I. GENERAL NOTES

A. THESE NOTES SHALL APPLY EXCEPT WHERE OTHERWISE INDICATED BY THE DRAWINGS OR SPECIFICATIONS.

B. WHERE A DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL LIKE OR SIMILAR CONDITIONS EVEN THOUGH NOT SPECIFICALLY MARKED ON THE DRAWINGS.

C. IF APPLICABLE, CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND CONDITIONS OF EXISTING BUILDINGS AFFECTING NEW CONSTRUCTION, AND DISCREPANCIES ARE TO BE REPORTED IMMEDIATELY TO THE ARCHITECT/ENGINEER IN WRITING.

D. GENERAL CONTRACTOR SHALL ENSURE THAT ALL MATERIALS ARE IN COMPLIANCE WITH THE PLANS AND SPECIFICATIONS.

E. ALL MASONRY WALLS BELOW GRADE SHALL BE BACKFILLED ON BOTH SIDES OF WALL SIMULTANEOUSLY. PROVIDE TEMPORARY BRACING AS REQUIRED TO ADEQUATELY SUPPORT STRUCTURE DURING CONSTRUCTION AND BACKFILLING. BRACING SHALL REMAIN IN PLACE UNTIL ALL FLOOR AND ROOF CONNECTIONS ARE COMPLETE.

F. CENTER LINE OF COLUMN = CENTER LINE OF FOOTING = CENTER LINE OF ANCHOR BOLT TEMPLATE UNLESS NOTED OTHERWISE. NO PIPING SHALL PASS THROUGH OR UNDER ANY FOOTING WITHOUT PRIOR APPROVAL OF THE ENGINEER.

G. DIMENSIONS AT FRAMED OPENINGS TO BE VERIFIED WITH APPLICABLE SUB-CONTRACTOR BEFORE FABRICATION OF STEEL. IF ANY DISCREPANCIES ARE FOUND, THE ARCHITECT/ENGINEER IS TO BE IMMEDIATELY NOTIFIED IN WRITING.

II. DESIGN CRITERIA:

A. BUILDING CODE : 2012 INTERNATIONAL BUILDING CODE w/ CITY AMENDMENTS

B. DESIGN LOADS:

1. ADDITIONAL DESIGN LOADS INDICATED ON STRUCTURAL DRAWINGS SHALL BE IDENTIFIED AS FOLLOWS:

   LL = LIVE LOAD
   WL = WIND LOAD
   EL = SEISMIC LOAD
   SN = SNOW LOAD
   SD = SNOW DRIFT

2. WIND:
   a. V_{ULT} = 115 mph
   b. RISK CATEGORY = II IMPORTANCE FACTOR = 1.00
   c. MEAN ROOF HT = 44'-0"
   d. EXPOSURE C
   e. END ZONE = 8'-0"
   f. ENCLOSED BUILDING, GC_{pi}=+/-0.18
   g. WIND PRESSURES FOR COMPONENTS AND CLADDING BY TRIBUTARY AREA. VALUES BASED ON WINDSPEED=115 MPH AND EXPOSURE=C. VALUES ARE UNFACTORED AND MAY BE USED IN EITHER STRENGTH DESIGN OR ALLOWABLE STRESS DESIGN LOAD COMBINATIONS OF ASCE 7-10.

COMPONENT & CLADDING PRESSURES, PSF

REGION	TRIBUTARY AREA			
	10 ft²	20 ft²	50 ft²	100 ft²
①	+16.0/-36.2	+16.0/-35.2	+16.0/-34.0	+16.0/-33.1
②	+16.0/-60.7	+16.0/-54.5	+16.0/-45.3	+16.0/-39.2
③	+16.0/-91.3	+16.0/-76.0	+16.0/-54.5	+16.0/-39.2
④	+36.2/-39.2	+34.6/-37.4	+32.5/-35.5	+30.6/-34.0
⑤	+36.2/-48.4	+34.6/-45.3	+32.5/-40.8	+30.6/-37.7

LOADS TO BE APPLIED PER FIGURES 30.4-1 AND 30.4-2A OF ASCE 7-10.

3. SEISMIC:
   a. RISK CATEGORY = II IMPORTANCE FACTOR = 1.00
   b. SITE CLASS = C
   c. S_s = 0.346 S_{DS} = 0.277
      S_1 = 0.159 S_{D1} = 0.174
   d. SEISMIC DESIGN CATEGORY = C
   e. EQUIVALENT LATERAL FORCE PROCEDURE
   f. BEARING WALL SYSTEM - LIGHT FRAMED WOOD STUD WALLS w/ WOOD STRUCTURAL PANELS R=6 1/2
   g. C_s= 0.0426
   h. DESIGN BASE SHEAR = 98ᵏ

4. SNOW:
   a. RISK CATEGORY = II IMPORTANCE FACTOR = 1.00
   b. Pg = 10.0 psf Pf = 10.0 psf RAIN-ON-SNOW = 5.0 psf
   c. Ce = 1.0 Ct= 1.0

C. FOUNDATIONS:

1. FOUNDATION DESIGN IS BASED ON SUBSURFACE EXPLORATION REPORT PREPARED BY: BEAVER ENGINEERING, INC. PROJECT # 18-7399 DATED: JANUARY 11, 2019
   ReMi STUDY BY EARTH SCIENCE ENGINEERING, LLC DATED: JANUARY 2019
   AGGREGATE PIER PROPOSAL BY HAYWARD BAKER, INC DATED: FEBRUARY 12, 2019
2. INSTALLATION OF RAMMED AGGREGATE PIERS SHALL PROVIDE A MINIMUM EQUIVALENT UNIFORM BEARING PRESSURE OF 2,500 p.s.f. AT COLUMN FOOTINGS AND 1,900 p.s.f. AT WALL FOOTINGS.
3. ALL GRADING AND FILLING SHALL BE DONE AS RECOMMENDED BY A GEOTECHNICAL ENGINEER. GENERAL CONTRACTOR SHALL ESTABLISH AND MAINTAIN SITE DRAINAGE TO DIRECT WATER AWAY FROM FOOTING EXCAVATIONS AND FILL PLACEMENT.
4. ALL FOOTINGS SHALL BE POURED ON FIRM, UNDISTURBED EARTH OR ENGINEERED CONTROLLED BACKFILL. BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 18 INCHES BELOW FINISH GRADE UNLESS OTHERWISE NOTED, AND TOP OF FOOTING MUST COURSE WITH FINISH FLOOR. FROST LINE FOR THIS PROJECT IS 12 BELOW FINISH GRADE.

III. CONCRETE:

A. CONCRETE SHALL BE PROPORTIONED, MIXED AND PLACED IN ACCORDANCE WITH ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS."

B. CEMENT TO BE PORTLAND TYPE 1 OR APPROVED EQUAL.

C. MIX DESIGN SHALL BE DOCUMENTED IN ACCORD WITH SECTION 03300 OF THE PROJECT SPECS AND ACI 301, CHAPTER 3 "PROPORTIONING". MIX DESIGNS WHICH ARE SUBMITTED WITHOUT THE REQUIRED DOCUMENTATION WILL BE REJECTED. FIELD SLUMPS RECORDED AT JOB SITE SHALL NOT EXCEED THE SLUMP ESTABLISHED FOR THE MIX DESIGN.

D. CONCRETE SHALL HAVE AN ALLOWABLE COMPRESSIVE STRENGTH AS NOTED BELOW:

   INTERIOR SLABS ON GRADE F'c = 3,000 PSI
   FOUNDATIONS F'c = 3,000 PSI

E. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

   FOOTINGS 3"
   SLAB-ON-GRADE 3"

F. EXPOSED CONCRETE CORNERS SHALL BE CHAMFERED 3/4 INCH UNLESS NOTED.

G. CONCRETE SHALL NOT BE POURED IN WATER OR ON FROZEN GROUND AND SHALL BE PROTECTED FROM FROST DURING CONSTRUCTION.

H. CONTRACTOR SHALL COORDINATE ALL CONTRACT DRAWINGS FOR THE LOCATION OF ANCHOR BOLTS, FLOOR DRAINS, INSERTS, ETC., BEFORE POURING CONCRETE.

I. SLABS:

1. SLAB THICKNESS INDICATED ON DRAWINGS IS MINIMUM AND SHALL BE MEASURED FROM LOW POINT ON FLOOR. CONTRACTOR SHALL COORDINATE ALL DRAWINGS TO ASSURE THAT ALL FLOORS HAVE PROPER SLOPE TO DRAIN IN TOILETS, SHOWERS, ETC.
2. "C.J.", AS INDICATED ON SLAB, INDICATES 3/4" DEEP SAW CUT CONTROL JOINT OR KEYED CONSTRUCTION JOINT IN SLAB-ON-GRADE. MAKE CUTS WITHIN 12 HOURS AFTER CONCRETE PLACEMENT.

J. REINFORCEMENT:

1. ALL DETAILING, FABRICATION AND PLACEMENT OF REINFORCING STEEL, MIXING, HANDLING, PLACING, FINISHING AND CURING OF CONCRETE SHALL BE IN ACCORDANCE WITH ACI-315 AND ACI-318.
2. WIRE MESH REINFORCEMENT SHALL BE CENTERED IN SLAB, BY USE OF HIGH CHAIR WELDING WASHERS OR CONTINUOUS BEAM BOLSTERS.
3. WELDED WIRE FABRIC AND WIRE SHALL BE LAPPED THE SPACING OF THE CROSS WIRES PLUS 2".

K. CONCRETE MASONRY UNITS (CMU):

1. DOWEL ALL VERTICAL REINFORCEMENT FROM FOUNDATIONS. HOLD VERTICAL BARS PLUMB. PROVIDE A MINIMUM OF 1/2" GROUT BETWEEN MAIN REINFORCEMENT AND CMU.
2. HOLLOW CONCRETE BLOCK (MASONRY) UNITS SHALL CONFORM TO A.S.T.M. C90, LIGHTWEIGHT, TYPE N1 WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI ON THE NET AREA . SAMPLE AND TEST IN ACCORDANCE WITH ASTM C140 (f'm=1500 PSI)
3. ALL MORTAR FOR MASONRY SHALL CONFORM TO A.S.T.M. C270, TYPE M OR S. USE TYPE N MORTAR FOR BRICK AND INTERIOR NON-LOAD BEARING PARTITIONS. ALL GROUT FOR USE IN MASONRY SHALL CONFORM TO A.S.T.M. C476, 3000 PSI @ 28 DAYS. DETERMINE COMPRESSIVE STRENGTH OF GROUT IN ACCORDANCE WITH ASTM C1019.
4. HORIZONTAL JOINT REINFORCEMENT (HJR) IN MASONRY WALLS SHALL BE CONTINUOUS TRUSS TYPE WITH 3/16" SIDE RODS AND CROSS-TIES OF GALVANIZED COLD DRAWN MILD STEEL WIRE CONFORMING TO A.S.T.M. A82.
5. MASONRY IS TO BE LAID IN RUNNING BOND UNLESS OTHERWISE NOTED. ALL CORNERS AND INTERSECTING WALLS SHOULD INTERLOCK.
6. FOR HOT WEATHER CONSTRUCTION, IMPLEMENT THE FOLLOWING PROCEDURES WHEN THE AMBIENT TEMP. EXCEEDS CONDITIONS BELOW:
   (a)100°F OR
   (b)90°F w/WIND VELOCITY GREATER THAN 8 mph
   a. DO NOT SPREAD MORTAR BEDS MORE THAN 4'-0" AHEAD OF MASONRY.
   b. SET MASONRY UNITS WITHIN ONE MIN. OF SPREADING MORTAR.

IV. STRUCTURAL STEEL:

A. DESIGN, FABRICATION, & ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE MANUAL OF STEEL CONSTRUCTION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION NINTH EDITION UNLESS OTHERWISE MODIFIED ON THE DRAWINGS OR IN SPECIFICATIONS.

B. MATERIAL SHALL MEET THE REQUIREMENTS OF THE FOLLOWING SPECIFICATION UNLESS NOTED:

   STRUCTURAL STEEL - ASTM A992 (Fy=50 ksi)
   ROUND PIPE STEEL - ASTM A53 GRADE B (Fy=35ksi)
   TUBE STEEL - ASTM A500 GRADE B (Fy=46ksi)
   HIGH STRENGTH BOLTS - ASTM A325-86A, 3/4"Ø
   WELD STEEL - AWS CLASS E70

C. ALL SHOP CONNECTIONS SHALL BE WELDED OR MADE WITH HIGH STRENGTH BOLTS UNLESS NOTED SPECIFICALLY.

D. FIELD CONNECTIONS SHALL BE MADE WITH 3/4" HIGH STRENGTH BOLTS. FIELD WELDING WILL BE ALLOWED ONLY WHERE NOTED ON THE DRAWINGS AND DETAILS.

E. ALL HIGH STRENGTH BOLTED CONNECTIONS SHALL BE BEARING TYPE WITH THREADS INCLUDED IN THE SHEAR PLANE UNLESS NOTED.

F. ALL HIGH STRENGTH FIELD BOLTED CONNECTIONS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD AS SPECIFIED IN "THE ALLOWABLE STRESS DESIGN SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."

G. FRAMED BEAM CONNECTIONS SHALL DEVELOP THE REACTION SHOWN ON ENDS OF BEAMS ON STRUCTURAL PLANS. WHERE REACTIONS ARE NOT SHOWN THE CONNECTION SHALL DEVELOP ONE-HALF THE ALLOWABLE UNIFORM LOAD FOR LATERALLY SUPPORTED BEAMS AS SHOWN IN TABLES IN PART 2 OF THE AISC MANUAL.

H. ALL STRUCTURAL STEEL BELOW GRADE SHALL BE ENCASED WITH A MINIMUM OF 4" CONCRETE COVER OR PAINTED WITH A COAL TAR

V. WOOD FRAMING NOTES

A. WALL STUDS SHALL BE DOUBLED AT ALL ANGLES, CORNERS, AND AROUND ALL OPENINGS.

B. REFER TO PLAN FOR ALL SHEAR WALL LOCATIONS.

C. PROVIDE ALL BLOCKING AND FIRE STOPS REQUIRED BY THE BUILDING OFFICIAL.

D. UNLESS OTHERWISE NOTED, ALL TIMBER CONNECTIONS SHALL BE NAILED IN CONFORMANCE WITH THE 2009 INTERNATIONAL BUILDING CODE.

VI. FASTENERS:

EXCEPT WHERE SPECIFICALLY NOTED OTHERWISE ON DRAWINGS:

A. ALL POWDER ACTUATED FASTENERS (P.A.F.) TO BE .145" SHANK DIAMETER x 1 1/4" LONG HILTI X-DNI 32 P8 OR .177" SHANK DIAMETER x 1 1/4" LONG HILTI DS 37 P10, TYP. U.N.O.

B. ALL EXPANSION ANCHORS TO BE HILTI KWIK BOLT II
   5/8"Ø - MIN. EMBED = 4"
   3/4"Ø - MIN. EMBED = 4 3/4"
   1"Ø - MIN. EMBED = 6"

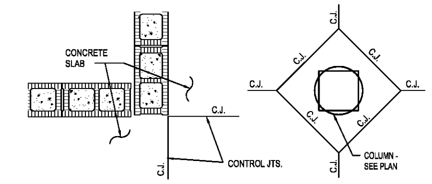
C. ALL SLEEVE ANCHORS TO BE HILTI CARBON STEEL SLEEVE ANCHORS.
   1/2"Ø - MIN. EMBED = 1 1/2"

D. ALL EPOXY ANCHORS TO BE HILTI HIT HY150 ADHESIVE ANCHORS.
   5/8"Ø - MIN. EMBED = 5"
   3/4"Ø - MIN. EMBED = 6 5/8"
   7/8"Ø - MIN. EMBED = 7 1/2"
   1"Ø - MIN. EMBED = 8 1/4"

E. ALL CONCRETE/MASONRY SCREWS TO BE HILTI KWIK-CON II+.
   3/16"Ø - MIN. EMBED = 1"
   1/4"Ø - MIN. EMBED = 1 3/4"

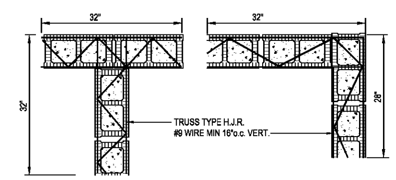
F. ALL DROP IN ANCHORS TO BE HILTI HDI

G. ALL FASTENERS ARE SIZED PER HILTI SPECIFICATIONS. ALL FASTENERS MAY BE SUBSTITUTED BY AN EQUIVALENT THAT MUST BE SUBMITTED TO THE ENGINEER FOR APPROVAL.

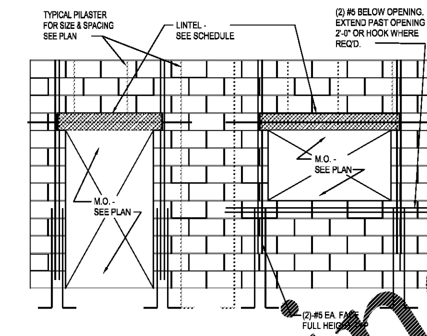


CONTROL JTS SHALL BE PLACED AT ALL CORNERS WHERE SLAB EDGES CHANGE DIRECTION AND ON 4 SIDES OF COLUMN LOCATIONS AS SHOWN ABOVE. INTERMEDIATE JOINTS SHALL BE PLACED SO THAT AREA DOES NOT EXCEED 400 SQ. FT. & RATIO OF SIDES SHALL NOT EXCEED 1.5:1. SEE SECTION 5, 5a OR 5b/S201.

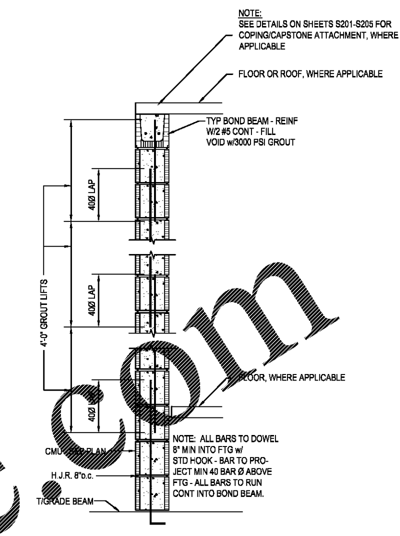
CONTROL/CONSTRUCTION JOINT PLACEMENT DETAIL



PREFABRICATED CORNERS & TEES



TYPICAL MASONRY OPENING (M.O.) REINFORCEMENT DETAIL



TYPICAL MASONRY WALL REINFORCEMENT DETAIL

MASONRY OPENING LINTEL SCHEDULE

OPENING WIDTH MIN	OPENING WIDTH MAX	LINTEL (EACH 4" WYTHE THICKNESS)	WALL DIMENSION AND REINFORCING (CONCRETE BLOCK & CONCRETE) DEPTH	4" WALL	8" WALL	12" WALL
——	[illegible]	L3"x3"x1/4"	7 5/8"	2 #5	2#5 BOTT	2#5 BOTT
2'-0"	3'-0"	L3"x3 1/2"x1/4"	7 5/8"	2 #5	2#5 BOTT	2#5 BOTT
3'-0"	5'-0"	L3"x3 1/2"x1/4"	7 5/8"	2 #5	2#5 BOTT	2#5 BOTT
5'-0"	6'-0"	L3 1/2"x4"x1/4"	7 5/8"	——	2#7 BOTT	2#5 BOTT
6'-0"	8'-0"	L3 1/2"x5"x1/4"	7 5/8"	——	2#8 BOTT	——
8'-0"	10'-0"	L3 1/2"x6"x5/16"	15 5/8"	——	2#8 BOTT	——

NOTE: DO NOT USE THIS SCHEDULE IF CONCENTRATED LOADS ARE APPLIED. PROVIDE 8" MIN BEARING EACH END.

COLUMN SCHEDULE

MARK	C1	C2	C3	C4	C5	C6
COLUMN	HSS3x3x1/4"	HSS4x4x1/4"	HSS5x5x1/4"	HSS5x5x5/16"	HSS8x8x3/8"	HSS4x4x1/4"
BASE PLATE	3/4" x 9" x 0'-9"	3/4" x 5" x 0'-10"	3/4" x 11" x 0'-11"	3/4" x 11" x 0'-11"	1 x 16" x 1'-4"	3/4" x 10" x 0'-10"
ANCHOR BOLTS	(4) 3/4"Ø F1554	(4) 3/4"Ø F1554	(4) 3/4"Ø F1554	(4) 7/8"Ø F1554	(8) 7/8"Ø F1554	(4) 3/4"Ø F1554
NOTES	SEE DETAIL C/S201	SEE DETAIL G/S201	SEE DETAIL C/S201	SEE DETAIL C/S201	SEE DETAIL H/S201	SEE DETAIL C/S20[illegible]

FOOTING SCHEDULE

MARK	F1	F2	F3	F4	F5
FOOTING	3'-0"x3'-0"x16 Dp	5'-6"x5'-6"x16 Dp	7'-0"x7'-0"x20" Dp	6'-0"x6'-0"x16" Dp	12'-0"x[illegible]" Dp
REINFORCEMENT	(4)#5's EACH WAY BOTTOM	(7) #5's EACH WAY BOTTOM	(8) #6's EACH WAY BOTTOM	[illegible] #5's EACH WAY BOTTOM	[illegible] LONG WAY TOP & BOTTOM (23) #5's SHORT WAY TOP & BOTTOM
NOTES					

WALL FOOTING SCHEDULE

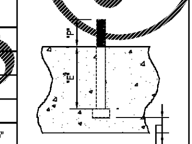
MK.	FOOTING SIZE	REBAR REQUIREMENTS CONTINUOUS	REBAR REQUIREMENTS TRANSVERSE	REMARKS
WF-1	2'-0" WIDE x 1'-0" Dp. CONT	(3) #5 BOTTOM	#5 @ 48"o.c. BOTTOM	THICKENED SLAB
WF-2	2'-0" WIDE x 1'-0" Dp. CONT	(3) #5 BOTTOM	#5 @ 48"o.c. BOTTOM	WALL FOOTING
WF-3	3'-0" WIDE x 1'-0" Dp. CONT	(4) #5 BOTTOM	#5 @ 32"o.c. BOTTOM	THICKENED SLAB
WF-4	3'-0" WIDE x 1'-0" Dp. CONT	(4) #5 BOTTOM	#5 @ 24"o.c. BOTTOM	WALL FOOTING

MINIMUM WOOD PROPERTIES

	DIMENSIONAL LUMBER							LAMINATED VENEER LUMBER (LVL)
	#2 SPRUCE-PINE-FIR (SPF)		#2 SOUTHERN YELLOW PINE (SYP)					
	2x4	2x6	2x4	2x6	2x8	2x10	2x12	
Fb	775 psi	775 psi	1,500 psi	1,250 psi	1,200 psi	1,050 psi	975 psi	2,925 psi
Fv	135 psi	135 psi	175 psi	175 psi	175 psi	175 psi	175 psi	285 psi
$Fc_{\parallel}$	1,000 psi	1,000 psi	1,650 psi	1,600 psi	1,550 psi	1,500 psi	1,450 psi	1600 psi
$Fc_{\perp}$	335 psi	335 psi	565 psi	565 psi	565 psi	565 psi	565 psi	750 psi
E	1,100,000 psi	1,100,000 psi	1,600,000 psi	1,600,000 psi	1,600,000 psi	1,600,000 psi	1,600,000 psi	2,000,000 psi

ANCHOR BOLT EMBEDMENTS SCHEDULE

DIAMETER	GRADE	"P"	"E"	"HK"
1/2"	F1554 (36KSI)	6"	10"	3"
5/8"	F1554 (36KSI)	6"	10"	3"
3/4"	F1554 (36KSI)	[illegible]	[illegible]	3"
7/8"	F1554 (36KSI)	[illegible]	11"	3.5"
[illegible]	F1554 (36KSI)	6"	12"	4"
1 1/8"	F1554 (36KSI)	6"	14"	4.5"
1 1/4"	F1554 (36KSI)	6"	15"	5"



HEADER SCHEDULE

MARK	H-1	H-2	H-3	H-4	H-5
SIZE	(3) 2x8 #2 SYP	(3) 2x12 #2 SYP	(3) 2x12 #2 SYP	(3) 2x8 #2 SYP	(3) 2x8 #2 SYP
JACK STUD	(1) 2x6	(1) 2x6	(2) 2x6	(1) 2x6	(1) 2x6
KING STUD	(1) 2x6	(1) 2x6	(1) 2x6	(2) 2x6	(1) 2x6

1. JACK SIZE INDICATED TO BE USED UNLESS NOTED OTHERWISE ON PLAN.
2. SEE DETAIL A/S202 FOR TYPICAL HEADER.

STUD WALL FRAMING SCHEDULE

LEVEL	INTERIOR WALLS BETWEEN ROOMS	EXTERIOR END WALLS	EXTERIOR FRONT AND REAR WALLS	BEARING WALLS @ CORRIDOR	ALL NON-LOAD BEARING WALLS
4th	2x6 @ 16"o.c. #2 SPF	2x6 @ 16"o.c. #2 SPF	2x6 @ 16"o.c. #2 SPF	2x6 @ 16"o.c. #2 SPF	2x6 @ 16"o.c. AND 2x4 @ 16"o.c. SEE ARCH. PLANS
3rd	2x6 @ 16"o.c. #2 SPF	2x6 @ 16"o.c. #2 SPF	2x6 @ 16"o.c. #2 SPF	2x6 @ 16"o.c. #2 SPF	
2nd	2x6 @ 16"o.c. #2 SPF	2x6 @ 16"o.c. #2 SPF	2x6 @ 16"o.c. #2 SPF	2x6 @ 16"o.c. #2 SPF	
1st	2x6 @ 16"o.c. #2 SYP	2x6 @ 16"o.c. #2 SYP	2x6 @ 16"o.c. #2 SYP	2x6 @ 16"o.c. #2 SPF	

1. TOP AND BOTTOM PLATES SHALL CONSIST OF LVL MATERIAL.

WALL SHEATHING SCHEDULE

MARK	LEVEL	SHEATHING	REMARKS
W-1	4th, 3rd, 2nd, 1st	19/32" OSB SHEATHING ON EXTERIOR FACE OF WALL ONLY 5/8" GWB SHEATHING ON INTERIOR FACE	ALL EXTERIOR WALLS
W-2	4th, 3rd, 2nd, 1st	19/32" OSB SHEATHING + 5/8" GWB SHEATHING BOTH FACES OF WALL	INTERIOR SHEAR WALLS
W-3	4th, 3rd, 2nd, 1st	5/8" GWB SHEATHING ON BOTH FACES OF WALL	INTERIOR NON-SHEAR WALLS

FASTENER NOTES:

1. 1st FLOOR INTERIOR WALLS @ BOTTOM PLATE - 5/8"Ø EXPANSION BOLTS @ EACH SIDE OF DOORS, WINDOWS, CORNERS AND/OR @ 48" o.c. MAX
2. 1st FLOOR EXTERIOR WALLS @ BOTTOM PLATE -5/8"Ø EPOXY BOLTS @ @ EACH SIDE OF DOORS, WINDOWS, CORNERS AND/OR @ 48"o.c. MAX
3. 19/32" OSB SHEATHING - 10d NAILS @ 4" o.c. @ PANEL EDGES AND @ 12" o.c. @ PANEL FIELD PER STUD. PANELS TO BE INSTALLED w/LONGER SIDE RUNNING HORIZONTALLY
4. WHERE OSB SHEATHING IS REQUIRED ONE BOTH FACES OF A WALL, THE VERTICAL SEAMS ON EITHER SIDE SHALL NOT OCCUR ON THE SAME STUD.
5. GYPSUM PANELS ORIENTED VERTICALLY w/ BREAK @ STUDS w/ #6x1¼" SCREWS @ 8" o.c. @ PANEL EDGES AND 12" o.c. @ PANEL FIELD PER STUD

LEGEND

SECTION CUT NUMBER OR DETAIL LETTER
DENOTES DRAWING WHERE SECTION OR DETAIL IS CUT.
DENOTES DRAWING WHERE SECTION OR DETAIL IS SHOWN.

March Adams & Associates
310 Divide Ave.
P.O. Box 1859
Chattanooga, Tennessee 37404
PH: (423) 698-6675
Consulting Engineers

HAMPTON INN
NEW HOTEL
ASHLAND CITY, TENNESSEE

WWW • DAVIDRCARTER • NET
DAVID R. CARTER AND ASSOCIATES

ISSUE DATE 05-30-19
▲REVISION LOG

PROJECT 00755

STRUCTURAL SCHEDULES & NOTES S001