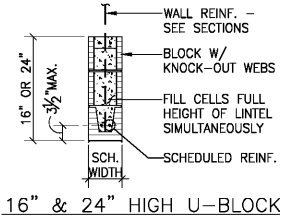


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LINTEL SCHEDULE					
MARK OR LOCATION	MAX. SPAN	TYPE	SIZE	REINFORCEMENT	REMARKS
BRICK	4'-0"	STEEL ANGLE	L6x4x5/16	-----	TRIM HORIZ. LEG TO 5 1/2" BEAR 8" EACH END
BRICK	6'-4"	STEEL ANGLE	L6x6x3/8	-----	TRIM HORIZ. LEG TO 5 1/2" BEAR 8" EACH END
BRICK	7'-8"	STEEL ANGLE	3/8"x7"x5 1/2" BENT	-----	LONG LEG VERTICAL BEAR 8" EACH END
B*CMU	3'-4"	U-BLOCK	8"x16"x8"	#5 TOP & BOTTOM	8" HIGH U-BLOCK
B*CMU	6'-4"	U-BLOCK	8"x16"x16"	2-#5 TOP & BOTTOM	16" HIGH U-BLOCK
B*CMU	7'-8"	U-BLOCK	8"x16"x24"	2-#5 TOP & BOTTOM	24" HIGH U-BLOCK
12*CMU	3'-4"	U-BLOCK	12"x16"x8"	2-#5 TOP & BOTTOM	8" HIGH U-BLOCK
12*CMU	14'-8"	U-BLOCK	12"x16"x24"	2-#5 TOP & BOTTOM	24" HIGH U-BLOCK

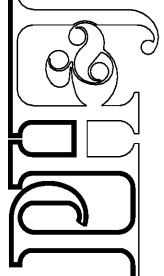
- NOTES:
- UNLESS NOTED OTHERWISE, BEAR 8" HIGH U-BLOCKS 8" EA. END AND 16" & 24" HIGH U-BLOCKS 16" EACH END.
 - FILL CELLS WITH CONCRETE FULL HEIGHT AT U-BLOCK. BEARING FOR ENTIRE LENGTH OF BEARING & REINF. EA. CELL W/ BAR SAME SIZE AS WALL REINFORCING. VERTICAL REINF. SHALL BE CONTINUOUS THRU LINTEL AT BEARING.
 - ALL COLD-FORMED STUDS SHALL USE WELDED CONNECTIONS.



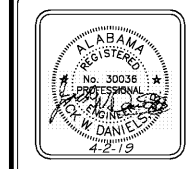
- GENERAL NOTES
- FOUNDATION:**
- PROVIDE 16" DIAMETER PILES WHERE INDICATED ON THE PLANS.
 - AUGER-CAST PILES SHALL BE CAPABLE OF SUPPORTING 180 KIIPS PER PILE WITH A FACTOR OF SAFETY OF 2. SEE THE SPECIFICATIONS AND GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION.
 - ALL PILES SHALL HAVE A MINIMUM TIP ELEVATION OF -43'-0" (APPROXIMATELY 60'-0" BELOW EXISTING GROUND SURFACE).
 - ELEVATIONS SHOWN ON PLAN ARE TOP OF PILE CAPS.
 - EXTREME CARE SHALL BE TAKEN WHILE EXCAVATING EARTH AROUND EXISTING PILES SO AS TO NOT DAMAGE THE PILE, PILE CAP OR REINFORCING.
 - ANY DAMAGED PILES OR REINFORCING WILL BE CAUSE FOR REJECTION OF THE PILE.
 - DRILL ALL PILE CAPS, COLUMNS, AND WALLS WHERE THEY ABUT WITH THE SAME STEEL AS VERTICAL, UNLESS DETAILED OTHERWISE.
 - PROVIDE HIGH STRENGTH MORTAR WITH A MINIMUM OF 5000 PSI COMPRESSIVE STRENGTH FOR THE PILES.
- CONCRETE:**
- ALL CONCRETE SHALL HAVE A COMPRESSIVE STRENGTH AT 28 DAYS OF $F_c = 5000$ PSI AND A MAXIMUM WATER-CEMENT RATIO OF 0.47. ALL CONCRETE FOR EXTERIOR APPLICATIONS SHALL CONTAIN ENTRAINED AIR, LIGHT-WEIGHT CONCRETE (WHERE SPECIFIED) SHALL HAVE A MAXIMUM DENSITY OF 110 PCF. SEE SPECS FOR ADDITIONAL INFORMATION.
 - REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60.
 - WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064.
 - UNLESS NOTED OTHERWISE PROTECTIVE COVERING OF REINFORCEMENT SHALL BE AS FOLLOWS (SEE DETAILS); FOOTINGS AND GRADE BEAMS 3" CLEAR BOTTOM AND SIDES, 1 1/2" CLEAR TOP. CONCRETE SLABS 3/4" CLEAR. WALLS 1 1/2" CLEAR SIDES. BEAMS 1 1/2" CLEAR TO STIRRUPS. CONCRETE COLUMNS 1 1/2" CLEAR TO TIES.
 - LAP ALL CONCRETE WALL VERTICAL REINFORCING AND CONCRETE BEAM HORIZONTAL REINFORCING WITH CLASS B LAP SPLICES. LAP ALL OTHER CONTINUOUS BARS WITH CLASS A SPLICES UNLESS NOTED OTHERWISE. PLACING PLANS AND DETAILS SHALL BE IN ACCORDANCE WITH THE LATEST A.C.I. DETAILING MANUAL.
 - STEEL FABRICATOR SHALL SUBMIT SHOP DRAWINGS FOR THE ARCHITECT AND/OR ENGINEER'S REVIEW.
 - DO NOT RUN CONDUITS, RACEWAYS, OR PIPES IN CONCRETE SLABS, BEAMS, OR COLUMNS WITHOUT SPECIFIC APPROVAL FROM BLACKBURN DANIELS O'BARR.
- MASONRY:**
- PROVIDE MASONRY HORIZONTAL JOINT REINFORCEMENT 16" O.C. VERTICAL IN ALL CONCRETE BLOCK WALLS. REINFORCEMENT SHALL BE FOR TOTAL WIDTH OF CAVITY WALLS.
 - WHERE CONCRETE OR STEEL BEAMS BEAR ON CONCRETE BLOCK WALLS, BLOCK CELLS SHALL BE FILLED WITH CONCRETE 1'-4" WIDE TO FOUNDATION AND REINFORCED WITH A #5 EACH CELL, UNLESS NOTED OTHERWISE.
 - CONCRETE OR GROUT FOR BLOCK FILL SHALL HAVE 2" INCH MAXIMUM SIZE COARSE AGGREGATE AND SUFFICIENT WATER TO CURE THE CONCRETE WILL FLOW INTO THE BLOCK CELLS WITHOUT CAVITIES. HEIGHT OF LIFT WHEN FILLING CELLS SHALL NOT EXCEED 4'.
 - CONCRETE OR GROUT FILL FOR C.M.U. SHALL HAVE A 20% BY COMPRESSIVE STRENGTH OF $F_c = 5000$ PSI. ON 16" DEEPER U-BLOCKS, FILL CELLS FULL HEIGHT OF LINTEL AT SAME TIME.
 - ANCHOR ALL MASONRY LINTEL AT STEEL COLUMNS WITH STRUT ANCHORS AT 16" O.C. VERTICALLY UNLESS OTHERWISE NOTED.
 - UNLESS INDICATED OTHERWISE PROVIDE MASONRY CONTROL JOINTS AT A MAXIMUM SPACING OF 25'-0". JOINT SHALL BE DISCONTINUOUS AT BOND BEAM.
 - PROVIDE REINFORCING SUPPORTS CENTER VERTICAL REINFORCING IN MASONRY WALLS.
 - PROVIDE 48 DIAMETER SPLICE IN VERTICAL MASONRY REINFORCING. MAPED SPECTRAL ACCELERATION (1-SECOND) $S_{d1} = 0.05$
 - PROVIDE CORNER BARS AND BEAMS AT CORNERS, TYPICAL C.M.U. SHALL BE PLACED IN A BONDING PATTERN UNLESS NOTED OTHERWISE.
 - REINFORCING SHALL BE CONTINUOUS THROUGH BOND BEAMS AND LINTELS (CUT OUT OR NOTCH BOTTOM OF U-BLOCKS AS REQUIRED) - DO NOT SUBSTITUTE BLOCK WITH KNOCK-OUT WEBS WHERE STANDARD U-BLOCK IS REQUIRED TO 1" CLEAR TOP OF BOND BEAM.
- STRUCTURAL STEEL:**
- ALL STRUCTURAL STEEL W AND WT SHAPES SHALL CONFORM TO ASTM A992 (GRADE 50). OTHER SHAPES SHALL CONFORM TO ASTM, A36, LATEST EDITION (EXCEPT STEEL JOISTS AND TUBE SECTIONS).
 - STRUCTURAL STEEL TUBE SECTIONS SHALL CONFORM TO ASTM A500, GRADE B, $F_y = 46.0$ KSI.
 - HEADED STUDS SHALL BE TYPE B SHEAR CONNECTORS ($F_u = 65$ KSI).
 - STEEL FABRICATOR SHALL SUBMIT SHOP DRAWINGS FOR THE ARCHITECT AND/OR ENGINEER'S REVIEW.
 - THE CONTRACTOR SHALL VERIFY ALL SHOP DRAWINGS DIMENSIONS WITH STRUCTURAL AND ARCHITECTURAL PLANS AND DETAILS.
 - BOLTED CONNECTIONS SHALL BE MADE WITH HIGH STRENGTH BOLTS CONFORMING TO ASTM A325. USE 3/4" INCH DIAMETER MINIMUM UNLESS NOTED OTHERWISE. ALL BOLTS SHALL BE TIGHTENED AS FULLY PRETENSIONED BEARING CONNECTIONS.
 - CONNECTIONS NOT SHOWN ON DRAWINGS SHALL BE DESIGNED BY THE FABRICATOR. WHERE POSSIBLE USE DOUBLE ANGLE CONNECTIONS. USE MAXIMUM NUMBER OF BOLTS FOR DEPTH OF BEAM WITH SINGLE ROW OF BOLTS. WHERE DOUBLE ANGLE CONNECTIONS ARE NOT POSSIBLE, FABRICATOR SHALL DESIGN CONNECTION FOR CAPACITY EQUIVALENT TO DBL-ANGLE CONNECTION WITH MAX NO. BOLTS UNLESS DETAILED OTHERWISE.
 - FOR DBL-ANGLE CONNECTIONS, MIN ANGLE THICKNESS SHALL BE 5/16" FOR 3/4" INCH DIAMETER BOLTS AND 3/8" FOR 7/8" INCH DIAMETER BOLTS AND LARGER.
 - UNLESS SHOWN OTHERWISE PROVIDE 1/2" X 7 1/2" X 7 1/2" BEARING PLATES ON 1 INCH GROUT WITH 2-3/4" DIAMETER ANCHOR BOLTS UNDER ALL STEEL BEAMS THAT BEAR ON MASONRY WALLS.
 - OPEN WEB STEEL JOIST SHALL CONFORM TO THE SPECIFICATIONS OF THE AISC AND SH TO THE LATEST OSHA STEEL ERECTION STANDARD.
 - UNLESS SHOWN OTHERWISE PROVIDE BRIDGING, BEARING SEATS, AND STABILIZER PLATES IN ACCORDANCE WITH ABOVE SPECIFICATIONS AND STANDARD.
 - ALL BRIDGING SHALL BE SECURELY ANCHORED AT END OF EACH RUN. WELD TO STEEL BEAM OR ANCHOR TO MASONRY WALL WITH 3/8" ANCHOR BOLTS.
 - WHERE JOISTS CAN NOT BEAR 2 1/2" INCHES ON STEEL BEAMS, STAGGER LOCATION OF JOISTS TO PROVIDE 2 1/2" INCHES MINIMUM BEARING.
 - ROOF JOISTS AND BRIDGING SHALL BE DESIGNED FOR A NET UPLIFT OF 40 PSF.
 - ANY MEMBER CALLED OUT TO BE BENT TO RADIUS SHALL BE FABRICATED OUT OF PLATE WITH EQUIVALENT SECTION PROPERTIES IF BENDING TO RADIUS IS IMPRACTICAL.

- COLD-FORMED STEEL STUDS:**
- PROVIDE COLD FORMED STEEL STUDS WHERE INDICATED ON THE PLAN. ALL SIZES AND GAGES SHOWN SHALL BE CONSIDERED MINIMUM.
 - LIGHT STEEL GALVANIZED (G-80) METAL STUDS, OF SIZES SHOWN COMPLETE WITH ALL ACCESSORIES REQUIRED, 16 GA AND HEAVIER MEMBERS SHALL MEET ASTM A-1003/1004M, GRADE D WITH 50 KSI YIELD.
 - PROVIDE SHOP DRAWINGS PREPARED BY COLD FORMED METAL FRAMING MANUFACTURER. SUBMIT FOR APPROVAL SHOWING PLANS, SECTIONS, ELEVATIONS, LAYOUTS, PROFILES, PRODUCT COMPONENTS, AND INDICATING SPACING OF MEMBERS, PROPOSED METHODS OF FRAMING LINTELS, GUTTER FRAMING, ETC. SHOW CONNECTION DETAILS WITH SOCRW TYPE AND LOCATIONS AND ALL OTHER FASTENER REQUIREMENTS. INCLUDE CATALOG DATA ON ALL PRODUCT MATERIALS.
 - MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS INCLUDING ALL CONNECTIONS. DESIGNS SHALL BE SIGNED BY A REGISTERED PROFESSIONAL ENGINEER, REGISTERED IN THE STATE OF ALABAMA.
 - MINIMUM CONNECTION OF THE TRACK TO THE FOUNDATION SHALL BE (2)-0.157IN DIAMETER X 1-1/2" EMBEDMENT MULTI X-U ANCHORS AT EACH STUD. FOLLOW ALL MANUFACTURERS RECOMMENDED EDGE DISTANCES AND SPACING REQUIREMENTS.
- BUILDING CANOPIES:**
- THE COMPLETE DESIGN OF THE CANOPIES INCLUDING ALL COMPONENTS SHOWN OR NOT SHOWN ON THE DRAWINGS SHALL BE ACCOMPLISHED BY THE CANOPY MANUFACTURER.
 - THE DESIGN SHALL BE MADE BY A REGISTERED ENGINEER, REGISTERED IN THE STATE OF ALABAMA AND HE SHALL AFFIX HIS REGISTRATION NUMBER TO ALL SHOP DRAWINGS AND CALCULATIONS.
- CODES:**
- ALL PARTS SHALL BE FURNISHED AND ERECTED ACCORDING TO APPLICABLE CODES AND SPECIFICATIONS OF THE FOLLOWING:
- AMERICAN CONCRETE INSTITUTE (ACI)
 - AMERICAN INSTITUTE OF STEEL CONSTRUCTORS (AISC)
 - AMERICAN WELDING SOCIETY (AWS)
 - OSHA STEEL ERECTION STANDARD (OSHA)
 - STEEL JOIST INSTITUTE (SJI)
 - INTERNATIONAL BUILDING CODE (IBC)
 - INTERNATIONAL BUILDING CODE (ICC)
- DESIGN LIVE LOADS:**
- ROOF: 20 PSF
CELL BEAMS: 40 PSF
- CATEGORY: IBC 2012/ASCE 7-10, IV
ULTIMATE DESIGN WIND SPEED (V_{ult}): 164 MPH
NOMINAL DESIGN WIND SPEED (V_{nom}): 128 MPH
WIND EXPOSURE: C
INTERNAL PRESSURE COEFFICIENTS: +/- 0.18
- COMPONENTS AND CLADDING ULTIMATE WIND PRESSURES:
NOTE: MULTIPLY ALL VALUES SHOWN BELOW BY 0.8 TO GET ALLOWABLE DESIGN PRESSURES.
- ROOF/TRELLIS AREA A = 10 SF
ZONE 1: -75.0PSF/31.3PSF
ZONE 2: -126.8PSF/31.3PSF
WALL/TRELLIS AREA A = 10 SF
ZONE 4: -74.9PSF/69.2PSF
ZONE 5: -92.2PSF/69.2PSF
CORNER ZONE = 7.5 FT
- SEISMIC: INTERNATIONAL BUILDING CODE (PER ASCE 7-10)
SEISMIC IMPORTANCE FACTOR: I_m=1.5
MAPED SPECTRAL ACCELERATION (SHORT-TERM) S_w=0.12
MAPPED SPECTRAL ACCELERATION (1-SECOND) S_{d1}=0.05
SITE CLASS: D
SHORT-PERIOD SPECTRAL RESPONSE ACCEL: S_{ds}=0.12g
1-SECOND SPECTRAL RESPONSE ACCEL: S_{d1}=0.08g
SEISMIC DESIGN CATEGORY: C
SEISMIC FORCE-RESISTING SYSTEM: ORDINARY
REINFORCED CONCRETE SHEAR WALLS
DESIGN BASE SHEAR (ULTIMATE): 810k
SEISMIC RESPONSE COEFFICIENT: C_s=0.032
RESPONSE MODIFICATION FACTOR: R=5
ANALYSIS PROCEDURE: ASCE 7 (SECT 12.8)
- SNOW: INTERNATIONAL BUILDING CODE
GROUND SNOW LOAD: P_g=5 PSF
- SPECIAL INSPECTIONS**
- ALL SPECIAL INSPECTIONS REQUIRED BY CHAPTER 17 OF THE IBC 2012 SHALL BE PERFORMED BY A DESIGNATED TESTING AGENCY OR AGENCIES RESPONSIBLE FOR SPECIAL INSPECTIONS.
- SEISMIC REQUIREMENTS FOR SPECIAL INSPECTIONS:**
- THE FOLLOWING STRUCTURAL COMPONENTS ARE DESIGNATED AS SEISMIC SYSTEMS AND/OR PART OF THE SEISMIC-FORCE-RESISTING SYSTEM OF THE BUILDING AND ARE SUBJECT TO THE REQUIREMENTS OF SECTIONS 1705.12 AND 1705.13 OF IBC 2012 AND PROJECT SPECIFICATIONS:
SHEAR WALLS (INCL. ANCHORAGE TO FOUNDATION)
THESE SPECIFIC COMPONENTS ARE IN ADDITION TO ALL GENERAL COMPONENTS LISTED IN SECTIONS 1705.12 AND 1705.13 OF IBC 2012 AND ARE SUBJECT TO ALL SPECIAL INSPECTIONS AND TESTING AS REQUIRED BY CHAPTER 17 OF IBC 2012, PROJECT SPECIFICATIONS, AND SCHEDULE OF SPECIAL INSPECTIONS. SPECIAL INSPECTION REPORTS SHALL BE SUBMITTED AS PER THE STATEMENT OF SPECIAL INSPECTIONS.
 - OTHER ARCHITECTURAL, MECHANICAL, OR ELECTRICAL COMPONENTS AND THEIR ANCHORAGES MAY ALSO BE DESIGNATED AS SEISMIC SYSTEMS. SEE OTHER DISCIPLINE'S DRAWINGS AND SPECIFICATIONS.
 - ALL SPECIAL INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE IBC 2012 SHALL BE PERFORMED. REFER TO TABLES SHOWN IN CHAPTER 17 FOR ALL APPLICABLE COMPONENTS THAT REQUIRE SPECIAL INSPECTIONS.
- WIND REQUIREMENTS FOR SPECIAL INSPECTIONS:**
- THE FOLLOWING STRUCTURAL COMPONENTS ARE DESIGNATED AS WIND SYSTEMS AND/OR PART OF THE MAIN WIND-FORCE-RESISTING SYSTEM OF THE BUILDING AND ARE SUBJECT TO THE REQUIREMENTS OF SECTION 1705.11 OF IBC 2012 AND PROJECT SPECIFICATIONS:
ROOF DIAPHRAGM SYSTEM AND ATTACHMENT (SHEAR) WALLS
JOIST CONNECTIONS TO SHEAR WALLS
SHEAR WALL ANCHORAGE TO FOUNDATION
THESE SPECIFIC COMPONENTS ARE IN ADDITION TO ALL GENERAL COMPONENTS LISTED IN SECTION 1705.11 OF IBC 2012 AND ARE SUBJECT TO ALL SPECIAL INSPECTIONS AND TESTING AS REQUIRED BY CHAPTER 17 OF IBC 2012, PROJECT SPECIFICATIONS, AND SCHEDULE OF SPECIAL INSPECTIONS. SPECIAL INSPECTION REPORTS SHALL BE SUBMITTED AS PER THE STATEMENT OF SPECIAL INSPECTIONS.
 - OTHER ARCHITECTURAL COMPONENTS AND THEIR ANCHORAGES MAY ALSO BE DESIGNATED AS WIND-RESISTING COMPONENTS. SEE OTHER DISCIPLINE'S DRAWINGS AND SPECIFICATIONS.
 - ALL SPECIAL INSPECTIONS AS REQUIRED BY CHAPTER 17 OF THE IBC 2012 SHALL BE PERFORMED. REFER TO TABLES SHOWN IN CHAPTER 17 FOR ALL APPLICABLE COMPONENTS THAT REQUIRE SPECIAL INSPECTIONS.

PHJ architects inc. Alabama
Montgomery,



CIP-2017-001
UPGRADES TO THE MOBILE COUNTY METRO JAIL
For The MOBILE COUNTY COMMISSION
MOBILE, ALABAMA



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DATE	APRIL 2, 2019 RTA		
REVISED			
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SHEET TITLE
GENERAL NOTES
AND LINTEL SCHEDULE

JOB NO.
PH&J 1801-GV

SEQUENCE
NO. 79 OF 175

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