

ABBREVIATIONS	
A	ADOL. ADDITIONAL ADJ. ARCHITECT/ENGINEER ALT. ALTERNATE ARCHITECTURAL AFF. ABOVE FINISHED FLOOR
B	BLDG. BUILDING BM. BEAM BOT. BOTTOM B'. BOTTOM OF
C	CFMF. COLD FORMED METAL FRAMING CGS. CENTER OF GRAVITY OF STRAND CJ. CONTRACTION JOINT CL. CENTERLINE CLR. CLEAR CMU. CONCRETE MASONRY UNIT COL. COLUMN COORD. COORDINATE CONN. CONNECTION CONT. CONTINUOUS
D	DBA. DEFORMED BAR ANCHOR DIM. DIMENSION DWL. DOWEL
E	EA. EACH EE. EACH END EF. EACH FACE ES. EACH SIDE EW. EACH WAY ELEV. ELEVATION, ELEVATOR EMBED. EMBEDMENT EQ. EQUAL EXIST. EXISTING EXP. JT. EXPANSION JOINT
F	FTG. FOOTING FND. FOUNDATION
G	GALV. GALVANIZED GC. GENERAL CONTRACTOR GR. GRADE (MATERIAL)
H	HK. HOOK HORIZ. HORIZONTAL
I	IF. INSIDE FACE INFO. INFORMATION
J	JT. JOINT
K	K. KIPS KSF. KIPS PER SQUARE FOOT
L	LLH. LONG LEG HORIZONTAL LLV. LONG LEG VERTICAL LSH. LONG SIDE HORIZONTAL LSV. LONG SIDE VERTICAL
M	MANUF. MANUFACTURER MAX. MAXIMUM MECH. MECHANICAL MEP. MECH. ELECTRICAL PLUMBING MIN. MINIMUM MISC. MISCELLANEOUS
O	ON CENTER OPG. OPENING OPP. OPPOSITE OH. OPPOSITE HAND OUF. OUTSIDE FACE
P	PL. PLATE PT. POST-TENSIONED PCF. POUNDS PER CUBIC FOOT PSF. POUNDS PER SQUARE FOOT
R	REINF. REINFORCEMENT, REINFORCING REQ'D. REQUIRED
S	SCHED. SCHEDULE SIM. SIMILAR SOG. SLAB ON GROUND STIFF. STIFFENER SYM. SYMMETRIC
T	TEMP. TEMPERATURE T&S. TOP AND BOTTOM T'. TOP OF TYP. TYPICAL
U	UNO. UNLESS NOTED OTHERWISE U+C. UZUN + CASE
V	VERT. VERTICAL VIF. VERIFY IN FIELD
W	WF. WIDE FLANGE WP. WORK POINT WWF. WELDED WIRE FABRIC W. WITH

MATERIALS LEGEND	
	STEEL
	CONCRETE
	EARTH
	GRAVEL
	ROCK
	CMU

1.00 GENERAL

1.01 ALL CONSTRUCTION SHALL CONFORM TO THE 2012 INTERNATIONAL BUILDING CODE WITH THE CURRENT GEORGIA STATE AMENDMENTS. REFERENCE TO OTHER STANDARD SPECIFICATIONS OR CODES SHALL MEAN THE BUILDING CODE ADOPTED EDITION OR THE NOTED EDITION, IF NOT BUILDING CODE ADOPTED.

1.02 VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS AFFECTING NEW CONSTRUCTION BEFORE STARTING WORK. NOTIFY THE ARCHITECT OF ANY DISCREPANCY.

1.03 NOTIFY THE ARCHITECT IN WRITINGS OF CONDITIONS ENCOUNTERED IN THE FIELD CONTRADICTORY TO THOSE SHOWN IN THE CONTRACT DOCUMENTS.

1.04 THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. THE STRUCTURAL SYSTEM AND ITS ELEMENTS SHALL NOT BE CONSIDERED STABLE UNTIL THE STRUCTURE IS COMPLETE.

1.05 COORDINATE STRUCTURAL CONTRACT DOCUMENTS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. NOTIFY THE ARCHITECT OF ANY CONFLICT AND/OR OMISSION.

1.06 COORDINATE AND VERIFY FLOOR AND ROOF OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS. FOR ADDITIONAL OPENINGS, INSERTS, SLEEVES, CURBS, PADS, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS SEE ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.

1.07 THE CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF DIMENSIONS SHOWN ON THE STRUCTURAL AND ARCHITECTURAL DRAWINGS. NOTIFY THE ARCHITECT OF ANY DISCREPANCY BEFORE STARTING SHOP DRAWINGS OR ANY WORK. FOR DIMENSIONS NOT SHOWN, SEE ARCHITECTURAL DRAWINGS.

1.08 REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE ARCHITECT DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. CONTRACTOR IS ALSO RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES OF CONSTRUCTION.

1.09 CONSTRUCTION SHALL BE TESTED AND INSPECTED BY A QUALIFIED AGENCY IN ACCORDANCE WITH CHAPTER 17 OF THE BUILDING CODE. SEE SPECIFICATIONS FOR THE SCHEDULE OF SPECIAL INSPECTIONS AND ADDITIONAL REQUIREMENTS.

2.00 STRUCTURAL LOAD CRITERIA

2.01 RISK CATEGORY _____ III

2.02 SUPERIMPOSED DEAD LOADS:

- ROOFING _____ 10 PSF
- MISCELLANEOUS ROOF LOAD _____ 10 PSF
- MISCELLANEOUS FLOOR LOAD _____ 10 PSF

2.03 LIVE LOADS:

- SCHOOLS:
 - ELEVATED FLOORS _____ 20 PSF
 - STAIRS _____ 100 PSF
 - PARTITIONS (UNREDUCIBLE) _____ 15 PSF
- ROOF _____ 20 PSF
- *LIVE LOAD REDUCTION IS TAKEN IN ACCORDANCE WITH THE CODE.
- **OR ACTUAL WEIGHT OF EQUIPMENT, WHICHEVER IS GREATER

2.04 SNOW LOAD CRITERIA:

- GROUND SNOW LOAD _____ Pg = 5 PSF
- FLAT-ROOF SNOW LOAD _____ Pf = 3.86 PSF
- SNOW EXPOSURE FACTOR _____ Ce = 1.0
- SNOW LOAD IMPORTANCE FACTOR _____ Is = 1.10
- THERMAL FACTOR _____ Ct = 1.0*
- *WITH ADDITIONAL LOADING DUE TO DRIFTING PER CODE

2.05 WIND LOAD CRITERIA:

- ULTIMATE DESIGN WIND SPEED, 3 SECOND GUST _____ V_{ULT} = 120 MPH
- NOMINAL DESIGN WIND SPEED _____ V_{ASD} = 93 MPH
- WIND EXPOSURE _____ B
- INTERNAL PRESSURE COEFFICIENT _____ G_{api} = ± 0.18
- COMPONENTS AND CLADDING ULTIMATE DESIGN WIND PRESSURES (PSF):

ZONE	EFFECTIVE WIND AREA (FT ²)		
	A < 10	A = 50	A > 100
WALLS	OTHER	26	24
	CORNER	27	25
ROOF	OTHER	26	25
	EDGE	24	23
PARAPETS	OTHER	26	24
	CORNER	27	25

NOTES:

- CORNER & EDGE ZONES SHALL EXTEND 3'-0" FROM BUILDING EDGES.
- *INDICATES POSITIVE PRESSURE; **INDICATES NEGATIVE PRESSURE (SUCTION).
- FOR UNWINDABLE PRESSURE ZONES, MULTIPLY TABULATED PRESSURES BY 0.6.

2.06 SEISMIC LOAD CRITERIA:

- SEISMIC IMPORTANCE FACTOR _____ Is = 1.25
- 1.0 SECOND SPECTRAL RESPONSE ACCELERATION _____ S_s = 0.187 G
- 0.2 SECOND SPECTRAL RESPONSE ACCELERATION _____ S₁ = 0.090 G
- SITE CLASS _____ D
- SITE COEFFICIENT _____ Fa = 1.6
- SITE COEFFICIENT _____ Fv = 2.4
- 0.2 SECOND SPECTRAL DESIGN RESPONSE ACCELERATION _____ S_{DS} = 0.199 G
- 1.0 SECOND SPECTRAL DESIGN RESPONSE ACCELERATION _____ S_{DI} = 0.144 G
- SEISMIC DESIGN CATEGORY _____ C
- BASIC SEISMIC FORCE RESISTING SYSTEM - CAFETERIUM:
- STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
- RESPONSE MODIFICATION COEFFICIENT _____ R = 3
- SYSTEM OVERSTRENGTH FACTOR _____ Do = 3
- DEFLECTION AMPLIFICATION FACTOR _____ Cd = 3
- DESIGN BASE SHEAR _____ 37.35 KIPS
- SEISMIC RESPONSE COEFFICIENT _____ Ca = 0.083
- BASIC SEISMIC FORCE RESISTING SYSTEM - CLASSROOM:
- COMBINATION INTERMEDIATE REINFORCED MASONRY SHEAR WALLS AND STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
- ANALYSIS PER EQUIVALENT LATERAL FORCE PROCEDURE

3.00 FOUNDATIONS AND SLAB-ON-GROUND

3.01 THE DESIGN OF FOUNDATIONS, RETAINING WALLS AND SLABS-ON-GROUND IS BASED ON THE CRITERIA ESTABLISHED IN THE GEOTECHNICAL REPORT BY GEOTECHNICAL ENGINEERS DATED MARCH 27, 2019. JOB # 190148.20. NEW FOUNDATION SIZES ARE BASED ON AN ALLOWABLE BEARING PRESSURE OF 2500 PSF. ENLARGED FOUNDATIONS BELOW THE NEW MEZZANINE ARE BASED ON AN ALLOWABLE BEARING PRESSURE OF 3000 PSF.

3.02 RETAINING WALLS ARE BASED ON THE FOLLOWING CRITERIA:

- EQUIVALENT LATERAL FLUID PRESSURE, ACTIVE _____ 42 PCF
- EQUIVALENT LATERAL FLUID PRESSURE, AT REST _____ 63 PCF
- EQUIVALENT LATERAL FLUID PRESSURE, PASSIVE _____ 360 PCF

• COEFFICIENT OF FRICTION _____ 0.40

3.03 THE SPECIAL INSPECTOR SHALL INSPECT THE CONDITION AND ASSURE THE ADEQUACY OF ALL SUBGRADES, FILLS AND BACKFILLS BEFORE PLACEMENT OF FOUNDATIONS, FOOTINGS, SLABS, AND WALLS AND SHALL SUBMIT REPORTS TO THE ARCHITECT DESCRIBING THE FINDINGS INCLUDING ANY NON-CONFORMING WORK.

3.04 SIDES OF FOUNDATIONS SHALL BE FORMED UNLESS CONDITIONS PERMIT EARTH FORMING. FOUNDATIONS POURED AGAINST THE EARTH REQUIRE THE FOLLOWING PRECAUTIONS: SLOPE SIDES OF EXCAVATIONS AS APPROVED BY THE ARCHITECT AND CLEAN UP SLOUGHING BEFORE AND DURING CONCRETE PLACEMENT.

3.05 WHERE FOOTING STEPS ARE NECESSARY, THEY SHALL BE NO STEEPER THAN ONE VERTICAL TO TWO HORIZONTAL.

3.06 BASEMENT WALLS (WALLS SUPPORTED AT THE TOP AND BOTTOM BY SLABS) REQUIRE THE FOLLOWING PRECAUTION: DO NOT BACKFILL AGAINST THE WALL BEFORE THE SLABS AT TOP AND BOTTOM ARE CAST AND HAVE REACHED 75% OF DESIGN STRENGTH.

3.07 UNLESS NOTED OTHERWISE, SLABS-ON-GROUND SHALL BE MINIMUM 5" THICK, UNREINFORCED, PLACED ON COMPACTED SUBGRADE. PROVIDE 15 MIL MINIMUM POLYETHYLENE VAPOR RETARDER BENEATH THE FLOOR SLAB WITH JOINTS LAPPED NOT LESS THAN 6 INCHES AND TAPED. PLACE CONTRACTION JOINTS WHERE SHOWN ON PLANS, AT COLUMN LINES AND AT INTERMEDIATE LINES SUCH THAT SPACING BETWEEN JOINTS DOES NOT EXCEED 12'-6" AND ASPECT RATIO OF PANELS DOES NOT EXCEED 1.5. LOCATE CONTRACTION JOINTS AT CONTRACTION JOINTS. PROVIDE A 4" SUBBASE LAYER OF COMPACTED GRADED AGGREGATE BENEATH THE VAPOR RETARDER.

3.08 UNLESS NOTED OTHERWISE, POLYSTYRENE FILL SHALL BE ASTM 578, TYPE VI OR XIB, 50 COMPRESSION STRENGTH ON 6 MIL POLYETHYLENE SLIP SHEET.

4.00 REINFORCED CONCRETE

4.01 ALL CONCRETE WORK SHALL CONFORM TO ACI 301-10, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS". DESIGN IS BASED ON ACI 318, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE".

4.02 UNLESS NOTED OTHERWISE, ALL CONCRETE SHALL BE NORMALWEIGHT AND HAVE THE FOLLOWING MINIMUM 28-DAY STRENGTHS:

- FOUNDATIONS _____ 3000 PSI
- FOUNDATION WALLS & RETAINING WALLS _____ 3000 PSI
- SLAB-ON-GROUND _____ 3500 PSI

4.03 CONCRETE MIX DESIGNS, IN ACCORDANCE WITH ACI 318 SECTION 5.3, SHALL BE SUBMITTED TO THE TESTING AGENCY AND THE ARCHITECT.

4.04 THE PROPOSED MATERIALS AND MIX DESIGN SHALL BE FULLY DOCUMENTED AND REVIEWED BY THE SPECIAL INSPECTOR. RESPONSIBILITY FOR OBTAINING THE REQUIRED DESIGN STRENGTH IS THE CONTRACTOR'S.

4.05 USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED.

4.06 THE AIR CONTENT IN ALL CONCRETE EXPOSED TO WEATHER AT POINT OF PLACEMENT SHALL BE PROPORTIONED ACCORDING TO ACI 318 TABLE 4.4.1 FOR EXPOSURE CLASS F1, MODERATE. PROVIDE AIR CONTENT THAT COMPLIES WITH FLOOR SYSTEM FIRE RATING REQUIREMENTS.

4.07 UNLESS NOTED OTHERWISE, SAMPLES FOR STRENGTH TESTS OF EACH CLASS OF CONCRETE PLACED EACH DAY SHALL BE TAKEN BY THE TESTING AGENCY NOT LESS THAN ONCE A DAY, NO LESS THAN ONCE FOR EACH 100 CUBIC YARDS OF CONCRETE, NOR LESS THAN ONCE FOR EACH 5000 SQUARE FEET OF SURFACE AREA FOR SLABS AND WALLS, WHEN FREQUENT TESTING WILL PROVIDE FEWER THAN FIVE TESTS FROM EACH BATCH. TEST SAMPLES SHALL BE MADE FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES FROM EACH BATCH. IF FEWER THAN FIVE BATCHES ARE USED, SAMPLE CONCRETE IN EACH BATCH WITH ASTM C172. PERFORM THE FOLLOWING TESTS IN ACCORDANCE WITH THE INDICATED STANDARD:

- SLUMP: ASTM C143
- AIR CONTENT:
 - ASTM C173 FOR NORMAL WEIGHT CONCRETE
 - ASTM C176 FOR LIGHTWEIGHT CONCRETE
- COMPRESSIVE STRENGTH: ASTM C39, WITH (1) CYLINDER AT 7 DAYS, (2) 6"x12" CYLINDERS OR (3) 4"x8" CYLINDERS AT 28 DAYS, AND (1) SPECIMEN HELD IN RESERVE
- UNIT WEIGHT OF LIGHTWEIGHT CONCRETE: ASTM C987

4.08 BEGIN CURING SLABS IMMEDIATELY AFTER FINISHING CONCRETE PER PROJECT SPECIFICATIONS. CURING METHOD SHALL BE COMPATIBLE WITH ADHERED FINISHES. A MOIST CURE OR DISSIPATING CURING COMPOUND SHALL BE USED AT CEMENTITIOUS FINISHES. SUBMIT PRODUCT DATA TO THE ARCHITECT FOR REVIEW.

4.09 HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED ONLY WHERE INDICATED. THE ARCHITECT SHALL APPROVE THE LOCATION OF VERTICAL CONSTRUCTION JOINTS. CONSTRUCTION JOINTS SHALL BE THOROUGHLY ROUGHENED BY MECHANICAL MEANS AND CLEANED.

4.10 UNLESS NOTED OTHERWISE, PLACE VERTICAL CONTRACTION JOINTS IN CONCRETE WALLS AT 20'-0" (MAX), NOT TO EXCEED THREE TIMES THE HEIGHT OF THE WALL, 3/4" DEEP, V-CHAMFERED ON BOTH FACES. CONSTRUCTION JOINTS SHALL FALL ON CONTRACTION JOINTS AND SHALL BE KEYS. STOP 50 PERCENT OF THE SPECIFIED LONGITUDINAL REINFORCING AT THE CONTRACTION/CONSTRUCTION JOINT.

4.11 CHAMFER OR ROUND ALL EXPOSED CORNERS MINIMUM 3/4".

4.12 DETAIL CONCRETE REINFORCING AND ACCESSORIES IN ACCORDANCE WITH ACI "DETAILING MANUAL", SP-08(04) SUBMIT SHOP DRAWINGS FOR APPROVAL, SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING REINFORCING STEEL AND ACCESSORIES. DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.

4.13 DETAIL ALL CONCRETE WALLS AND BEAMS IN ELEVATION UNLESS SPECIFICALLY APPROVED OTHERWISE. CUT SECTIONS SHOWING BAR LOCATIONS AND CONCRETE COVER.

4.14 REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 UNLESS NOTED OTHERWISE.

4.15 WELDED WIRE FABRIC (MESH) SHALL CONFORM TO ASTM A1034 AND SHALL BE PROVIDED IN FLAT SHEETS (ROLLS NOT PERMITTED) LAP TWO SQUARES AT SPLICES.

4.16 TIE ALL REINFORCING STEEL AND EMBEDMENTS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN THE POSITION OF REINFORCING WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES. "STICKING" DOWELS INTO WET CONCRETE IS NOT PERMITTED.

4.17 PROVIDE CONTINUOUS REINFORCING WHEREVER POSSIBLE; SPLICE ONLY AS SHOWN OR APPROVED; STAGGER SPLICES WHERE POSSIBLE; USE CLASS "B" TENSION SPLICE UNLESS NOTED OTHERWISE. DOWELS SHALL MATCH THE SIZE AND SPACING OF THE SPECIFIED REINFORCING AND SHALL BE LAPPED WITH CLASS "B" TENSION SPLICES. UNLESS NOTED OTHERWISE LAP LENGTHS EXPRESSED IN NUMBER OF BAR DIAMETERS SHALL BE AS FOLLOWS:

BAR SIZE	CLASS	NORMALWEIGHT CONCRETE, f _c (psi)			
		3,000	4,000	5,000	6,000
#6 OR SMALLER	A	44 DIAM	38 DIAM	34 DIAM	31 DIAM
	B	57 DIAM	49 DIAM	44 DIAM	40 DIAM
#7 OR LARGER	A	55 DIAM	47 DIAM	42 DIAM	39 DIAM
	B	71 DIAM	62 DIAM	55 DIAM	50 DIAM

NOTES:

- INCREASE THE ABOVE LAP LENGTHS BY A FACTOR OF 1.3 FOR TOP BARS AND BY A FACTOR OF 1.3 FOR LIGHTWEIGHT CONCRETE.

4.18 REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE:

- CONCRETE CAST AGAINST EARTH (NOT FORMED) _____ 3"
- FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:
 - #6 THROUGH #10 BARS _____ 2"
 - #5 BARS AND SMALLER _____ 1 1/2"
- CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
 - SLABS AND WALLS _____ 1"
 - BEAM STIRRUPS AND COLUMN TIES _____ 1 1/2"

4.19 DO NOT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS WITHIN THE SLAB OR WALL UNLESS SPECIFICALLY SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.

4.20 DO NOT WELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE ARCHITECT.

4.21 FLOOR FINISH TOLERANCES FOR INTERIOR SLABS SHALL BE MEASURED IN ACCORDANCE WITH ASTM E1155. MINIMUM FLATNESS VALUES SHALL BE IN ACCORDANCE WITH SPECIFICATION SECTION 03300. EXTERIOR SLABS SHALL DRAIN FREELY WITH A MAXIMUM VARIATION FROM THE INDICATED PLANE OF 1/4" IN 10'-0".

5.00 CONCRETE MASONRY

5.01 CONCRETE MASONRY DESIGN AND CONSTRUCTION SHALL CONFORM TO THE FOLLOWING:

- ACI 530.1 / ASCE 5 / TMS 402, "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES"
- ACI 530.1 / ASCE 6 / TMS 602, "SPECIFICATIONS FOR MASONRY STRUCTURES"

5.02 UNLESS NOTED OTHERWISE, PROVIDE HOLLOW, LOAD-BEARING CONCRETE MASONRY UNITS (CMU) CONFORMING TO ASTM C90, WITH A DENSITY OF LESS THAN 105 PCF.

5.03 PROVIDE CONCRETE MASONRY WITH MINIMUM COMPRESSIVE STRENGTH, f_m = 2,000 PSI, CORRESPONDING TO UNIT STRENGTH OF 2,800 PSI ON NET CROSS-SECTIONAL AREA OF CMU DETERMINED IN ACCORDANCE WITH ASTM C140.

5.04 PROVIDE TYPE "S" MORTAR IN ACCORDANCE WITH THE PROPORTION SPECIFICATION OF ASTM C270 UNLESS NOTED OTHERWISE. MORTAR BED JOINTS SHALL NOT EXCEED 5/8 INCHES IN THICKNESS. VOLUMES SHALL BE MEASURED BY MEANS OF THE USE OF A BOX OR OTHER CONTAINER OF PREDETERMINED CAPACITY.

5.05 PROVIDE GROUT FOR REINFORCED MASONRY IN ACCORDANCE WITH ASTM C476 WITH MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI UNLESS NOTED OTHERWISE. GROUT ONLY REINFORCED CELLS AND WHERE OTHERWISE NOTED.

5.06 LAP REINFORCING BARS AS FOLLOWS, UNLESS NOTED OTHERWISE:

- #4 BARS _____ 24"
- #5 BARS _____ 24"
- #6 BARS _____ 24"

5.07 PROVIDE TRUSS OR LADDER TYPE HORIZONTAL JOINT REINFORCING COMPLYING WITH ASTM A951, WITH MINIMUM 3" GA LONGITUDINAL LACING CODED, PLACED 16 INCHES ON CENTER UNLESS NOTED OTHERWISE. PROVIDE REINFORCING "L" S AT WALL CORNERS AND "T" S AT WALL INTERSECTIONS.

5.08 PROVIDE MINIMUM CLEAR VERTICAL GROUDED FULL HEIGHT, IN THE FIRST FULL CELL AT EACH SIDE OF OPENINGS, AT WALL ENDS, AT INSIDE OF CONTRACTION JOINTS AND AT CORNERS.

5.09 REINFORCING UNITS IN REINFORCED BOND UNLESS NOTED OTHERWISE.

5.10 MASONRY WALLS SHALL BE ANCHORED TO STRUCTURE AT TOP OF WALL AND AT EACH FLOOR AND ROOF LEVEL. SEE TYPICAL DETAILS. PROVIDE BOND BEAMS REINFORCED WITH 2#5 AT EACH FLOOR AND ROOF ANCHORAGE LEVEL AND AT TOP OF WALL.

5.11 SHOP DRAWINGS SHALL BE SUBMITTED FOR APPROVAL SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING REINFORCING STEEL AND ACCESSORIES. SHOW WALL STEEL IN ELEVATION. INDICATE CONTROL JOINT LOCATIONS. NO FABRICATION SHALL BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED AND APPROVED.

5.12 UNLESS NOTED OTHERWISE, PROVIDE VERTICAL CONTROL JOINTS ACCORDING TO THE FOLLOWING CRITERIA:

- AT 25'-0" ON CENTER MAXIMUM
- AT 12'-0" MAXIMUM FROM BUILDING CORNERS
- NO CLOSER THAN 1'-4" TO OPENING EDGES
- NO CLOSER THAN 1'-4" TO MAJOR BEAM OR JOIST BEARINGS
- DISCONTINUE HORIZONTAL JOINT REINFORCING AT CONTROL JOINTS IN THE FIELD OF THE WALL, BUT NOT AT CORNERS OR INTERSECTIONS.

5.13 BOND BEAMS, CMU UNITS, MASONRY BENEATH STEEL BEAM AND JOIST BEARINGS, AND OTHER STRUCTURAL ELEMENTS SHALL EXTEND UNINTERRUPTED ACROSS CONTROL JOINTS. PROVIDE RAKED JOINTS IN THESE ELEMENTS TO MATCH THE CONTROL JOINT.

5.14 UNLESS NOTED OTHERWISE, REINFORCE ALL WALLS AS FOLLOWS:

- EXTERIOR WALLS _____ #6@48" VERTICAL
- INTERIOR WALLS _____ #4@48" VERTICAL

5.15 SAMPLE AND TEST GROUT IN ACCORDANCE WITH ASTM C1019. CAST AND LABORATORY CURE ONE SET OF FIVE CUBES FOR EACH 5000 SQUARE FEET OF MASONRY WALL SURFACE. TEST ONE CUBE AT SEVEN DAYS FOR INFORMATION, THREE CUBES AT 28 DAYS FOR A STRENGTH TEST AND HOLD ONE CUBE IN RESERVE.

6.00 STRUCTURAL STEEL

6.01 ALL STRUCTURAL STEEL CONSTRUCTION SHALL CONFORM TO THE AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".

6.02 SHOP DRAWINGS PREPARED IN ACCORDANCE WITH AISC'S DETAILING MANUAL "DETAILING FOR STEEL CONSTRUCTION, 3RD EDITION" SHALL BE SUBMITTED FOR APPROVAL. NO FABRICATION SHALL BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED AND APPROVED.

6.03 UNLESS NOTED OTHERWISE, STRUCTURAL STEEL WIDE FLANGES AND TEES SHALL CONFORM TO ASTM A992, GRADE 50; ROUND, SQUARE AND RECTANGULAR HSS SECTIONS SHALL CONFORM TO ASTM A500, GRADE C; ROUND PIPES SHALL CONFORM TO ASTM A53, GRADE B; AND ALL OTHER SHAPES AND PLATES SHALL CONFORM TO ASTM A36 OR A572, GRADE 50.

6.04 STEEL FRAMING CONNECTIONS SHALL BE BOLTED OR WELDED:

- BOLTED JOINTS SHALL CONFORM TO RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS". BOLTS SHALL CONFORM TO ASTM A325 AND SHALL BE MINIMUM 3/4" DIAMETER, UNLESS NOTED OTHERWISE. ALL BOLTS SHALL BE CONSIDERED BEARING TYPE WITH BOLTS PRE-TENSIONED, UNLESS OTHERWISE NOTED. PROVIDE DIRECT TENSION INDICATORS (LOAD INDICATING WASHERS) IN ACCORDANCE WITH ASTM F959 OR TENSION CONTROL BOLTS (TWIST OFF BOLTS) IN ACCORDANCE WITH ASTM F1852 FOR ALL HIGH STRENGTH BOLTS.
- WELDS SHALL CONFORM TO THE "STRUCTURAL WELDING CODE" OF THE AMERICAN WELDING SOCIETY, AWS D1.1. USE E70XX ELECTRODES. WELDING PROCESSES AND OPERATORS SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATIONS PROCEDURES". WELDERS SHALL CARRY PROOF OF QUALIFICATIONS ON THEIR PERSONS.

6.05 ANCHOR RODS SHALL CONFORM TO ASTM F1554, GR 55, S1 (WELDABLE) UNLESS OTHERWISE NOTED. FOR ANCHOR RODS TO BE GALVANIZED, THE END OF THE ANCHOR ROD INTENDED TO PROJECT FROM THE CONCRETE SHALL BE STEEL DIE STAMPED WITH THE GRADE IDENTIFICATION AS REQUIRED BY SUPPLEMENT S3.

6.06 DO NOT USE GAS CUTTING TORCHES FOR CORRECTING FABRICATION ERRORS IN THE STRUCTURAL FRAMING.

6.07 STRUCTURAL STEEL CONNECTIONS NOT SHOWN ON THE CONTRACT DOCUMENTS ARE A DEFERRED SUBMITTAL. SEE DEFERRED SUBMITTAL SECTION OF THE GENERAL NOTES. UNLESS NOTED OTHERWISE BEAM END CONNECTIONS SHALL BE PROPORTIONED AS FOLLOWS:

- MINIMUM 5/16" THICK DOUBLE ANGLE OR 3/8" THICK SINGLE-PLATE SHEAR CONNECTIONS, FULL DEPTH OF THE BEAM, WELDED OR BOLTED WITH VERTICAL BOLT SPACING = 3", AND
- WHERE BEAM REACTIONS ARE SHOWN, CONNECTIONS SHALL DEVELOP THE REACTION GIVEN, OR
- WHERE BEAM REACTIONS ARE NOT SHOWN, CONNECTIONS SHALL BE PROPORTIONED TO SUPPORT 60% OF THE TOTAL UNIFORM LOAD CAPACITY (ULC) SHOWN IN THE UNIFORM LOAD TABLES OF THE AISC MANUAL, FOR THE SPECIFIED BEAM SIZE, SPAN, AND GRADE OF STEEL. FOR COMPOSITE BEAMS, PROPORTION CONNECTIONS FOR 80% OF THE ULC.

6.08 PROVIDE A SHOP COAT OF STANDARD PRIMER PAINT. PRIMER SHALL BE COMPATIBLE WITH FINISH COAT. TOUCH UP AREAS DAMAGED IN HANDLING AND ERECTION WITH THE SAME PAINT USED FOR SHOP COAT. STEEL SURFACES TO BE WELDED OR ENCASED IN CONCRETE OR FIREPROOFING, CONNECTIONS DESIGNATED AS SLIP CRITICAL TYPE, OR SURFACES RECEIVING WELDED SHEAR CONNECTORS IN THE FIELD SHALL NOT BE PAINTED.

6.09 PLACE NON-SHRINK, HIGH STRENGTH GROUT (MINIMUM 6,000 PSI) UNDER BASE PLATES AFTER SETTING AND LEVELING, AND PRIOR TO PLACING ELEVATED SLAB CONCRETE.

PERKINS + WILL

1315 Peachtree St NE
Atlanta, GA 30309
404.873.2000
404.873.2932
www.perkinswill.com

CONSULTANTS

Civil
Bradlow Land Planning
15 Simpson St NW, Atlanta, GA 30308

Structural
Uzun-Cable
1230 Peachtree St NE, Suite 2500 Atlanta, GA 30309

MEP
Matheson Bai & Associates
11285 Elkins Rd., Suite F3, Roswell, GA 30076

Landmarking
Perkins+Will
1315 Peachtree St NE, Atlanta, GA 30309

Construction
Camacho
3103 Medlock Bridge Road, Norcross, GA 30071

Owner
KIPP Metro Atlanta Schools
1445 Maynard Road NW, Atlanta, GA 30331

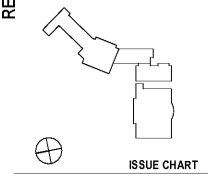
Health

Contractor

PROJECT

KIPP ATLANTA COLLEGIATE - Addition and Renovations
98 Anderson Ave NW
Atlanta GA 30314

RELEASED FOR CONSTRUCTION 06/07/2019



DATE	ISSUE	DATE
Job Number	18220	
Drawn	ZZ	
Checked	SG	
Approved	JH	

TITLE

GENERAL NOTES

SHEET NUMBER

S00-01

Copyright © 2018 Perkins+Will

Order Plans @ WWWWWWW.com