

GENERAL STRUCTURAL NOTES

- A. CONTRACTOR NOTES
1. THE CONTRACT DRAWINGS REPRESENT THE DESIRED RESULT OF CONSTRUCTION, THE METHODS OF CONSTRUCTION AND THE RISKS INVOLVED DURING CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR...
2. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS & ELEVATIONS PRIOR TO CONSTRUCTION AND SHALL REPORT ANY DISCREPANCIES TO THE ARCHITECT/ENGINEER BEFORE COMMENCING ANY WORK...
3. THE CONTRACTOR'S PROPOSED SUBSTITUTIONS SHALL BE APPROVED BY THE ARCHITECT/ENGINEER PRIOR TO COMMENCING ANY PERTINENT WORK...
4. CONTRACTOR & ALL SUBCONTRACTORS SHALL COORDINATE ALL DRAWINGS DURING BIDDING & SHALL REPORT ANY DISCREPANCIES...
B. DESIGN CRITERIA
1. THE STRUCTURE HAS BEEN DESIGNED ACCORDING TO THE 2017 FLOOR DA BUILDING CODE 6TH EDITION AND FOR SPECIFIC LOADS AS LISTED BELOW:
1. ROOF LOADS (NEW CONSTRUCTION)
A. D.L. = 20 P.S.F.
B. L.L. = 20 P.S.F. (NON-REDUCIBLE)
C. SNOW LOAD
a. GROUND SNOW LOAD Pg = 0 P.S.F.
b. FLAT ROOF SNOW LOAD Ps = 0 P.S.F.
c. SNOW EXPOSURE FACTOR Ce = 1.0
d. SNOW IMPORTANCE FACTOR Is = 1.0
e. THERMAL FACTOR Ct = 1.0
D. MECHANICAL EQUIPMENT LOADS AND LOCATIONS USED FOR DESIGN ARE AS INDICATED ON THE STRUCTURAL DRAWINGS...
2. FLOOR LOADS
A. LIVE LOAD
a. SLAB ON GRADE = 100 P.S.F.
B. LATERAL LOADS (WIND)
A. ULTIMATE DESIGN WIND SPEED, V-ULT = 175 MPH
B. NOMINAL DESIGN WIND SPEED, V-NOM = 136 MPH
C. WIND EXPOSURE = C
D. RISK CATEGORY = II
E. THE APPLICABLE INTERNAL PRESSURE COEFFICIENT = +0.18
COMONENTS AND CLADDING DESIGN LOADS. SEE COMPONENTS AND CLADDING EXTERNAL PRESSURE LOADS SCHEDULE ON THIS SHEET.
a. MANUFACTURERS PROVIDING MATERIALS SUCH AS DOOR AND WINDOWS SHALL PROVIDE CALCULATIONS FOR THEIR MATERIALS...
A. LATERAL LOADS (EARTHQUAKE)
1. SEISMIC IMPORTANCE FACTOR I = 1.0
2. BUILDING OCCUPANCY CATEGORY II
3. MAPPED SPECTRAL RESPONSE ACCELERATIONS
a. Sa = 0.164
b. S1 = 0.021
D. SITE CLASS = D (ASSUMED)
E. SPECTRAL RESPONSE COEFFICIENTS
a. Sds = 0.947
b. Sd1 = 0.332
F. SEISMIC DESIGN CATEGORY = A
G. BASIC STRUCTURE SYSTEM = BEARING WALL SYSTEM
H. SEISMIC RESISTING SYSTEM = ORDINARY REINFORCED MASONRY SHEAR WALLS
DESIGN BASE SHEAR V = 9,000 LB
J. RESPONSE MODIFICATION FACTOR R = 2
K. ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE (ELFP)
C. FOUNDATION, FILLING AND EXCAVATION
1. THE CONTRACTOR SHALL REFER TO THE GEOTECHNICAL SUBSURFACE INVESTIGATION REPORT AND SPECIFICATIONS FOR ALL REQUIREMENTS RELATED TO EXCAVATION, PREPARATION ON THE SUBGRADE, COMPACTION PROCEDURES, AND FOR ANY OTHER GEOTECHNICAL REQUIREMENTS...
THE RECOMMENDATIONS PRESENTED HEREIN ARE IN ACCORDANCE WITH THE SUBSURFACE INVESTIGATION REPORT PREPARED BY: PROFESSIONAL SERVICE INDUSTRIES, INC. (PSI) (INTERTEK) PROJECT NO. 10077-104, DATED OCTOBER 19, 2018.
1. THE DESIGN IS BASED ON AN ALLOWABLE NET SOIL BEARING CAPACITY = 2,500 P.S.F.
D. CAST IN PLACE CONCRETE
1. ALL CONCRETE SHALL BE TYPE II AND DEVELOP 4,000 P.S.I. COMPRESSIVE STRENGTH IN 28 DAYS.
2. REINFORCING BARS SHALL BE DEFORMED AND SHALL CONFORM TO ASTM A615, Fy = 60 K.S.I. REINFORCING BARS INDICATED TO BE WELDED SHALL CONFORM TO ASTM A818. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064.
3. SPLICES IN CONTINUOUS VERTICAL OR HORIZONTAL REINFORCING BARS SHALL BE PER LATEST ACI 318 OR (40) BAR DIAMETERS LAP SPLICE, WHICHEVER IS GREATER, UNLESS NOTED OTHERWISE AND SHALL BE EITHER CONTINUOUS OR SPLICED WITH DOWELS AT CORNERS.
4. CLEARANCES BETWEEN REINFORCING BARS AND CONCRETE SURFACES SHALL BE AS FOLLOWS:
DESCRIPTION COVER IN INCHES
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3
CONCRETE EXPOSED TO EARTH OR WEATHER:
#6 THROUGH #18 BARS 2
#8 BAR AND SMALLER 1 1/2
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
SLAB, WALLS, #1 BAR AND SMALLER 3/4
BEAMS, COLUMNS, PRIMARY REINFORCEMENT, TIES AND STRIPPUS 1 1/2
E. STRUCTURAL STEEL
1. ALL STRUCTURAL STEEL WIDE FLANGE MEMBERS SHALL CONFORM TO ASTM A992, Fy = 50 K.S.I. UNLESS NOTED OTHERWISE...
2. ANCHOR BOLTS SHALL CONFORM TO ASTM A36, ASTM 1554.
3. ALL CONCRETE STUD AND CONCRETE BLOCK ANCHORS SHALL BE H/LT OR APPROVED EQUAL.
4. ALL BOLTS FOR STEEL BEAM CONNECTIONS SHALL BE HIGH STRENGTH, A325 AND 3/4" DIAMETER UNLESS NOTED OTHERWISE.
5. ALL HEADED STUD ANCHORS SHALL BE MANUFACTURED BY 'NELSON STUD' OR APPROVED EQUAL.
6. ALL WELDS SHALL BE MADE WITH E70XX RODS AND WELDING SHALL CONFORM TO THE LATEST AWS CODE.
7. STEEL PIPE COLUMNS SHALL CONFORM TO ASTM A501, Fy = 38 K.S.I. OR ASTM A53, TYPE E OR S, GRADE B.
8. STEEL TUBE COLUMNS SHALL CONFORM TO ASTM A1085, GRADE A, Fy = 50 K.S.I.
9. AFTER ERECTION OF COLUMNS AND INSTALLATION OF NON-SHRINK GROUT, COAT ALL EXPOSED STEEL BELOW FINISH FLOOR ELEVATION WITH ONE COAT OF COLD APPLIED ASPHALT MASTIC COMPLYING WITH SSPC-PART 12.
F. MASONRY
1. ALL WALLS NOT SUBJECT TO EARTH OR WEATHER SHALL BE 2-CELL BLOCK MADE OF LIGHT WEIGHT AGGREGATE. ALL WALLS SUBJECT TO EARTH OR WEATHER SHALL BE 2-CELL BLOCK MADE OF NORMAL WEIGHT AGGREGATE. ALL BLOCK SHALL BE OF LOAD BEARING TYPE WITH COMPRESSIVE STRENGTH OF NOT LESS THAN Fm = 2,000 P.S.I. AT 28 DAYS. UNITS SHALL MEET THE CURRENT SPECIFICATION OF ASTM C-90, GRADE N, TYPE 1.
2. ALL MORTAR FOR UNIT MASONRY SHALL COMPLY WITH THE REQUIREMENTS OF ASTM C-270, TYPE S.
3. HORIZONTAL WIRE REINFORCING SHALL BE 9 GAUGE CONTINUOUS LADDER TYPE UNITS COMPLYING WITH ASTM A-82 AND SPACED AT 6" O.C. IN WALLS ABOVE 12'-0" IN HEIGHT AND SPACED AT 18" O.C. IN ALL OTHER APPLICATIONS.
4. PROVIDE BOND BEAM AT TOP AND BOTTOM OF ALL LOAD BEARING AND NON-LOAD BEARING CMU WALLS. IN NON-LOAD BEARING WALLS PROVIDE ADDITIONAL BOND BEAMS AT 4'-0" O.C. VERTICALLY. NON-LOAD BEARING WALLS SHALL BE REINFORCED WITH (1) #5 CONT. FOR 8' CMU WALL U.O.
5. VERTICAL REINFORCING SHALL BE (1) #5 @ 2'-0" O.C. UNLESS NOTED OTHERWISE AND ONE ADDITIONAL BAR AT EACH DOOR/WINDOW JAMB, ADJACENT TO CONTROL JOINTS, AND END BEARING DOOR/WINDOW OPENINGS AND AT EACH BUILDING CORNER. ADDITIONAL REINFORCING SHALL BE REQUIRED AS SHOWN ON THE PLANS AND DETAILS.
6. SPLICES IN CONTINUOUS VERTICAL OR HORIZONTAL REINFORCING BARS SHALL BE PER ACI-430 OR (40) BAR DIAMETERS, WHICHEVER HAS A GREATER LAP SPACING UNLESS NOTED OTHERWISE. ANCHORS SHALL BE CONTINUOUS OR SPLICED WITH DOWELS AT CORNERS. AT VERTICAL BAR LAPS PROVIDE DOWELS TO ASSURE THE BARS ARE LAPPED TOGETHER.
7. FILL CELLS OF BLOCK AT VERTICAL REINFORCING AND WHERE INDICATED WITH 3/8" MAX LARGER AGGREGATE CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF 2,500 P.S.I. COMPLYING WITH ASTM C476.
8. ALL LOAD BEARING CMU WALLS SHALL BE CONSTRUCTED SO THAT BLOCK IS INTERLOCKING AT ALL WALL CORNERS AND INTERSECTIONS. A VERTICAL COLD JOINT SHALL NOT BE PERMITTED AT THE JUNCTURE OF TWO LOAD BEARING CMU WALLS. ALL NON-LOAD BEARING CMU WALLS SHALL BE CONSTRUCTED SO THAT BLOCK IS INTERLOCKING AT THE CORNERS/INTERSECTIONS OF AT LEAST ONE END OF THE WALL.

- G. BAR JOISTS
1. BAR JOISTS SHALL BE DESIGNED AND FABRICATED ACCORDING TO THE LATEST STANDARDS OF THE STEEL JOIST INSTITUTE. JOIST FABRICATORS SHALL BE A MEMBER OF THE STEEL JOIST INSTITUTE.
2. BAR JOISTS SHALL BE WELDED TO SUPPORTING BEAMS, OR WELD PLATES WITH 1" OF 3/16" WELD ON EACH SIDE OF BAR JOIST, UNLESS NOTED OTHERWISE.
3. PROVIDE JOIST BRIDGING TO MEET THE REQUIREMENTS OF SJI. PROVIDE UPLIFT BRIDGING FOR A NET UPLIFT OF 59 PSF (ULTIMATE).
4. BRIDGING TERMINATING AT MASONRY WALLS OR STEEL BEAMS SHALL BE ANCHORED TO WALL OR BEAM. BRIDGING SHALL BE ANCHORED AT ENDS PRIOR TO APPLYING ROOF OR FLOOR LOADS.
5. BAR JOISTS AT COLUMN LOCATIONS TO BE BOLTED TO SUPPORTING BEAM AT TIME OF ERECTION.
6. MINIMUM BEARING REQUIREMENTS:
BAR JOISTS: 2 1/2" ON STRUCTURAL STEEL 4" ON STEEL BEARING PLATES ON MASONRY OR CONCRETE
7. UNLESS SHOWN ON STRUCTURAL DRAWINGS, NO PROVISIONS HAVE BEEN MADE IN THE BAR JOIST DESIGN TO ACCOUNT FOR CONCENTRATED LOADS. CONCENTRATED LOADS IN EXCESS OF 100 POUNDS WILL REQUIRE JOIST REINFORCING PER THE DETAILS.
H. PLYWOOD
1. ALL LUMBER SHALL BE GRADED IN ACCORDANCE WITH NFA STANDARDS.
2. ALL PLYWOOD SHALL BE STRESS RATED AT (4824), EXTERIOR OR GRADE (CD-X) WITH THICKNESS SHOWN ON PLANS.
3. PLYWOOD ON ROOF SHALL BE PLACED WITH FACE GRAIN ACROSS SUPPORTS AND STAGGERED SO CONTINUOUS PANEL JOINTS OCCUR ONLY IN ONE DIRECTION, PERPENDICULAR TO THE SPAN OF THE SUPPORT. SCREW PLYWOOD WITH #10 SELF-TAPPING SCREWS AT COLD FORMED SUPPORTS. SCREWS SHALL BE SPACED AT A MINIMUM 4" O.C. AT SUPPORTED EDGES OF PANEL AND AT 12" AT INTERMEDIATE SUPPORTS UNLESS NOTED OTHERWISE.
4. PLYWOOD SHALL BE SCREWED TO WALL STUDS WITH #10 SCREWS AT 6" O.C. AT PANEL EDGES (BLOCK ALL EDGES) AND 12" O.C. AT ALL INTERMEDIATE SUPPORTS, UNLESS NOTED OTHERWISE.
5. ALL PLYWOOD AND DRYWALL EDGES SHALL BE SUPPORTED WITH BLOCKING TO MATCH STUDS, TYP.
I. METAL DECK
1. ROOF DECK:
1 1/2" DEEP, TYPE 'B' (WIDE RIB), 20 GAUGE, GALVANIZED SHEET W/DTH = 36"
2. PREPARE AND REPAIR DAMAGED GALVANIZED COATING ON BOTH SURFACES OF DECK WITH GALVANIZING REPAIR PAINT ACCORDING TO ASTM 780 AND MANUFACTURER'S INSTRUCTIONS.
3. METAL DECK MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE (SDI). ALL PRODUCTS SHALL BE FACTORY MUTUAL APPROVED.
4. STEEL DECK SHALL BE CONTINUOUS OVER A MINIMUM OF 4 SUPPORTS.
5. DECK ATTACHMENT PATTERN SHALL BE AS INDICATED ON THE DRAWINGS.
6. PROVIDE 1 1/2" MINIMUM DECK BEARING AT ALL SUPPORTS. DECK SHALL BE PLACED AT THE PERIMETER WITH A COMPLETE RIB BEARING ON THE STEEL SUPPORT.
7. ALL METAL DECK WELDING IS ONLY TO BE PERFORMED BY AN AWS D1.3 CERTIFIED WELDER. CERTIFICATIONS ARE TO BE AVAILABLE ON SITE FOR VERIFICATION.
J. COLD-FORMED STEEL STRUCTURAL FRAMING
1. METAL STUD MANUFACTURER SHALL BE A MEMBER OF THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA).
2. METAL STUD STRENGTH CRITERIA:
GAUGES 20 THROUGH 18 - 33 KSI MIN. YIELD STRESS
GAUGES 16 THROUGH 12 - 50 KSI MIN. YIELD STRESS
TRACK - 33 KSI MIN. YIELD STRESS
3. METAL STUD MEMBER DESIGNATION: 600 S 152-33 M/L S GAUGE
33 20
43 18
54 16
66 14
97 12
WEB DEPTH 600 x 6"
MEMBER TYPE S x STUD
FLANGE WIDTH: 162 x 1522' (1 5/8")
MINIMUM THICKNESS IN MILS.
4. METAL STUDS AND ACCESSORIES SHALL HAVE A G60 GALVANIZED COATING UNLESS NOTED OTHERWISE.
5. FASTENERS (USE IN ACCORDANCE WITH MANUFACTURER'S INSTALLATION RECOMMENDATIONS):
SCREWS:
NOTE: ALL S.M.P.S.O.N CONNECTIONS REFERENCED IN THESE PLANS SHALL USE SCREWS OR ANCHORS AND BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS.
FASTENING STUD TO STUD:
USE #10 SELF-DRILLING S.M.P.S.O.N X METAL SCREWS UNLESS OTHERWISE NOTED.
MIN. EDGE DISTANCE = 3/4"
MIN. SPACING BETWEEN FASTENERS = 3/4"
FASTENING STUD TO CONCRETE OR MASONRY (UNLESS NOTED OTHERWISE IN THE DETAILS):
USE 3/8" DIA. S.M.P.S.O.N TITEN HD ANCHORS.
MIN. EMBEDMENT = 3/28" MIN. EDGE DISTANCE = 3"
MIN. SPACING BETWEEN FASTENERS = 4"
FASTENING TO STRUCTURAL STEEL:
USE #12 SELF-DRILLING S.M.P.S.O.N X METAL SCREWS UNLESS OTHERWISE NOTED.
MIN. EMBEDMENT = FULL PENETRATION; MIN. EDGE DISTANCE = 1/2"
MIN. SPACING BETWEEN FASTENERS = 1"
6. FASTENING (MINIMUM FASTENING REQUIREMENTS, UNLESS NOTED OTHERWISE):
TRACK TO CONCRETE:
(1) FASTENERS AT 16" O.C. TYP. W/ ADD'L FASTENER 3" FROM END OF TRACK AND (2) FASTENERS MINIMUM 6" JAMBS.
TRACK TO STRUCTURAL STEEL:
12" O.C. TYP. W/ ADD'L FASTENER 3" FROM END OF TRACK AND (2) FASTENERS MINIMUM 6" JAMBS.
STUD TO TRACK:
(1) #10 SCREW MINIMUM AT EACH FLANGE.
STUD TO STUD:
(4) #10 SCREWS MINIMUM.
STUD TO STEEL OR CONCRETE:
USE FASTENERS REFERENCED ABOVE IN DETAILS FOR CONNECTIONS, MINIMUM 2 FASTENERS.
7. WALL OPENING INTO STUDS:
IF WALL OPENING INTO STUDS IS NOT NECESSARY WHERE WALL SHEATHING IS ATTACHED ON BOTH SIDES OF THE STUDS, PROVIDE WALL SHEATHING TO BE ATTACHED TO BOTH SIDES OF STUDS. IF WALL OPENING INTO STUDS IS NECESSARY WHERE WALL SHEATHING IS ATTACHED ON ONE SIDE OF STUDS, PROVIDE BRACING BY ONE OF THE FOLLOWING METHODS:
- 2" WIDE, 18 GAUGE, STEEL STRAPS RUN HORIZONTALLY THROUGH STUD PUNCH OUTS AND ATTACHED AT EACH STUD.
NOTE: VERTICAL SPACING OF THE BRACING IS LIMITED TO A MAXIMUM OF 4'-0" THROUGHOUT THE HEIGHT OF THE WALL. PROVIDE ALL ACCESSORIES AS REQUIRED BY THE METAL STUD MANUFACTURER.
9. JOISTS SHALL BE FABRICATED TO PROVIDE 1/2" OF UNPINCHED WEB AT BEARING ENDS.
10. PROVIDE JOIST WEB STIFFENERS PER METAL STUD MANUFACTURER'S RECOMMENDATIONS AT BEARING POINTS.
11. PROVIDE END BLOCKING WHERE JOISTS ARE NOT RESTRAINED AGAINST ROTATION.
12. NO LIGHT GAUGE MEMBER THAT HAS BEEN TRIMMED OR CUT SHALL BE INSTALLED SO THAT AN EDGE OF A WEB PUNCH OUT OCCURS WITHIN 10" MINIMUM OF THE TRIMMED OR CUT MEMBER END.
13. MEMBER CUTTING MUST BE PERFORMED USING A SAW OR SHEAR. NO TORCH CUTTING IS ALLOWED AT ANY TIME. ALSO CUTTING OF ADDITIONAL HOLES, OTHER THAN THOSE PROVIDED BY THE MANUFACTURER, IN THE MEMBER WEB IS NOT PERMITTED AT ANY TIME.
14. SPLICING OF AXIALLY LOADED MEMBERS IS NOT ALLOWED AT ANY TIME.
15. BUILT UP LIGHT GAUGE HEADERS, CONSTRUCTED FOR EXTERIOR WALL CONDITIONS, SHALL HAVE INSULATION PLACED WITHIN THEM PRIOR TO THEIR INSTALLATION IN WALL SYSTEM.
16. SUBMITTALS:
PRODUCT DATA: FOR EACH TYPE OF COLD FORMED METAL FRAMING PRODUCT AND ACCESSORY INDICATED.
17. QUALITY ASSURANCE:
INSTALLER QUALIFICATIONS: AN EXPERIENCED INSTALLER WHO HAS COMPLETED COLD FORMED METAL FRAMING SIMILAR IN MATERIAL, DESIGN, AND EXTENT TO THAT INDICATED FOR THIS PROJECT AND WHOSE WORK HAS RESULTED IN CONSTRUCTION WITH A RECORD OF SUCCESSFUL IN SERVICE PERFORMANCE.
INSTALLATION SHALL CONFORM TO THE MANUFACTURER'S INSTALLATION INSTRUCTIONS.
AS IS (SPECIFICATIONS): COMPLY WITH AISC 'S' SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS' OR 'LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATION FOR COLD FORMED STEEL STRUCTURAL MEMBERS'.

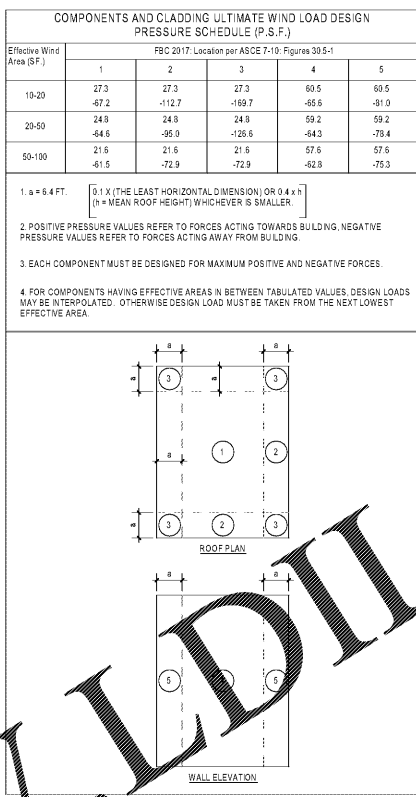


Table with 5 columns: Effective Wind Area (SF.), 1, 2, 3, 4, 5. Values range from -61.5 to 69.5. Includes Roof Plan and Wall Elevation diagrams.

J. STATEMENT OF SPECIAL INSPECTIONS
1. SPECIAL INSPECTIONS SHALL BE REQUIRED FOR THE FOLLOWING TYPES OF WORK. OWNER SHALL EMPLOY SPECIAL INSPECTION AGENCY... COORDINATE REQUIREMENTS WITH THE LOCAL JURISDICTION.
SCHEDULE OF SPECIAL INSPECTIONS (2015 IBC)
STEEL
Code Section Inspection Required Frequency of Inspection
1705.2.1.1 Material verification of high-strength bolts, nuts and washers.
a. Identification markings to ABC 360.
b. Manufacturer certificate of compliance.
1705.2.1.2 Inspection of high strength bolting.
a. Beating-type connections.
1705.2.1.5 Inspection of welding:
a. Structural Steel:
1. Complete and partial penetration groove welds.
2. Multipass fillet welds.
4. Single-pass fillet welds less than or equal to 5/16".
5. Floor and deck welds.
CONCRETE
Code Section Inspection Required Frequency of Inspection
1705.3.1 Inspection of reinforcing steel, including prestressing tendons and placement.
1705.3.3 Inspection of anchor cast in concrete where allowable loads have been increased or where strength design is used.
1705.3.4 Inspection of all steel-installed hardware (e.g., anchors, bolts, nuts, washers, etc.) to be verified for correct installation.
1705.3.6 Verify the time log for concrete is completed. If time log is not completed, make a strength test for strength tests, perform field tests for concrete strength, and determine the compressive strength of the concrete.
1705.3.7 Inspection of concrete and shotcrete placement for proper application techniques.
1705.3.8 Inspection for maintenance of specified curing temperatures and techniques.
1705.3.12 Inspection formwork for shape, location, and dimensions of the concrete member being formed.
MASONRY
Code Section Inspection Required Frequency of Inspection
1705.4.2 MASONRY: (SS01) ASCE 6 at start of masonry construction, verify the following:
a. Proportions of site-prepared mortar.
b. Construction of mortar joints.
c. Location of reinforcing, connectors, prestressing tendons and anchorages.
1705.4.3 Prior to grouting verify the following:
a. Grout space is clean.
b. Construction of mortar joints.
1705.4.4 During masonry construction verify the following:
a. Size and location of structural elements.
b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction.
c. Protection of masonry during cold weather (temperature below 40 degrees F) or hot weather (temperature above 90 degrees F).
SOIL
Code Section Inspection Required Frequency of Inspection
1705.5.1 Verify materials below footings are adequate to achieve the design bearing capacity.
1705.5.2 Verify excavations are extended to proper depth and have reached proper material.
1705.5.3 Perform classification and testing of controlled fill materials.
1705.5.4 Verify use of proper materials, densities and lift thicknesses during placement and compaction of controlled fill.
1705.5.5 Prior to placement of controlled fill, observe subgrade and verify that site has been prepared properly.
CAST IN PLACE CONCRETE PIERS
Code Section Inspection Required Frequency of Inspection
1705.8.1 Observe drilling operation and maintain complete and accurate records for each pier.
1705.8.2 Verify placement, cleanliness and ultimate bearing strain capacity. Verify embedment into bedrock (if applicable).

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SHEET NO.: S001
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