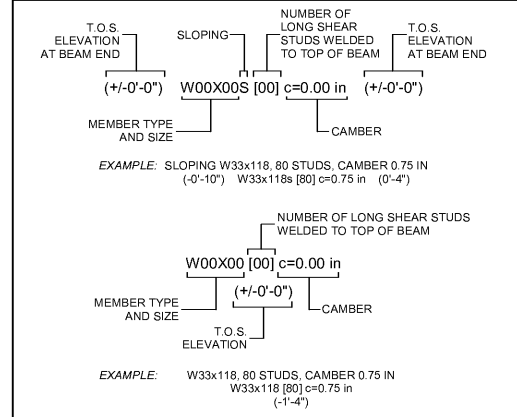


STRUCTURAL STEEL

- DESIGN, FABRICATION AND ERECTION OF THE STRUCTURAL STEEL SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING STANDARDS...
1. DESIGN, FABRICATION AND ERECTION OF THE STRUCTURAL STEEL SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING STANDARDS, EXCEPT WHERE MORE STRINGENT REQUIREMENTS ARE SHOWN:
- AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" INCLUDING THE "COMMENTARY" AND SUPPLEMENTS THERE TO AS ISSUED.
- AISC "SPECIFICATIONS FOR ARCHITECTURALLY EXPOSED STRUCTURAL STEEL"
- AISC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS"
- AMERICAN WELDING SOCIETY (AWS) D1.1 "STRUCTURAL WELDING CODE-STEEL"
- AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"
- ACI 318-BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
2. THE OWNER SHALL ENGAGE A TESTING AND INSPECTION AGENCY TO "TO PERFORM THE SPECIAL INSPECTIONS REQUIRED BY THE SCHEDULE OF SPECIAL INSPECTIONS.
3. ALL CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR'S ENGINEER FOR THE FORCES INDICATED. DESIGN CALCULATIONS SHALL BE SUBMITTED AS PER THE SHOP DRAWING NOTES AND SPECIFICATIONS
4. STEEL MATERIALS SHALL BE PROVIDED IN ACCORDANCE WITH THE FOLLOWING SPECIFICATIONS
- WIDE FLANGE STEEL SHAPES ASTM A-992
- OTHER ROLLED STEEL SHAPES ASTM A-36
- HSS (TUBULAR SHAPES ) ASTM A-500 GRADE B
- STEEL PIPE ASTM A-53, TYPE E OR S, GRADE B
- BOLTS ASTM F3125, GRADE F1552, TYPE 1
- ANCHOR RODS ASTM F1554 GR36
- SHEAR STUDS ASTM A29, TYPE A
5. SEAMS OF HSS SECTIONS SHALL BE MADE WITH CONTINUOUS FULL PENETRATION BUTT WELDS.
6. MEMBERS INDICATED AS AESS STEEL ON THE PLANS SHALL CONFORM TO THE SPECIFICATIONS FOR ARCHITECTURALLY EXPOSED STRUCTURAL STEEL. CONTRACTOR SHALL VERIFY WITH ARCHITECT IF MOCK UPS ARE REQUIRED BEFORE PROCEEDING WITH FABRICATION AND/OR ERECTION.
7. PROVIDE CONTINUOUS MINIMUM SIZED FILLET WELDS PER AISC. FILLER MATERIALS SHALL HAVE A MINIMUM YIELD STRENGTH OF 58 KSI. EXPOSED WELDS SHALL BE GROUND SMOOTH.
8. ALL EXTERIOR EXPOSED STRUCTURAL STEEL SHALL BE GALVANIZED UNLESS NOTED OTHERWISE.
9. STRUCTURAL STEEL SURFACE PREPARATION SHALL CONFORM TO SSPC-SP3. MEMBERS SHALL BE PAINTED WITH A SHOP PRIMER UNLESS NOTED OTHERWISE.
10. SHOP AND FIELD HOLES SHALL BE PUNCHED OR DRILLED.
11. GENERAL CONTRACTOR SHALL VERIFY ALL ROOFTOP EQUIPMENT SIZES, WEIGHTS AND LOCATIONS AND COORDINATE WITH ARCHITECT AND ROOF CONTRACTOR FOR PENETRATION OPENINGS WITH THE STEEL FABRICATOR AND M.E.P. CONTRACTOR.



METAL DECK

- 1. METAL DECK SHALL BE DESIGNED, DETAILED, AND FABRICATED IN ACCORDANCE WITH THE "ROOF DECK DESIGN MANUAL" AND "FLOOR DECK DESIGN MANUAL" OF THE STEEL DECK INSTITUTE, LATEST EDITION. COMPOSITE STEEL FLOOR DECK SHALL BE IN CONFORMANCE WITH THE "STANDARD FOR COMPOSITE STEEL FLOOR DECK" OF THE STEEL DECK INSTITUTE, LATEST EDITION.
2. THE OWNER SHALL ENGAGE A TESTING AND INSPECTION AGENCY TO "TO PERFORM THE SPECIAL INSPECTIONS REQUIRED BY THE SCHEDULE OF SPECIAL INSPECTIONS.
3. INSTALL IN ACCORDANCE WITH SDC SUGGESTED SPECIFICATIONS UNLESS NOTED OTHERWISE ON THE DRAWINGS. INDIVIDUAL DECK SHEETS SHALL BE MINIMUM THREE SPAN CONTINUOUS, WITH LAPS TO BE PLACED OVER SUPPORTS.
4. METAL DECK PROPERTIES ARE BASED ON PRODUCTS MANUFACTURED BY VULCRAFT, INC. DECKS BY OTHER MANUFACTURERS MAY BE SUPPLIED PROVIDED LOAD CARRYING CAPACITY BASED ON MANUFACTURER'S STANDARD LOAD TABLES, DEFLECTION CHARACTERISTICS, AND UL FIRE RATINGS EQUAL TO OR EXCEEDING THOSE OF MATERIALS SPECIFIED AND IF APPROVED BY THE PRIME PROFESSIONAL AND STRUCTURAL ENGINEER.
5. METAL DECK SUPPLIER SHALL PROVIDE ADDITIONAL REINFORCEMENT FOR ALL ADDITIONAL FRAMING, CLOSURE ANGLES AND PLATES, POUR STOPS, SCREENS, ANGLES, AND ROOF SUMP PANS AS REQUIRED AT THE EDGES OF ALL ROOF AND FLOOR OPENINGS AND AT ALL SLAB DEPRESSIONS OR CHANGES OF DECK DIRECTION PER SECTION AND DETAIL DRAWINGS, INCLUDING THOSE WHICH HAVE NOT BEEN DETAILED. COORDINATE SIZE, LOCATION, AND QUANTITY OF OPENINGS WITH M.E.P. CONTRACTOR THROUGH GENERAL CONTRACTOR.
6. SEE DECK ATTACHMENT SCHEDULE FOR DECK ATTACHMENT REQUIREMENTS.
7. NO MECHANICAL OR ELECTRICAL PIPING, FIXTURES, AND UNITS OR SYSTEMS MAY BE HUNG DIRECTLY FROM THE ROOF DECK.
8. METAL DECK SHALL BE DESIGNED AND INSTALLED TO RESIST UPLIFT LOADS AS INDICATED WITHIN THE DESIGN LOADS.
9. METAL DECK UNITS Z2 GAGE AND LIGHTER SHALL BE WELDED TO THE SUPPORTING STEEL FRAMING WITH WELDING WASHERS, AS REQUIRED AND SPECIFIED BY THE DECK MANUFACTURER. WASHERS SHALL BE SPACED IN ACCORDANCE WITH THE METAL DECK MANUFACTURER'S RECOMMENDATIONS. ALL WELDS AND BURN AREAS SHALL BE CLEANED AND PAINTED WITH AN APPROVED PRIMER.
10. PRIOR TO AND DURING CONCRETE PLACEMENT, THE FLOOR DECK SHALL BE PLANKED TO PREVENT DAMAGE TO THE DECK. CONCENTRATED AND IMPACT LOADS SHALL BE AVOIDED.
11. STEEL DECK SUPPLIER SHALL SUBMIT SHOP DRAWINGS INDICATING THE SHEAR STUD PLACEMENT.
12. SHEAR CONNECTORS SHALL BE HEADED STUDS CONFORMING TO ASTM A108, GRADES 1010, 1015, 1017, OR 1020. SHEAR CONNECTORS SHALL BE MACHINE WELDED TO STEEL.
13. SHEAR CONNECTORS SHALL BE EQUALLY SPACED OVER THE LENGTH OF THE BEAM UNLESS NOTED OTHERWISE. WHERE THE NUMBER OF SHEAR CONNECTORS CORRUPTIONS AVAILABLE IS LESS THAN THE NUMBER OF SHEAR CONNECTORS REQUIRED, USE PAIRS OF SHEAR CONNECTORS STARTING FROM EACH END OF THE BEAM AND CONTINUING TOWARD THE CENTER UNTIL IT IS POSSIBLE TO RETURN TO A SINGLE SHEAR CONNECTOR IN EACH CORRUPTION.
14. PROVIDE FLEXIBLE RUBBER CLOSURES BETWEEN FLUTES OF METAL DECKING AND MASONRY FIRE WALLS.

Abbreviation List

Table with 4 columns: Abbreviation, Description, Abbreviation, Description. Includes terms like A.F.F., B.C.X., B.O.F., B.S., CANT., C.J., C.L., C.M.U., CONT.

WOOD

- 1. DESIGN OF ALL WOOD FRAMING SHALL BE BASED UPON ALLOWABLE STRESS DESIGN, IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, LATEST EDITION.
2. FABRICATION, ERECTION, AND BRACING SHALL BE IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, LATEST EDITION.
3. ALL LUMBER SHALL HAVE GRADE IDENTIFIED ON THE LABEL OF AN APPROVED LUMBER GRADING AGENCY WITH DOC P520 OR EQUIVALENT.
4. ALL STEEL TIMBER FASTENINGS AND JOIST HANGERS SHALL BE A MINIMUM OF 16 GA. GALVANIZED STEEL WITH A RATED LOAD CAPACITY EQUAL TO OR EXCEEDING THE IMPOSED LOADING REQUIREMENTS.
5. ALL WOOD MEMBERS IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED LUMBER UNLESS NOTED OTHERWISE.
6. ANCHOR ALL SILL PLATES WITH 1/2" ANCHOR BOLTS WITH 3" HOOK AND MIN. 7" EMBEDMENT SPACED AT 48" o.c. OR 1/2" SIMPSON TITEN HD WITH MIN. 5" EMBEDMENT SPACED AT 48" o.c. OR EQUAL.
7. PROVIDE SOLID BRIDGING OR A CONTINUOUS HEADER AT THE BEARING OF ROOF OR FLOOR JOISTS ON SILL PLATES.
8. PROVIDE DOUBLE JOISTS ON EACH SIDE OF ANY ROOF OR FLOOR JOIST WHICH IS INTERRUPTED BY AND OPENING OR OTHER CONSIDERATIONS, UNLESS NOTED OTHERWISE. NO MORE THAN ONE ROOF OR FLOOR JOIST SHALL BE INTERRUPTED WITHOUT CONSULTING THE ENGINEER.
9. ALL DOUBLE JOISTS SHALL BE SPIKED TOGETHER WITH 2 ROWS OF 10d NAILS @ 16" o.c.
10. ALL WOOD BEAMS MADE UP OF 3 OR MORE MEMBERS SHALL BE BOLTED TOGETHER WITH 1/2" BOLTS @ 16" o.c. STAGGERED TOP & BOTTOM.
11. PROVIDE A MINIMUM OF TWO (2) SUPPORT ALL WOOD HEADERS, POST DEPTH SHALL MATCH WALL CONSTRUCTION.
12. WOOD MEMBERS SHALL BE DESIGNED TO ACCOMMODATE AND SUPPORT MECHANICAL UNITS AS SHOWN ON THE MECHANICAL DRAWINGS.
13. WOOD STRUCTURAL PANELS SHALL CONFORM TO THE REQUIREMENTS FOR ITS TYPE AND DOC P520 OR EQUIVALENT.
14. WOOD SHEATHING SHALL MEET THE REQUIREMENTS OF THE PLYWOOD DESIGN SPECIFICATION PUBLISHED BY THE APA, LATEST EDITION.
15. ALL WOOD SHEATHING SHALL BE APA RATED.
16. ALL FLOOR AND ROOF SHEATHING SHALL BE CONTINUOUS OVER A MINIMUM OF 3" MINIMUM.
17. WOOD TRUSS JOISTS AND GULLAM BEAMS ARE TO BE DESIGNED BY THE MANUFACTURER TO THE LOADS AND DEFLECTIONS SPECIFIED. DESIGN CALCULATIONS SHALL BE SUBMITTED AS PER THE SHOP DRAWING NOTES AND SPECIFICATIONS.
18. ALL HOLES IN WOOD JOISTS SHOULD BE COORDINATED WITH MECHANICAL AND VERIFIED TO ME ACCEPTABLE BY MANUFACTURER DOCUMENTATION.
19. MINIMUM WOOD FASTENINGS SHALL CONFORM TO IBC TABLE 2304.9.1 UNLESS NOTED OTHERWISE.
20. MINIMUM WOOD MEMBER DESIGN VALUES SHOWN IN TABLE BELOW:

MINIMUM WOOD DESIGN VALUES (PSI)

Table with 6 columns: Species, Fb, Fc, Fc parallel, Fv, E, Moisture Content. Rows include Sawm Lumber (2"-4" thick) and Structural Composite Lumber.

CONCRETE MASONRY

- 1. ALL MATERIALS AND CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF TMS 402/ACI 530/ASCE 5, BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES LATEST EDITION.
2. THE OWNER SHALL ENGAGE A TESTING AND INSPECTION AGENCY TO "TO PERFORM THE SPECIAL INSPECTIONS REQUIRED BY THE SCHEDULE OF SPECIAL INSPECTIONS.
3. HOLLOW MASONRY UNITS SHALL BE GRADE N-1 CONFORMING TO ASTM C-90, UNLESS NOTED OTHERWISE WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI (MASONRY ASSEMBLY Fm = 2,000 PSI).
4. ALL FILL FOR MASONRY WALLS SHALL BE PEA GRAVEL CONCRETE OR GROUT CONFORMING TO ASTM C476 WITH A MINIMUM GROUT STRENGTH OF 3,000 PSI AT 28 DAYS. FILL SHALL BE PLACED IN 5'-0" LIFTS IN ACCORDANCE TO INTERNATIONAL MASONRY INSTITUTE GUIDELINES AND ACI 530.1 / ASCE 6 FOR LOW LIFT GROUTING.
5. ALL MORTAR SHALL BE TYPE "M" CONFORMING TO ASTM C270 WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI AT 28 DAYS.
6. ALL MORTAR FOR MASONRY OTHER THAN VENEER SHALL BE PORTLAND CEMENT LIME MORTAR, NOT MASONRY CEMENT.
7. VERTICAL WALL REINFORCING SHALL BE INTERMEDIATE GRADE DEFORMED BARS OF NEW BILLET STEEL CONFORMING TO ASTM A-615, GRADE 60.
8. JOINT REINFORCING SHALL BE 0.149" RODS MIN. CONFORMING TO ASTM A-361.
9. ALL MORTAR JOINTS IN MASONRY WALLS (HORIZONTAL AND VERTICAL) SHALL BE FULL MORTAR BEDDING.
10. ALL SECTIONS OF BEARING WALLS HAVING A HORIZONTAL PLAN DIMENSION OF 2'-0" OR LESS SHALL BE ALL OF SOLID CONCRETE FILLED) MASONRY CONSTRUCTION FOR THE FULL HEIGHT OF THE WALL SECTION.
11. PROVIDE A MINIMUM OF ONE COURSE OF SOLID BLOCK UNDER ALL JOIST AND SLAB BEARING ENDS FOR THE FULL WIDTH OF THE WALL, UNLESS NOTED OTHERWISE.
12. BRACE AND SHORE ALL NEW MASONRY WALLS AS REQUIRED UNTIL ROOF AND FLOOR DECKS HAVE BEEN COMPLETELY INSTALLED.
13. THE TOP OF ALL MASONRY WALLS SHALL BE BRACED TO THE STEEL FRAMING. SEE TYPICAL DETAILS ON FRAMING SECTIONS AND DETAIL DRAWINGS.
14. BACKFILL AGAINST MASONRY WALLS SHALL BE PLACED AT EQUAL HEIGHTS ON EACH SIDE OF WALL UP TO FINAL GRADE.
15. QUALITY ASSURANCE PROGRAM TO BE PROVIDED PER SECTION 3.1 OF TMS 402/ACI 530.
16. FILL ALL BLOCK CORES SOLID FROM FOUNDATION TO SLAB ON GRADE.
17. BOND BEAMS WITH HORIZONTAL REINFORCEMENT NOT LESS THAN (2) #4 REBAR SHALL BE PROVIDED CONTINUOUSLY AT STRUCTURALLY CONNECTED JOINTS AND FLOOR LEVELS AT THE TOP OF WALLS, AT THE BOTTOM OF WALLS AT THE TOP OF THE FOUNDATION WHEN DOWELING INTO THE WALL.
18. ALL MASONRY WORK TO BE EXECUTED IN HOT OR COLD WEATHER SHALL BE IN CONFORMANCE WITH THE RECOMMENDATIONS FOR COLD WEATHER CONSTRUCTION FOUND IN TMS 402/ACI 530.1. THE FOLLOWING ADJUSTMENTS FOR ALL CONDITIONS WHEN THE TEMPERATURE OF THE MASONRY OR MORTAR IS BELOW 40°F SHALL BE MAINTAINED ABOVE:
- DEGREE F FOR MINIMUM GROUT TEMPERATURE SHALL BE MAINTAINED ABOVE 40°F.
- THE METHODS DESCRIBED IN TMS 402/ACI 530.
19. UNLESS NOTED OTHERWISE ON THE DRAWINGS OR THE SPECIFICATIONS, THE FOLLOWING MINIMUM REINFORCING SHALL BE USED:
- ALL CORES WITH REINFORCING SHALL BE GROUTED SOLID.

MINIMUM WALL REINFORCING

Table with 2 columns: LOCATION, REINFORCING. Rows include Interior Steel Beam, Interior Steel of Concrete Linets, Interior Solid Bearing Walls, Vertical Control Joints, Top of Wall, Bottom and Top of EA Opening.

GENERAL NOTE

REFER ALSO TO APPLICABLE SECTION OF SPECIFICATIONS BOUND IN THE PROJECT MANUAL. IN THE EVENT OF INCONSISTENCY BETWEEN NOTES ON THIS DRAWING AND SPECIFICATIONS IN THE PROJECT MANUAL, THE ITEM SHALL BE FURNISHED WHICH, IN THE OPINION OF THE ARCHITECT, IN ACCORDANCE WITH 4.2.12, BEST ADVANCES THE INTENT OF THE WORK. IN THE EVENT OF CONFLICTS OR DISCREPANCIES, THE PRICE OF THE WORK SHALL BE BASED ON USE OF ITEMS OF HIGHER QUALITY OR COST.

CONCRETE

- 1. ALL WORK SHALL CONFORM TO THE "SPECIFICATIONS FOR STRUCTURAL CONCRETE" (AMERICAN CONCRETE INSTITUTE, ACI 301) AND THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318), LATEST EDITIONS, UNLESS NOTED OTHERWISE IN THE DRAWINGS OR PROJECT SPECIFICATIONS.
2. CONSTRUCTION TOLERANCES SHALL CONFORM TO THE "SPECIFICATION FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS" (ACI 117), LATEST EDITION.
3. SHOP DRAWINGS SHALL BE PREPARED IN ACCORDANCE WITH THE "GUIDE TO PRESENTING REINFORCING STEEL DESIGN DETAILS" (ACI 315) AND THE "ACI DETAILING MANUAL" (SP-066), LATEST EDITIONS.
4. CONCRETE MATERIALS SHALL BE PROVIDED IN ACCORDANCE WITH THE FOLLOWING SPECIFICATIONS:
- PORTLAND CEMENT ASTM C150 NOT ALLOWED
- HYDRAULIC CEMENT ASTM C618
- FLY ASH AND NATURAL POZZOLAN ASTM C618 NOT ALLOWED
- SILICA FUME NOT ALLOWED
- NORMALWEIGHT AGGREGATE ASTM C33
- LIGHTWEIGHT AGGREGATE ASTM C930
- MIXING WATER ASTM C1602
- ADMIXTURES
- WATER REDUCTION AND SETTING TIME MODIFICATION ASTM C494
- PRODUCING FLOWING CONCRETE ASTM C107
- AIR ENTRAINMENT ASTM C260
- INHIBITING CHLORIDE-INDUCED CORROSION ASTM C1582
5. REINFORCING MATERIALS:
- REINFORCING BARS PLAIN-STEEL ASTM A615, GRADE 60, DEFORMED
- REINFORCING BARS LOW-ALLOY-STEEL ASTM A706, DEFORMED (WELDABLE)
- REINFORCING BARS EPOXY-COATED ASTM A775
- WELDED WIRE FABRIC ASTM A1064
- EPOXY-COATED WELDED WIRE FABRIC ASTM A884, CLASS A COATING
6. REINFORCING SHALL COMPLY WITH ACI FOR FABRICATING, PLACING, AND SUPPORTING REINFORCEMENT, AND THE FOLLOWING:
- BAR SPLICE LOCATIONS SHALL CONFORM TO ACI 318
- HORIZONTAL REINFORCING BARS IN WALLS AND FOOTINGS SHALL BE CONTINUOUS AROUND CORNERS
- UNLESS DETAILED CALCULATIONS ARE PROVIDED, REINFORCING BARS DEVELOPMENT LENGTHS, EMBEDMENT DEPTHS AND LAP SPLICE DISTANCES FOR Fc = 4000psi, NORMAL-WEIGHT CONCRETE, SHALL BE AT LEAST:
7. EMBEDDED AND MISCELLANEOUS STEEL SHALL COMPLY WITH ASTM A36.
8. CONTINUOUS WALL OR FOOTING PLACEMENT SHALL NOT EXCEED 60 FEET BETWEEN CONSTRUCTION JOINTS.
9. ALL EXPOSED CONCRETE CORNERS SHALL BE CHAMFERED 3/4" AT 45 DEGREES UNLESS NOTED OTHERWISE.
10. PROVIDE CONSTRUCTION AND SLABS ON GRADE ARE NOT DESIGNED FOR OR INTENDED TO BE EXPOSED TO FREEZE/THAW CYCLING OR RESIST FROST HEAVE FORCES. GENERAL CONTRACTOR TO COORDINATE CONSTRUCTION AND PROVIDE INTERIOR FOOTINGS BELOW FROST PENETRATION AS REQUIRED FOR WINTER CONSTRUCTION.
11. PROVIDE CONSTRUCTION CONTROL JOINT INSTALLATION PLAN AND PLACEMENT SEQUENCE PLAN SUBMITTALS FOR REVIEW OF STRUCTURAL ENGINEER OF RECORD PRIOR TO PLACING CONCRETE.
12. PROVIDE WALL AND SLAB THRU-SLEEVE PLAN SUBMITTALS FOR REVIEW OF STRUCTURAL ENGINEER OF RECORD PRIOR TO PLACING CONCRETE.
13. CORE-DRILLING CONCRETE WALLS, SLABS, BEAMS OR COLUMNS WITHOUT REINFORCING LOCATION SCANS AND PRIOR APPROVAL OF STRUCTURAL ENGINEER OF RECORD SHALL NOT BE PERMITTED.
14. CONDUITS AND PIPES SHALL NOT BE EMBEDDED IN ELEVATED CONCRETE SLABS SUPPORTED BY METAL DECK OR CONCRETE SLABS ON GRADE.
15. FIBER REINFORCED CONCRETE SHALL COMPLY WITH "GUIDE TO DESIGN WITH FIBER-REINFORCED CONCRETE" (ACI 544.4), LATEST EDITION. DESIGN CALCULATIONS SHALL BE SUBMITTED AS PER THE SHOP DRAWING NOTES AND SPECIFICATIONS.
16. NON-SHRINK GROUT SHALL CONFORM TO ASTM... AND ACHIEVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 3,000 PSI.
17. STRUCTURAL ADHESIVES SHALL ONLY BE USED WITH PRIOR PERMISSION OF THE STRUCTURAL ENGINEER OF RECORD OR AS SHOWN ON CONSTRUCTION DOCUMENTS. THE BASIS OF DESIGN FOR ADHESIVES IS HY-207 "HILT" OR SET BY "SIMPSON" - THE USE OF ANY OTHER STRUCTURAL ADHESIVES SHALL BE SUBJECT TO REVIEW BY THE EOR PRIOR TO INSTALLATION.
18. MINIMUM WIRE WELDED FABRIC REQUIREMENTS FOR SLABS:
- 3-1/2" FLOOR SLAB ON METAL DECK 6x6 W2x2
- 4" FLOOR SLAB ON METAL DECK 6x6 W2x2
- 5" FLOOR SLAB ON GRADE 4x4 W4x4
19. COLD-WEATHER CONCRETE PLACEMENT SHALL COMPLY WITH THE PROVISIONS OF ACI 306R, THE USE OF CALCIUM CHLORIDE, AND OTHER MATERIALS CONTAINING ANTI-FREEZE AGENTS SHALL BE LIMITED TO ACCELERATORS SHALL NOT BE PERMITTED UNLESS APPROVED BY THE ARCHITECT.
20. HOT-WEATHER CONCRETE PLACEMENT SHALL COMPLY WITH THE PROVISIONS OF ACI 305.
21. MINIMUM CONCRETE CLEAR COVER SHALL BE PROVIDED FOR REINFORCEMENT PER IBC AND THE FOLLOWING:
- CONCRETE AGAINST EARTH & PERMANENTLY COVERED 3"
- CONCRETE EXPOSED TO EARTH OR WEATHER #6 & LARGER 1-1/2"
- CONCRETE AGAINST WEATHER OR GROUND #4 & LARGER 3/4"
- BEAMS AND COLUMNS 1-1/2"

REBAR LAP SPLICE LENGTHS

Table with 3 columns: BAR LOCATION, BAR SIZE, and Splice Lengths (l, ls, ldh, ls, ldh, ls, ldh). Rows include VERT. & HORIZ. WALL BARS, FOOTING TOP BARS, FOOTING BOTTOM BARS.

NOTE: VALUES ABOVE ARE FOR NORMAL WEIGHT CONCRETE ONLY.

CONCRETE MIX DESIGN CRITERIA

Table with 6 columns: EXPOSURE CLASS, MAX. w/c RATIO, MIN. f.c., AIR CONTENT, CEMENT TYPE. Rows include Footings and Foundation Walls, Interior Slabs on Grade, Interior Elevated Slabs, Exterior Slabs.

CONTRACTOR NOTE

CONCRETE WALL SHALL BE IN PLACE A MINIMUM OF 14 DAYS BEFORE BACKFILLING. AT CONTRACTOR'S OPTION, BACKFILL MAY BE PLACED AGAINST WALL THAT HAS BEEN TEMPORARILY SHORED. HOWEVER, CONTRACTOR SHALL BE RESPONSIBLE FOR CORRECTING ANY MOVEMENT OF WALL DUE TO BACKFILLING.

DESIGN LOADS:

Table with 2 columns: GOVERNING DESIGN CODE, BUILDING RISK CATEGORY. Values include 2018 NORTH CAROLINA BUILDING CODE, ASCE 7-10, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, II.

Table with 2 columns: DESIGN FLOOR LIVE LOADS, Values include Private Rooms and Corridors Serving Them (40 PSF), Public Rooms and Corridors Serving Them (100 PSF), Lobbies (100 PSF), Partitions (Applied to Occupancies Less Than 80 PSF) (15 PSF), Mechanical Unit Loads (See Mech DWGS), Stairs and Exits (100 PSF), Handrails, Guards and Grab Bars (Per 1607.8).

Table with 2 columns: DESIGN ROOF LIVE LOADS, Roof Live Load (20 PSF).

Table with 2 columns: DESIGN ROOF SNOW LOADS, Values include Ground Snow Load (Pg) (15 PSF), Flat Roof Snow Load (Pf) (15 PSF), Snow Exposure Factor (Ce) (1.0), Snow Load Importance Factor (Is) (1.00), Thermal Factor (Ct) (1.0), Minimum Uniform Design Snow Load (Not Reducible) (15 PSF), Snow Drift (See Diagram), Unbalanced Roof Snow Loads (N/A), Roof Slope Factor (Cs) (N/A), Sloped Roof Snow Load (N/A), Windward Snow Load (N/A), Leeward Snow Load Extending X'-X" From Ridge (N/A), Leeward Snow Load X'-X" From Ridge Extending X'-X" From Ridge (N/A).

Table with 2 columns: DESIGN WIND LOAD PARAMETERS, Values include Ultimate Design Wind Speed (V) (120 MPH), Wind Exposure Category - Plan North-South (B), Wind Exposure Category - Plan West-East (B), Internal Pressure Coefficient (+/- 0.18).

Table with 3 columns: WALL COMPONENTS & CLADDING SURFACE PRESSURES, h ≤ 60ft, Wind Zone, Elevation, Pressures. Rows include Negative Zone 4, Negative Zone 5, Positive Zone 4 & 5.

- NOTE:
- 'A' IS THE CONTRIBUTORY AREA TO COMPONENT.
- LINEAR INTERPOLATION IS PERMITTED FOR AREAS BETWEEN 10 SF AND 500 SF.
- SEE WIND LOAD UPLIFT DIAGRAM FOR WIND ZONE LOCATIONS.

Table with 3 columns: WALL COMPONENTS & CLADDING SURFACE PRESSURES, h ≤ 60ft, Wind Zone, Elevation, Pressures. Rows include Negative Zone 1, Negative Zone 2, Negative Zone 3, Positive Zone 1, 2 & 3, Overhang Zone 2, Overhang Zone 3.

- NOTE:
- 'A' IS THE CONTRIBUTORY AREA TO COMPONENT.
- LINEAR INTERPOLATION IS PERMITTED FOR AREAS BETWEEN 10 SF AND 100 SF.
- SEE WIND LOAD UPLIFT DIAGRAM FOR WIND ZONE LOCATIONS.

SHOP DRAWINGS

- 1. DUPLICATION/PHOTOCOPYING OF THESE STRUCTURAL DRAWINGS SHALL NOT BE PERMITTED FOR SHOP DRAWINGS. THIS INCLUDES PARTIAL AND/OR COMPLETE DUPLICATION OF PLAN SHEETS AS BACKGROUNDS FOR SHOP DRAWINGS. ALL SECTIONS INCLUDED IN THESE CONTRACT DRAWINGS MAY NOT BE DUPLICATED/PHOTOCOPIED ON ANY SHOP DRAWINGS.
2. ORIGINAL SHOP DRAWINGS SHALL BE SUBMITTED FOR THE ARCHITECT/ENGINEER REVIEW FOR THE FOLLOWING ITEMS AND AS PER THE SPECIFICATIONS:
a. SLAB ON GRADE CONTROL JOINT LOCATION
b. CONCRETE MIX DESIGNS
c. FOUNDATIONS
d. RETAINING WALLS
e. ELEVATED SLABS
f. SLAB ON GRADE LISTING FIBER MESH REINFORCEMENT & DOSAGE RATE
d. MASONRY CERTIFICATIONS AND GROUT DESIGN
e. STEEL JOIST AND DECKING
f. STRUCTURAL STEEL
g. LIGHT GAGE METAL FRAMING
h. PRECAST CONCRETE
3. CONTRACTOR SHALL SUBMIT, FOR REVIEW, DRAWINGS AND CALCULATIONS FOR ALL PERFORMANCE ASSEMBLIES IDENTIFIED IN THE GENERAL NOTES AND SPECIFIC MATERIAL NOTES AS WELL AS LISTED BELOW. THE DESIGN OF THESE ASSEMBLIES IS THE RESPONSIBILITY OF THE CONTRACTOR'S ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION. ALL SUBMITTALS SHALL BEAR THE ENGINEER'S SEAL AND SIGNATURE. REVIEW SHALL BE FOR GENERAL CONFORMANCE WITH THE PROJECT REQUIREMENTS AS INDICATED ON THE DRAWINGS AND IN THE GENERAL NOTES.
a. METAL STAIRS AND METAL RAILINGS: DESIGNS SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL LOADS REQUIRED BY APPLICABLE BUILDING CODES. WHERE HEADERS OR OTHER TYPES OF STRUCTURAL MEMBERS HAVE BEEN DESIGNATED BY THE STRUCTURAL ENGINEER OF RECORD TO SUPPORT THE STAIRS, THE CONNECTIONS FROM THE STAIRS SHALL BE DESIGNED SO THAT NO ECCENTRIC OR TORSIONAL FORCES ARE INDUCED IN THESE STRUCTURAL MEMBERS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR FURNISHING AND INSTALLING HARDWARE AS REQUIRED BY THE STAIR DESIGN.

Table with 2 columns: DESIGN DEAD LOADS - NOT INCLUDING STRUCTURE SELF WEIGHT, Values include Roof (10 PSF), Floor (20 PSF).

Table with 2 columns: SEISMIC DESIGN DATA, Values include Seismic Importance Factor (Ie) (1.00), Mapped Spectral Acceleration (Ss) (0.136), Mapped Spectral Acceleration (S1) (0.068), Site Class (E), Design Spectral Response Coeff. (Sds) (0.226), Design Spectral Response Coeff. (Sd1) (E), Seismic Design Category (E).

Table with 2 columns: ANALYSIS PROCEDURE, PLAN NORTH-SOUTH DIRECTION, Values include Seismic Force Resisting System (Light Framed Walls Sheathed in Wood Structural Panels), Design Base Shear (V) (3.0 K), Response Modification Coeff. (R) (3.0), Seismic Response Coeff. (Cs) (0.348), Seismic Force Resisting System (Light Framed Walls Sheathed in Wood Structural Panels), Design Base Shear (V) (73.0 KIPS), Seismic Response Modification Coeff. (R) (6.5), Seismic Response Coeff. (Cs) (0.0348).

ADDITIONAL NOTES:

- a. THE CONTRACTOR IS CAUTIONED AS TO NOT STORE ANY CONSTRUCTION MATERIALS OR UNDERTAKE ANY CONSTRUCTION OPERATION THAT EXCEEDS THE DESIGN LOAD CAPACITIES NOTED.
b. THE STRUCTURE HAS BEEN DESIGNED FOR THE DEAD AND LIVE LOADS INDICATED ABOVE, ANY INCREASE OF LOADS DUE TO CHANGE IN USAGE OR CONSTRUCTION MATERIALS, ETC. SHALL HAVE THE WRITTEN APPROVAL OF THE ENGINEER.
c. THE STABILITY OF THE STRUCTURE IS DEPENDENT UPON THE DIAPHRAGM ACTION OF THE ROOFS. THE CONTRACTOR IS COMPLETELY RESPONSIBLE FOR THE METHODS OF CONSTRUCTION AND SHALL PROVIDE ALL GUYS, BRACING AND SHORING REQUIRED TO ACCOMMODATE ALL INTERIM LOADING CONDITIONS THROUGHOUT THE CONSTRUCTION PHASE.
d. WEIGHT OF EQUIPMENT SHOWN ON THE STRUCTURAL DRAWINGS HAVE BEEN CONSIDERED IN THE DESIGN OF THE FRAMING. ANY ADDITIONAL EQUIPMENT NOT SHOWN ON THE STRUCTURAL DRAWINGS AND EXCEEDING 300 POUNDS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER FOR APPROVAL PRIOR TO INSTALLATION.

FOUNDATIONS AND BACKFILL

- 1. SOIL BEARING CAPACITY OF 2,000 PSF WAS UTILIZED IN THE DESIGN OF THE FOUNDATION SYSTEM.
2. SLAB ON GRADE DESIGN BASED ON MODULUS OF SUBGRADE REACTION OF 100 PCF.
3. PLACE EXTERIOR FOOTINGS AND FROST WALLS EXPOSED TO WEATHER AT LEAST 12" BELOW THE ADJACENT OUTSIDE FINISHED GRADE TO PROTECT FROM FROST DAMAGE.
4. THE OWNER SHALL ENGAGE THE SERVICES OF A REGISTERED PROFESSIONAL GEOTECHNICAL ENGINEER TO MONITOR AND INSPECT ALL EARTH WORK AND TO PERFORM THE SPECIAL INSPECTIONS REQUIRED BY THE SCHEDULE OF SPECIAL INSPECTIONS. THE GEOTECHNICAL ENGINEER SHALL SUPERVISE THE PLACING OF THE COMPACTED FILL AND ALL THE MATERIALS AND EQUIPMENT USED FOR THIS PURPOSE. THE CONTRACTOR SHALL CERTIFY BY SEAL AND SIGNATURE, THAT THE EARTH-WORK IS IN ACCORDANCE WITH THE GEOTECHNICAL REPORT, PLANS AND SPECIFICATIONS.
5. THE SUB-FLOOR MATERIALS SHALL CONSIST OF AT LEAST FOUR TO SIX INCHES OF GRAVEL OR CRUSHED STONE. THE SUB-FLOOR MATERIALS SHALL BE COMPACTED BY AT AT LEAST FOUR COURSES OF EXCAVATION FOR VIBRATORY ROLLER OR UNTIL NO FURTHER COMPACTION IS OBSERVED. SEE THE DRAWINGS AND SPECIFICATIONS FOR THE VAPOR BARRIER SIZE, TYPE AND LOCATION.
6. PRIOR TO CONCRETE PLACEMENT THE FOUNDATION BOTTOM SHALL BE COMPACTED USING A WALK BEHIND VIBRATORY ROLLER OR GAS-POWERED TAMPER.
7. IF SOIL OF THE BEARING CAPACITY NOTED ABOVE IS NOT ENCOUNTERED AT THE ELEVATIONS SHOWN ON PLAN, UNSUITABLE SOILS SHALL BE REMOVED AND REPLACED WITH STRUCTURAL FILL IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEER'S DIRECTION.
8. IF BEDROCK IS ENCOUNTERED AT OR ABOVE FOUNDATION SUBGRADE ELEVATION, THE BEDROCK SURFACE SHALL BE OVER EXCAVATED A MINIMUM OF SIX INCHES AND BACKFILLED WITH CRUSHED STONE TO THE SUBGRADE ELEVATION.
9. THE GEOTECHNICAL ENGINEER SHALL APPROVE SAMPLES OF ALL CONTRACTOR PROPOSED COMPACTED FILL MATERIAL. IMPORTED STRUCTURAL FILL SHALL BE FREE OF ORGANIC MATTER, ASH, CINDERS AND DEMOLITION DEBRIS.
10. CONTROL OF MOISTURE FOR PLACING FILL WILL BE BASED ON THE RESULTS OF ASTM D-1557 MODIFIED PROCTOR TESTS.
11. ALL COMPACTED FILL SHALL HAVE A DENSITY OF AT LEAST 98% OF MAXIMUM DRY DENSITY AS DETERMINED BY THE ASTM-1557 MODIFIED PROCTOR TEST. PLACING OF FILL WITH MOISTURE CONTENT OUTSIDE THE LIMITS FOR PROPER COMPACTION SHALL NOT BE PERMITTED.
12. THE EXISTING SUBGRADE IN ALL FILL AND CUT AREAS SHALL BE COMPACTED TO A FIRM, STABLE CONDITION AS DETERMINED BY THE GEOTECHNICAL ENGINEER. PRIOR TO PLACEMENT OF FILL AND CONSTRUCTION OF EXTERIOR FOOTINGS.
13. FILL MATERIAL SHALL BE PLACED IN LOOSE LIFTS NOT EXCEEDING 8" IN THICKNESS AND SHALL BE MIXED, SPREAD, AND PLACED IN SUCH A WAY AS TO PRODUCE A UNIFORM THICKNESS OF MATERIAL AFTER PLACING.
14. EACH LAYER OF FILL MATERIAL SHALL BE COMPACTED ON ALL PORTIONS OF THE SURFACE OF EACH LIFT OF FILL BY RUBBER TIRE ROLLERS, SHEEPS' FOOT ROLLERS, VIBRATORY ROLLERS, ETC. AS NECESSARY TO ATTAIN THE MAXIMUM DRY DENSITY NOTED ABOVE AND AS APPROVED BY THE GEOTECHNICAL ENGINEER.
15. WHENEVER IN PLACE DENSITIES ARE FOUND TO BE BELOW LIMITS ACCEPTABLE TO THE GEOTECHNICAL ENGINEER, ADDITIONAL ROLLING TO PRODUCE THE SPECIFIED DENSITIES SHALL BE REQUIRED.
16. PLACING OF FILL WHEN FREE WATER IS STANDING ON THE EXISTING SURFACE SHALL NOT BE PERMITTED.
17. PLACING OF FILL IN A FROZEN CONDITION OR ON TOP OF FROZEN MATERIAL SHALL NOT BE PERMITTED.
18. SUBGRADE SUITABILITY SHALL BE VERIFIED BY GEOTECHNICAL ENGINEER WITHIN 24 HOURS PRIOR TO PLACING CONCRETE FOUNDATIONS AND CONCRETE SLABS ON GRADE TO CONFIRM THAT NO DETRIORATION HAS OCCURRED SUBSEQUENT TO TESTING PERFORMED AT TIME OF FILL PLACEMENT. UNSUITABLE CONDITIONS SHALL BE CORRECTED PER GEOTECHNICAL ENGINEER'S RECOMMENDATION.
19. EXCAVATIONS SHOULD BE BACKFILLED OR HAVE CONCRETE PLACED AS SOON AS POSSIBLE DURING CONSTRUCTION TO MINIMIZE ADVERSE EFFECTS TO SUBGRADE CONDITIONS. GRADING DEPRESSIONS SHOULD BE AVOIDED.
20. WATER SHALL BE PREVENTED FROM ENTERING OPEN EXCAVATIONS. ANY MATERIAL NOTES AS WELL AS LISTED BELOW. THE DESIGN OF THESE ASSEMBLIES IS THE RESPONSIBILITY OF THE CONTRACTOR'S ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION. REVIEW SHALL BE FOR GENERAL CONFORMANCE WITH THE PROJECT REQUIREMENTS AS INDICATED ON THE DRAWINGS AND IN THE GENERAL NOTES.
21. THE CONTRACTOR SHALL PROVIDE FOR DEWATERING AS REQUIRED DURING EXCAVATION AND CONSTRUCTION.

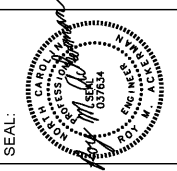
CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS FOR THE ENTIRE PROJECT BEFORE PROCEEDING WITH THE WORK.



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SEAL: BARRY J. ISSETT, PROFESSIONAL ENGINEER, STATE OF NORTH CAROLINA, LICENSE NO. 1003218000

TYPICAL NOTES: HOLIDAY INN EXPRESS & SUITES, INTERSECTION OF HAYNES PLACE & LAMM RD., WILSON, NC 27783

STATUS: PERMIT, PROJ. DATE: 06-05-19, PROJ. NO. 1083218.000, PRINTED: 6/5/2019 4:24:04 PM, DRAWN BY: RPK, CHECKED BY: RMA, SHEET NO. S1.0, SCALE: As indicated