

1 FRAMING PLAN
3/16" = 1'-0"

2 TOWER FRAMING PLAN
3/16" = 1'-0"

- FRAMING GENERAL NOTES**
1. BEAM CONNECTIONS ARE TO BE DESIGNED BY DELEGATED ENGINEER. SUBMITTALS TO BE PROVIDED AS COVERED IN THE SPECIFICATIONS ON SHEET S1.
 2. SEE SHEET S1 FOR DESIGN CRITERIA AND SPECIFICATIONS.
 3. REACTIONS SHOWN ON BEAMS ARE LRFD BASED ON THE WORST CASE LOADING COMBINATION PER ASCE 7.
 4. USE 10-KIPS AS THE MINIMUM DESIGN LOADING FOR MEMBERS WHERE NO REACTION HAS BEEN SHOWN.
 5. USE BOLTED CONNECTIONS FOR W SHAPES TO HSS COLUMNS. WELD HSS MEMBERS TO HSS MEMBERS - BLIND BOLTED / BOLTED CONNECTIONS CAN BE USED WITH APPROVAL FROM ENGINEER. SEE TYPICAL DETAILS REFERENCED IN DRAWINGS.
 6. NOTE TO JOIST MANUFACTURER: PROVIDE STANDARD BRIDGING COMPLYING WITH THE APPLICABLE STEEL JOIST INSTITUTE SPECIFICATIONS TYPICAL FOR GRAVITY AND UPLIFT LOADS SUPERIMPOSED ON ALL JOISTS. DIAGONAL BRIDGING SHALL BE PROVIDED BETWEEN ADJACENT JOISTS WHENEVER BOTTOM BOARD HORIZONTAL BRIDGING IS DISCONTINUOUS.
 7. DESIGN JOIST FOR AN ADDITIONAL CONCENTRATED LOAD APPLIED TO THE TOP CHORD WHERE SHOWN ON THE ROOF FRAMING PLAN. LOADS SHOWN ARE BASED UPON EQUIPMENT SPECIFIED TYPE OF DESIGN VEHICLE THAT EQUIPMENT LOCATIONS AND UNIT WEIGHTS HAVE BEEN IDENTIFIED.
 8. REFER TO DESIGN CRITERIA SHEET S1 FOR JOIST LOADINGS.
 9. () SLOPE INDICATES GENERAL DIRECTION OF STEELDECK AND ARCHITECTURAL SHEATHING PITCH WITH ELEVATIONS ON PLAN AND ARCHITECTURAL DRAWINGS.
 10. METAL ROOF DECK SHALL HAVE A MINIMUM OF 3 SPANS.
 11. PROVIDE ADDITIONAL SUPPORT AROUND DRAINS. SEE TYPICAL DETAILS ON DRAWING S7.1. COORDINATE LOCATIONS WITH ARCHITECTURAL AND PLUMBING DRAWINGS.
 12. COORDINATE AND VERIFY ALL SECTIONS AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
 13. FINALIZE THE LOCATION OF HVAC UNITS AND ROOF OPENINGS WITH MECHANICAL AND PLUMBING PLANS.
 14. [VJ32.XX] INDICATES VIRTUAL JOIST SIZE AND HAS BEEN INCLUDED AS A REFERENCE FOR JOIST ENGINEER AND IS INTENDED TO SUPPLEMENT THE DESIGN LOADS GIVEN BELOW AND ON SHEET S1.

- METAL ROOF DECK**
1. 1.5B20 METAL ROOF DECK. (F_y = 80 ksi)
 2. FASTEN USING (7) 5/8" PUDDLE WELDS @ EA. SUPPORT PER 36" WIDTH (36/7), (8) #10 TEK SCREWS @ MID-SPAN OF SIDELAPS AND 5/8" PUDDLE WELDS @ 6" O.C. ALONG EDGES
- ROOF DECK CLOSURE ANGLE**
1. CONT L4X4X3/8" U.N.O.
 2. DECK CLOSURE ANGLE SHOULD BE CONTINUOUS ALONG THE LENGTH OF THE BUILDING

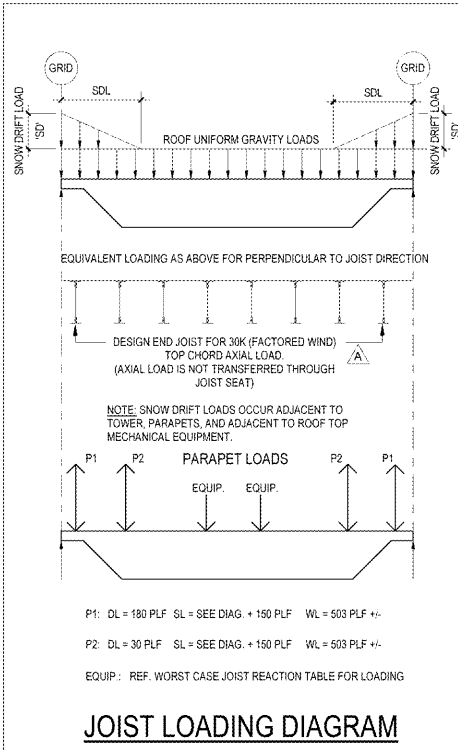
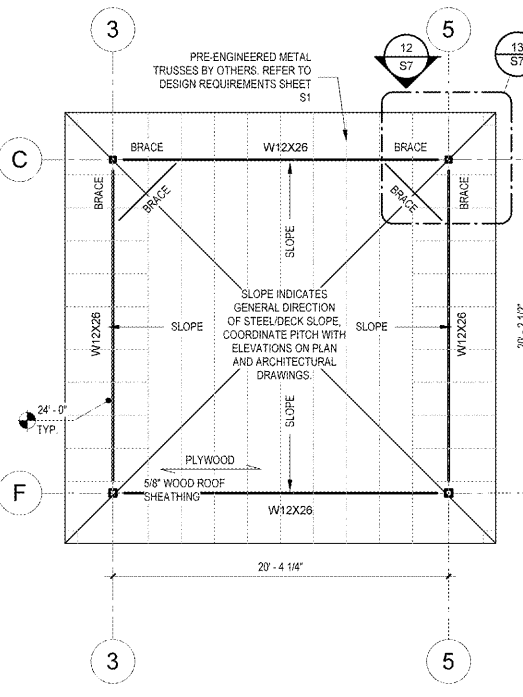
WORST CASE JOIST REACTION

MECH UNIT	MAX. VERTICAL REACTION FROM EQUIP. SUPPORT FRAME ON JOIST TOP CHORD
EF-1	100 LB
EF-2	100 LB
AC-1	750 LB
AC-2	750 LB
AC-3	750 LB
REMOTE REFRIG	1350 LB

MECHANICAL EQUIPMENT SCHEDULE

MARK	R.O LENGTH	R.O WIDTH
RTU-1	7'-7 7/8"	4'-2 3/4"
RTU-2	7'-7 7/8"	4'-2 3/4"
RTU-3	3'-2"	6'-4 1/4"

ALL DIMENSIONS MUST BE VERIFIED BY CONTRACTOR WITH EQUIPMENT SUPPLIER PRIOR TO FABRICATION OF STEEL DIMENSIONS PROVIDED ARE BASED ON COOK GRAVITY VENTS AND LENNOX RTU'S SHOWN ON MECHANICAL DRAWINGS



JOIST LOADING DIAGRAM

P1: DL = 180 PLF SL = SEE DIAG. + 150 PLF WL = 503 PLF +/-
 P2: DL = 30 PLF SL = SEE DIAG. + 150 PLF WL = 503 PLF +/-
 EQUIP.: REF. WORST CASE JOIST REACTION TABLE FOR LOADING

1925 Prospect Ave.
Orlando, FL 32814
P (407) 661-9100
F (407) 661-9101
www.c.p.com

Chalaci & Peterson
Architects Engineers Planners

CLIENT NAME
WAWA
260 W. BALTIMORE PIKE
WAWA, PA 19063

PROJECT NAME
W50FB-VA 2018.01-MA
STORE #8668-SUD
10891 DAVIDSON PLACE
MANASSAS, VA 20109

SHEET TITLE
ROOF FRAMING PLAN

Revision Schedule

No.	Description	Date
1	PERMIT SET	01/04/2019
1	REVISION 1	02/05/2019
1	PERMIT COMMENTS	03/15/2019
1	BID SET	06/27/2019

PROJECT NO.
2170584
DATE
01/04/2019
DRAWN
RC
CHECKED
JJ

S3