

STRUCTURAL DESIGN DATA

POST-TENSIONED CONCRETE NOTES

- THE REQUIRED POST-TENSIONED FORCES SHOWN ARE EFFECTIVE FORCES AND DO NOT INCLUDE ANY POST-TENSION LOSSES. THE NUMBERS AND SIZES OF THE TENDONS SHALL BE DETERMINED BY THE POST-TENSIONING CONTRACTOR AND SHALL BE SUBJECT TO THE APPROVAL OF THE ARCHITECT.
- THE POST-TENSIONING CONTRACTOR SHALL SUBMIT CALCULATIONS, TENDON FULLY ENCAPSULATED SYSTEM. STRESSING USED IN POST-TENSIONING SHALL CONFORM TO ASTM A418 WITH LATEST REVISIONS, STRESS RELIEVED, LOW RELAXATION, 7-WIRE EXTRA HIGH STRENGTH STRAND WITH A GUARANTEED MINIMUM ULTIMATE STRENGTH OF 270,000 PSI.
- THE POST-TENSIONING CONTRACTOR SHALL SUBMIT TO THE ARCHITECT, FOR REVIEW, DETAILED PRESTRESSING PROCEDURES AND SEQUENCES, AS WELL AS DETAILED CALCULATIONS TO SUBSTANTIATE THE ABOVE. ALL PRESTRESSING LOSSES SHALL BE ACCOUNTED FOR IN THE CALCULATIONS.
- THE STRESSING OF THE TENDONS MAY COMMENCE WHEN THE CONCRETE HAS ATTAINED A COMPRESSIVE STRENGTH OF 8000 PSI.
- THE POST-TENSIONING SYSTEM SHALL BE AN UNBONDED MONO-STRAND TENDON FULLY ENCAPSULATED SYSTEM. STRANDS USED IN POST-TENSIONING SHALL CONFORM TO ASTM A418 WITH LATEST REVISIONS, STRESS RELIEVED, LOW RELAXATION, 7-WIRE EXTRA HIGH STRENGTH STRAND WITH A GUARANTEED MINIMUM ULTIMATE STRENGTH OF 270,000 PSI.
- TENDONS MAY BE TEMPORARILY OVERSTRESSED TO A MAXIMUM OF 0.8 FULT AND LOCKED OFF AT A MAXIMUM STRESS OF 0.7 FULT. FRICTION LOSSES SHALL BE BASED ON EXPERIMENTALLY DETERMINED WOBBLE AND CURVATURE COEFFICIENTS AND SHALL BE VERIFIED DURING STRESSING OPERATIONS.
- UNLESS OTHERWISE SPECIFIED ON THE DRAWINGS, TENDONS SHALL BE PLACED IN SMOOTH PARABOLIC CURVES BETWEEN POINTS DIMENSIONED. HIGH AND LOW POINTS CORRESPOND TO COLUMN CENTERLINE AND MIDSPAN RESPECTIVELY. UNLESS OTHERWISE NOTED, ALL DIMENSIONS LOCATING TENDONS PROFILES APPLY TO THE CENTER OF GRAVITY OF THE GROUP OF TENDONS. TENDON PLACEMENT SHALL NOT VARY MORE THAN PLUS OR MINUS 1/8" VERTICALLY FROM THE POINTS DIMENSIONED.
- UNLESS NOTED OTHERWISE, TENDON MIDSPAN ORDNATE SHALL BE 1 1/4" AND SUPPORT (ORDNATE) SHALL BE 3/4". MEASURED FROM THE BOTTOM OF THE SLAB. TENDON ANCHORAGES AT BOTH STRESSING AND DEAD ENDS SHALL BE LOCATED AT SLAB MID-DEPTH.
- FOR BANDED TENDONS, MAXIMUM BAND WIDTH SHALL BE 3/0", EXCEPT AT ANCHORING ENDS, WHERE SPLAYING IS SHOWN ON THE DRAWINGS.
- TENDONS SHALL BE SECURELY SUPPORTED AT INTERVALS NOT EXCEEDING 36 INCHES ON CENTER. MAXIMUM NUMBER OF TENDONS IN A BUNDLE SHALL BE FOUR (4).
- STRESSING AND DEAD END ARE INTERCHANGEABLE, AT CONTRACTOR'S OPTION. EMPLOY "BOTH ENDS STRESSING" FOR UNIFORMLY SPACED TENDONS.
- A MINIMUM OF TWO (2) TENDONS SHALL PASS THROUGH COLUMN IN BOTH DIRECTIONS.
- THE POST-TENSIONING CONTRACTOR SHALL DESIGN, SUPPLY AND INSTALL ALL REQUIRED ANCHORAGE ZONE REINFORCEMENT, BUT IN NO CASE SHALL THAT REINFORCEMENT BE LESS THAN 2 #5 CONT. BARS INSTALLED PARALLEL TO THE EDGE OF SLAB AT ANCHORAGE.
- ALL OPENINGS IN POST-TENSIONED SLABS OR BEAMS SHALL BE FORMED OR SLEEVED. CORING OF POST-TENSIONED BEAMS OR SLABS WILL NOT BE PERMITTED WITHOUT THE WRITTEN CONSENT OF THE ARCHITECT. ALL OPENINGS AND/OR SLEEVES MUST BE SHOWN ON THE SHOP DRAWINGS. ANY ADDITIONAL OPENINGS, NOT SHOWN ON THE APPROVED SHOP DRAWINGS WILL REQUIRE A WRITTEN APPROVAL FROM THE ARCHITECT PRIOR TO CONSTRUCTION. MAXIMUM ALLOWED PENETRATION OF DRILLED-IN INSERTS IS 3/4". PROVIDE CAST-IN INSERTS FOR HANGING DUCTS, PIPES OR OTHER MECHANICAL EQUIPMENT WHERE POSSIBLE, AND WHERE THE SPECIFIED MAXIMUM DRILLED-IN INSERT WILL NOT PROVIDE SUFFICIENT CAPACITY.
- AFTER ACCEPTANCE AND APPROVAL OF STRESSING RECORDS BY THE ARCHITECT, CUT OFF TENDON TAILS ABOUT 1/2" INSIDE THE POCKET, SEAL TENDON TAILS WITH RUST INHIBITING TREATMENT AND DRY PACK ALL POST-TENSIONING POCKETS WITH NON-SHRINK, NON-FERROUS GROUT. THE FINAL COLOR AND FINISH OF THE PATCH IN EXPOSED AREAS SHALL MATCH THE ADJACENT CAST-IN-PLACE CONCRETE AS VERIFIED AND APPROVED BY THE ARCHITECT PRIOR TO CONSTRUCTION.
- SEE SPECIFICATIONS FOR FURTHER REQUIREMENTS FOR ALL POST-TENSIONED CONCRETE WORK.

CONCRETE NOTES

- ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF: 318-11
- EXCEPT WHERE OTHERWISE INDICATED, CONCRETE TYPES AND MINIMUM 28-DAY COMPRESSIVE STRENGTHS SHALL BE AS FOLLOWS:

SLAB ON GRADE	3000 PSI REGULAR WGT.
FOUNDATION	3000 PSI REGULAR WGT.
FLOOR SLABS	6000 PSI REGULAR WGT.
COLUMNS	6000 PSI
- CEMENT SHALL CONFORM TO ASTM C150 TYPE I. USE ONLY ONE BRAND OF CEMENT FOR ALL EXPOSED TO NEW CONCRETE. AGGREGATES SHALL CONFORM TO ASTM C33 (REGULAR WEIGHT) AND ASTM C330 (LIGHTWEIGHT). ALL CONCRETE SHALL CONTAIN AN APPROVED WATER REDUCING ADMIXTURE. NO CALCIUM CHLORIDE SHALL BE USED IN ANY CONCRETE.
- ALL REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185.
- ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED AND SPACED IN FORMS AND SECURED IN PLACE IN ACCORDANCE WITH THE PROCEDURES AND REQUIREMENTS OUTLINED IN THE LATEST EDITION OF THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", ACI 315. BAR SUPPORTS IN CONTACT WITH EXPOSED SURFACES SHALL BE PLASTIC TIPPED.
- CHECKED SHOP DRAWINGS SHOWING REINFORCING DETAILS, INCLUDING STEEL SIZE, SPACING AND PLACEMENT SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION.
- THE CONTRACTOR SHALL SUBMIT DETAILED DRAWINGS SHOWING THE LOCATIONS OF ALL CONSTRUCTION JOINTS, REVEALS, CURBS, SLAB DEPRESSIONS, SLEEVES, OPENINGS, ETC.
- ALL REINFORCING SPLICES SHALL CONFORM TO THE REQUIREMENTS OF ACI 318, LATEST EDITION, BUT IN NO CASE SHALL BE LESS THAN 36 BAR DIAMETERS, UNLESS NOTED OTHERWISE. ALL WELDED WIRE FABRIC SHALL BE LAPPED TWO (2) FULL MESH PANELS AND TIED SECURELY, WHERE REQUIRED, DOWELS SHALL MATCH SIZE AND NUMBER OF MAIN REINFORCING, UNLESS NOTED OTHERWISE.
- A MINIMUM OF TWO (2) TOP BARS SHALL PASS THROUGH EACH COLUMN IN EACH OF THE TWO DIRECTIONS. IF FULL SPAN BOTTOM BARS ARE INDICATED ON THE DRAWINGS, A MAXIMUM OF TWO (2) SUCH BARS SHALL EITHER PASS THROUGH A COLUMN AND BE FULLY DEVELOPED IN TENSION BY WAY OF ACI 318 REQUIRED DEVELOPMENT LENGTH OR STANDARD HOOK, OR BE SPLICED IN TENSION WITH TWO (2) ADJACENT SPAN BARS IN THE DIRECTION UNDER CONSIDERATION.
- 10NOT USED
- ADDITIONAL BARS SHALL BE PROVIDED AROUND ALL FLOOR AND WALL OPENINGS, AS SHOWN ON TYPICAL DETAILS.
- ALLOW A MINIMUM OF THREE (3) HOURS BETWEEN PLACEMENT OF CONCRETE FOR COLUMNS, WALLS OR PIERS AND PLACEMENT OF CONCRETE ON THE ADJACENT FLOOR.
- SEE FLOOR PLANS, SCHEDULES AND SPECIFICATIONS FOR CAMBER REQUIREMENTS.
- UNLESS NOTED OTHERWISE, ALL MECHANICAL, PLUMBING AND ELECTRICAL EQUIPMENT PADS SHALL BE REINFORCED WITH AT LEAST ONE (1) LAYER OF #6-W#4 W.W.F. SEE HVAC PLUMBING AND ELECTRICAL DRAWINGS FOR ADDITIONAL REINFORCING REQUIREMENTS OF PADS.
- CONSTRUCTION JOINTS IN ALL FOUNDATION WALLS SHALL BE NO FURTHER APART THAN 40 FEET IN ANY DIRECTION. ALL CONSTRUCTION JOINTS SHALL BE BROUGHT, CLEANED AND MOISTENED IMMEDIATELY PRIOR TO PLACING NEW CONCRETE. SEE DRAWINGS FOR CONSTRUCTION JOINT DETAILS.
- PLACE ALL SLABS-ON-GRADE WITH AN APPROVED PATTERN AND SEQUENCE OF CONSTRUCTION AND CONSTRUCTION JOINTS TO MINIMIZE SHRINKAGE CRACKS. THE MAXIMUM SPACING BETWEEN JOINTS SHALL BE 40 FEET. A SUGGESTED ARRANGEMENT AND DETAILS ARE SHOWN ON THE DRAWINGS.
- CONCRETE TESTING WILL BE PERFORMED BY THE OWNER'S TESTING LABORATORY IN ACCORDANCE WITH ACI 301-99 CHAPTER 16, EXCEPT AS FOLLOWS:
 - FOR COMPRESSIVE STRENGTH TESTS: 1) TEST TENSIONED CONCRETE IN ONE SET OF FIVE (5) SPECIMENS FOR EACH 100 CUBIC YARDS OF FRACTION THEREOF OF EACH CONCRETE CLASS PLACED IN ANY ONE DAY. TEST ONE (1) SPECIMEN AT 3 DAYS, TWO (2) SPECIMEN AT 7 DAYS, TWO (2) SPECIMENS AT 28 DAYS. KEEP ONE IN RESERVE.

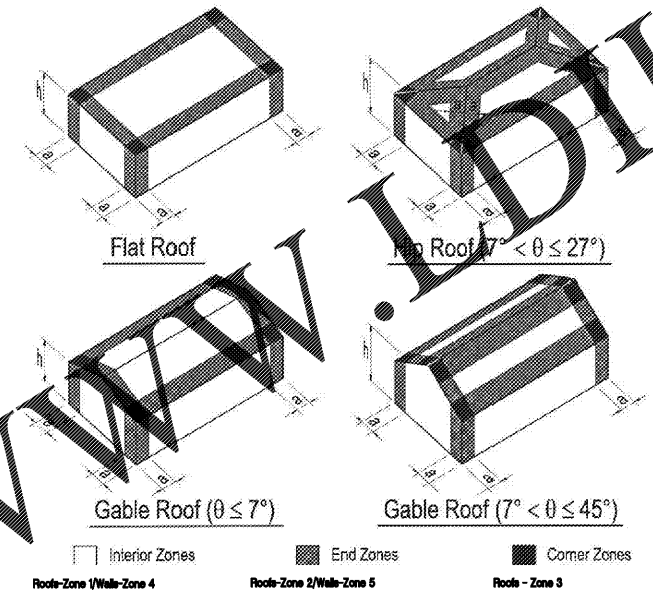
COMPONENTS & CLADDING ULTIMATE WIND DESIGN PRESSURE, Vult=115 mph

AREA, ft ²	ROOF DESIGN PRESSURE, psf					WALL PRESSURE, psf					OVERHANG NET DESIGN PRESSURE	
	ZONE 1	ZONE 2	ZONE 3	ZONE 4	ZONE 5	ZONE 2	ZONE 5	ZONE 2	ZONE 5	ZONE 2	ZONE 5	
10	+22.3	-27.4	+22.3	-29.6	+22.3	+30.9	-36.5	+30.9	-42.7	-55.6	-64.6	
20	+22.3	-26.8	+22.3	-29.1	+22.3	+30.9	-35.8	+30.9	-41.9	-55.6	-62.8	
35	+22.3	-25.7	+22.3	-28.6	+22.3	+30.9	-34.2	+30.9	-40.8	-55.6	-60.2	
45	+22.3	-25.1	+22.3	-27.5	+22.3	+30.9	-33.2	+30.9	-40.2	-55.6	-58.6	

COMPONENTS & CLADDING NOMINAL WIND DESIGN PRESSURE, Vord = 89 mph

AREA, ft ²	ROOF DESIGN PRESSURE, psf					WALL PRESSURE, psf					OVERHANG NET DESIGN PRESSURE	
	ZONE 1	ZONE 2	ZONE 3	ZONE 4	ZONE 5	ZONE 2	ZONE 5	ZONE 2	ZONE 5	ZONE 2	ZONE 5	
10	+19.1	-23.1	+19.1	-28.6	+19.1	+29.1	-26.3	+29.1	-25.3	-30.9	-36.5	
20	+19.1	-22.4	+19.1	-27.1	+19.1	+29.1	-24.8	+29.1	-23.8	-30.9	-34.1	
35	+19.1	-21.1	+19.1	-25.3	+19.1	+29.1	-22.6	+29.1	-20.1	-30.9	-32.7	
45	+19.1	-19.6	+19.1	-24.1	+19.1	+29.1	-21.9	+29.1	-19.4	-30.9	-31.2	

NOTE: FOR HIP ROOF WITH ANGLE $\leq 25^\circ$, ZONE 3 SHALL BE TREATED AS ZONE 2
NOTE: NET DESIGN PRESSURE ACTING IN EITHER DIRECTION NORMAL TO THE SURFACE OF THE COMPONENTS AND CLADDING MATERIALS SHALL NOT BE LESS THAN 16 PSF.



- Notes:
- Pressures shown are applied normal to the surface, for exposure B, at h=30 ft (9.1m). Adjust for other conditions using Equation 50.3-1.
 - Plus and minus signs signify pressures acting toward and away from the surfaces, respectively.
 - For hip roofs with $\theta \leq 25^\circ$, Zone 3 shall be treated as Zone 2.
 - For effective wind areas between zones given, value may be interpolated, otherwise use the value associated with the lower effective wind area.
 - Exception:
 - 19' maximum of least horizontal dimension of 0.4h, whichever is smaller, but not less than either 4% of least horizontal dimension or 1.8 (0.9 m).
 - Mean roof height, in feet (meters), except that eave height shall be used for roof angles $< 10^\circ$.
 - Angle of plane of roof from horizontal, in degrees.

STRUCTURAL GENERAL NOTES

DESIGN DATA:
CODE: INTERNATIONAL BUILDING CODE, 2012 EDITION
ACI 318-11, ASCE 7-10,
NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION,
ANSI/AF&PA NDS-2009.

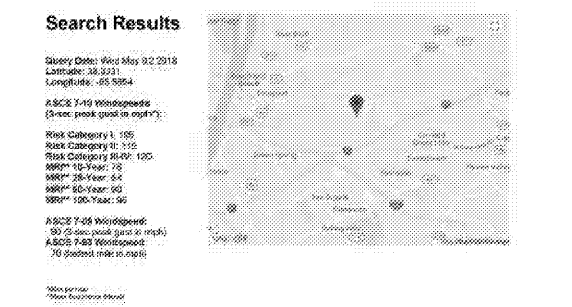
DESIGN LOADS:
DEAD LOAD:
a. Floor System
Floor (1" Gypcrete on 3/4" plywood) 9
Mechanical/Electrical/Plumbing 4
Ceiling & Misc. 6
Partition 8
Insulation 1
b. Roof System
Flat Roofing: Insulation 20 PSF
Roofing sheathing & felt (plywood) 5
Mechanical/Electrical/Plumbing 4
Ceiling & Misc. 5
Insulation 1
c. Decking/Truss System
1/2" Wt. Concrete 35 PSF
Membrane 2
Sheathing 3
Ceiling 6

WIND LOADS: SNOW = 15 psf
WIND = Truss manufacturer to take into account 115 mph design wind speed for the design of roof trusses and balcony trusses.
HANDRAIL AND GUARDRAIL = 50 pf OR A CONCENTRATED LOAD OF 200 lbs APPLIED IN ANY DIRECTION AT THE TOP OF THE RAILING.
Contractor shall submit cut sheets for all equipment including but not limited to hvac package units, air handlers, generators and chillers. Information shall include weight and any special support requirements. See architectural and mechanical drawings for details relating to roof mounted equipment curbs.

WIND LOAD CALCULATIONS

WIND LOADS: ULTIMATE DESIGN WIND SPEED, Vult=115 mph (3-SEC GUST)
NOMINAL DESIGN WIND SPEED, Vord=89 mph
CODE: INTERNATIONAL BUILDING CODE, 2012 EDITION, SEC 1609, 1620
ASCE 7-10

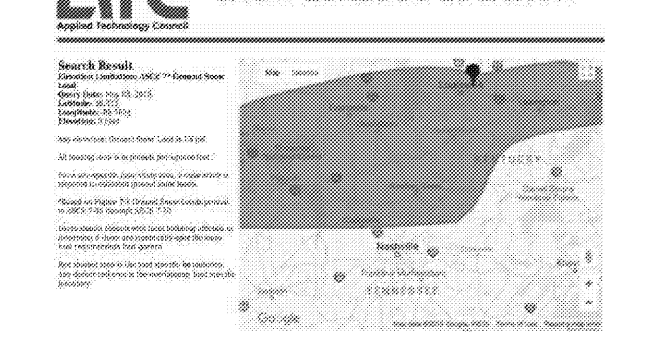
BUILDING RISK CATEGORY : II
WIND EXPOSURE: "B"
MEAN ROOF HEIGHT: h = 40' +/-
INTERNAL PRESSURE COEFFICIENT: PER BUILDING ENCLOSURE CLASSIFICATION
GUST-EFFECT FACTOR: G=0.85
WIND DIRECTIONALITY FACTOR: kd=0.85
TOPOGRAPHIC FACTOR: kzt=1.0



SEISMIC NOTES

NEUSGS Design Maps Detailed Report
ASCE 7-10 Standard (28.4.3.7) 25.4.3.7-4
Title: "Seismic Hazard Analysis Report for the Proposed Development"
Section 11.4.1 - Seismic Hazard Analysis
Section 11.4.2 - Seismic Hazard Analysis
Section 11.4.3 - Seismic Hazard Analysis
Section 11.4.4 - Seismic Hazard Analysis
Section 11.4.5 - Seismic Hazard Analysis
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ATC SNOW LOADS BY LOCATION



GROUND SNOW LOAD WEBSITE DISCLAIMER
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