
JT	JOINT	WB	WALL BEAM (TILT)
K	KIPS	WC	WALL COLUMN (TILT)
KO	KNOCK OUT	W/	WITH
KSF	KIPS PER SQUARE FOOT	W/O	WITHOUT
KSI	KIPS PER SQUARE INCH	WD	WOOD
L	ANGLE	WGT	WEIGHT
LB	LOW BEAM (MASONRY)	WH	WEEPHOLE
LBS	POUNDS	WP	WORKING POINT
LD	DEVELOPMENT LENGTH	WT	STEEL TEE SECTION
LDH	LONG DIMENSION HORIZONTAL	WWF	WELDED WIRE FABRIC
LDV	LONG DIMENSION VERTICAL	@	AT
LGTH	LENGTH	&	AND
LLBB	LONG LEG BACK TO BACK		
LLH	LONG LEG HORIZONTAL		
LLV	LONG LEG VERTICAL		



1 3D VIEW - PERSPECTIVE
CS1

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PROJECT NO. 2170584
DATE 01/04/2019
DRAWN RC
CHECKED JJ

Revision Schedule		
No.	Description	Date
	PERMIT SET	01/04/2019
1	PERMIT COMMENTS	03/15/2019
	BID SET	06/27/2019

PROJECT NAME
STACKED 8 GAS CAN. 2018.01-MA
STORE #8658-SUD
10691 DAVIDSON PLACE (1)
MANASSAS, VA 20109

CLIENT NAME
WAWA
260 W. BALTIMORE PIKE
WAWA, PA 19063

SHEET TITLE
DESIGN CRITERIA AND NOTES

CS1

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