

STRUCTURAL NOTES:

A. CONTRACTORS NOTES:

- THE CONTRACT DOCUMENTS REPRESENT THE INTENT OF THE CONSTRUCTION. THE PROCEDURES AND RISKS ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE STRUCTURAL INTEGRITY OF THE BUILDING SHALL BE MAINTAINED AT ALL TIMES.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATIONS OF ALL DIMENSIONS AND ELEVATIONS. ANY DISPARITY SHALL BE REPORTED TO THE ARCHITECT/ENGINEER PRIOR TO THE BEGINNING OF WORK.
- ANY CHANGES MADE BY THE CONTRACTOR SHALL BE APPROVED BY THE ARCHITECT/ENGINEER PRIOR TO THE BEGINNING OF WORK.

B. DESIGN CRITERIA

APPLICABLE CODE: 2018 NORTH CAROLINA STATE BUILDING CODE
THIS STRUCTURE HAS BEEN DESIGNED ACCORDING TO THE FOLLOWING:
BUILDING RISK CATEGORY: III

LATERAL LOADS:

1. ROOF DEAD LOAD	16 P.S.F.
2. ROOF LIVE LOAD	20 P.S.F.
3. GROUND SNOW LOAD	10 P.S.F.
4. MWFRS NET WIND UPLIFT	10 P.S.F.
5. MECHANICAL EQUIPMENT LOADS AND LOCATIONS USED FOR DESIGN ARE AS INDICATED ON THE STRUCTURAL DRAWINGS. ACTUAL LOADS INDICATED ON THE SHOP DRAWINGS.	

LATERAL LOADS:

I. WIND:	
ULTIMATE WIND SPEED:	120 M.P.H.
NOMINAL WIND SPEED:	90 M.P.H.
WIND EXPOSURE:	'B'
WIND IMPORTANCE FACTOR: $I_w = 1.00$	
INTERNAL PRESSURE COEFFICIENT: ± 0.18	

2. SEISMIC:

SEISMIC IMPORTANCE FACTOR:	$I_w = 1.25$	BASIC SEISMIC-FORCE RESISTING SYSTEM:	
SEISMIC USE GROUP:	III	LIGHT FRAMED WALLS W/	
SEISMIC SITE CLASSIFICATION:	C	WOOD STRUCTURAL PANELS	
SEISMIC CATEGORY:	D	DESIGN BASE SHEAR:	0.049W
$S_s = 0.237g$	$S_1 = 0.102g$	SEISMIC RESPONSE COEFFICIENT:	0.049
$S_{0.2} = 0.253g$	$S_{0.6} = 0.162g$	RESPONSE MODIFICATION FACTOR:	6.5
		ANALYSIS PROCEDURE USED:	EQUIVALENT LATERAL FORCE PROCEDURE

C. FOUNDATION AND BACKFILLING

- ALLOWABLE SOIL BEARING CAPACITY DESIGNED AT 2000 P.S.F. PER GEOTECHNICAL ENGINEERING REPORT NUMBER 71185152 BY TERRACON CONSULTANTS, INC. DATED NOVEMBER 19, 2018.
- THE BOTTOM OF EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 18" BELOW LOWEST EXTERIOR GRADE FOR FROST PROTECTION.
- WHERE FILL IS REQUIRED IT SHALL BE PLACED IN ACCORDANCE WITH THE INSTRUCTIONS AND UNDER THE SUPERVISION OF A QUALIFIED GEOTECHNICAL ENGINEER.
- FOOTINGS MAY BEAR DIRECTLY ON THE SOIL AT THE LOWER LEVEL, OR ON LEAN BACKFILL PLACED IN EXCAVATIONS. THE FOOTINGS SHOULD ALSO BEAR ON PROPERLY COMPACTED, ENGINEERED FILL, EXTENDING DOWN TO SUITABLE SOIL (SEE GEOTECHNICAL REPORT).
- FOOTINGS MAY BE POURED INTO EARTH-FORMED TRENCH ONLY IF SOIL CONDITIONS PERMIT.

D. CAST IN PLACE CONCRETE

- CONCRETE FOR FOOTINGS SHALL BE 3000 P.S.I. COMPRESSIVE STRENGTH IN 28 DAYS. ALL OTHER CONCRETE SHALL BE 4000 P.S.I. STRENGTH.
- PROVIDE TEST MATERIAL FOR CONCRETE MATERIAL AND MIX DESIGN TEST.
- REINFORCEMENT SHALL BE DEFORMED STEEL AND CONFORM TO ASTM A615 Fy=60 K.S.I. WELDED WIRE FABRIC SHALL BE ASTM A185
- CONTINUOUS SPLICES, VERTICAL OR HORIZONTAL, SHALL BE 40 BAR DIAMETER. SPLICES SHALL BE CONTINUOUS OR SPLICED WITH DOWELS.
- BAR CLEARANCES SHALL BE:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3"
CONCRETE EXPOSED TO EARTH OR ELEMENTS	2"
#6-#18 BAR	1 1/2"
#5 OR SMALLER	1"
CONCRETE NOT EXPOSED TO ELEMENTS OR EARTH SLABS/WALLS	3/4"
#11 OR SMALLER	1"
BEAMS, COLUMNS, PRIMARY REINFORCEMENT TIES AND STIRRUPS	1 1/2"
- PRIOR TO FABRICATION, SUBMIT SHOP FABRICATION DRAWINGS FOR BENDING AND PLACEMENT OF CONCRETE REINFORCEMENT. COMPLY WITH ACI 315 "MANUAL OF STANDARD PRACTICE FOR STANDARD DETAILING REINFORCED CONCRETE STRUCTURES" SHOWING BAR SCHEDULES, STIRRUP SPACING, DIAGRAMS OF BENT BARS, ARRANGEMENT OF CONCRETE REINFORCEMENT. INCLUDE SPECIAL REINFORCEMENT REQUIRED AND OPENINGS THROUGH CONCRETE.

E. STRUCTURAL STEEL

- STRUCTURAL STEEL SHALL COMPLY WITH ASTM A572 UNLESS NOTED.
- ANCHOR BOLTS SHALL COMPLY WITH A307.
- ALL STRUCTURAL STEEL BEAM CONNECTION BOLTS SHALL BE HIGH STRENGTH ASTM A325 3/4" DIAMETER UNLESS NOTED.
- WELDS SHALL BE OF E70XX RODS AND CURRENT AWS CODE.
- PIPE COLUMNS SHALL BE ASTM A53, Fy=35 K.S.I. GRADE B
- TUBE COLUMNS SHALL BE ASTM A500, Fy=46 K.S.I. GRADE B
- AFTER THE ERECTION OF THE COLUMNS AND INSTALLATION OF A NON SHRINK GROUT, ALL EXPOSED STEEL SHALL BE COATED BELOW FINISHED FLOOR ELEVATION WITH ONE COAT OF ASPHALT MASTIC COMPLYING WITH SSPC-PAINT 12.X.
- THE STEEL STRUCTURE IS LATERALLY UNSTABLE AND IS DEPENDENT ON DIAPHRAGM ACTION OF THE METAL ROOF DECK AND ATTACHMENT TO THE MASONRY WALL SYSTEM FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES. PROVIDE ALL TEMPORARY SUPPORTS REQUIRED FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES UNTIL THESE ELEMENTS ARE FULLY SECURED TO EACH OTHER AND CAPABLE OF PROVIDING THIS SUPPORT.

F. WELDING:

- ALL STRUCTURAL STEEL WELDING SHALL CONFORM TO AMERICAN WELDING SOCIETY STANDARD D1.1, LATEST EDITION.
- REINFORCING STEEL WELDING SHALL CONFORM TO A503. REINFORCING SHALL CONFORM TO ASTM A-706.
- ALL STRUCTURAL STEEL AND REINFORCING STEEL WELDING ELEMENTS SHALL CONFORM TO AWS A5.1 OR A5.5 E-70XX.
- FIELD WELDING SHALL BE LIMITED TO CONNECTIONS.
- ALL EXPOSED WELDED JOINTS SHALL BE GRIND TO SMOOTH AND SUBJECT TO ARCHITECT APPROVAL. THE CONTRACTOR SHALL SUBMIT WRITTEN PROCEDURE AND DETAILS TO THE ARCHITECT FOR REVIEW PRIOR TO BEGINNING THE WORK.

G. STEEL JOISTS:

- STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE "STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS," "K-SERIES" LATEST EDITION, BY THE STEEL JOIST INSTITUTE (SJI).
- STEEL JOIST BEARING AT EXTERIOR WALLS SHALL BE DESIGNED TO RESIST 1000 POUNDS LATERAL FORCE PERPENDICULAR TO THE JOIST AND APPLIED AT THE TOP OF THE JOIST BEARING. STIFFENERS AT THE JOIST BEARING SHALL BE ADDED IF REQUIRED.
- STEEL JOIST BRIDGING SHALL BE PROVIDED IN ACCORDANCE WITH THE SJI SPECIFICATION. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE PLACED AND STEEL JOIST ENDS FIXED PRIOR TO THE APPLICATION OF ANY LOADS. BRIDGING THAT TERMINATES AT, OR IS INTERRUPTED BY, STRUCTURAL STEEL BEAMS, MASONRY WALLS OR CONCRETE WALLS SHALL BE ATTACHED THERETO. COORDINATE BRIDGING LOCATIONS TO AVOID INTERFERENCE WITH ALL MECHANICAL, ELECTRICAL AND FIRE PROTECTION EQUIPMENT.
- UNLESS NOTED OTHERWISE, K-SERIES JOISTS SHALL BE ATTACHED TO SUPPORTING STEEL WORK OR STEEL BEARING PLATES WITH (2) 1/8" FILLET WELD (ONE EACH SIDE) 1" LONG, WITH (2) 1/2" DIAMETER BOLT, OR WITH A COMBINATION OF (1) 1/8" FILLET WELD AND (1) 1/2" DIAMETER BOLT.
- HOLES IN STEEL JOIST CHORDS WILL NOT BE PERMITTED, EXCEPT FOR BOLTED CONNECTIONS AT THE BEARING END OF THE STEEL JOIST.
- ALL ITEMS SUCH AS MECHANICAL EQUIPMENT, DUCT WORK, PIPES, CEILING, FIXTURES, ETC. THAT ARE TO BE SUPPORTED OR HUNG FROM THE STEEL JOIST SHALL BE FRAMED WITH AUXILIARY FRAMING TO THE PANEL JOINTS OF THE STEEL JOIST. METHODS OF FRAMING THAT INDUCE BENDING TO THE STEEL JOIST CHORDS OR WEB MEMBERS WILL NOT BE PERMITTED.
- THE CONTRACTOR SHALL VERIFY BY A CERTIFIED LETTER TO THE ARCHITECT, THAT THE FINAL LOCATION OF THE SPRINKLER SYSTEM MAIN BRANCH LINES MATCHES THE LOCATION SHOWN ON FIRE PROTECTION DRAWINGS. ANY DEVIATIONS FROM THIS SPECIFIED DRAWING MUST BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER IN WRITING. THE COST OF ADDITIONAL SUPPORTS DUE TO DEVIATIONS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- PLACEMENT OF SPRINKLER CONTRACTORS PIPING AND STEEL CONTRACTORS JOIST BRIDGING SHALL BE COORDINATED BY THE GENERAL CONTRACTOR. THE GENERAL CONTRACTOR SHALL MAKE PROVISIONS FOR THE STEEL ERECTOR TO ADJUST/ RELOCATE BRIDGING OR SPRINKLER CONTRACTOR TO ADJUST OR RELOCATE PIPING WHERE CONFLICTS OCCUR. ALL CHANGES REQUIRED OF THE JOIST BRIDGING SHALL BE PER SJI REQUIREMENTS.
- SPRINKLER PIPE SUPPORT CRITERIA:
 - IF LARGER THAN 6 INCH DIAMETER PIPING IS USED, THE STRUCTURAL ENGINEER OF RECORD MUST BE NOTIFIED.
 - MAXIMUM SPACING OF SUPPORTS FOR 6 INCH DIAMETER PIPES SHALL BE 12'-6", BUT NO MORE THAN TWO JOIST SPACES FOR PIPES PERPENDICULAR TO JOISTS.
 - PIPES SPANNING PERPENDICULAR TO STEEL JOIST:
 - ALL 6 INCH AND 8 INCH DIAMETER PIPES ADJACENT TO EACH OTHER SHALL BE SUPPORTED FROM ALTERNATE STEEL JOIST.
 - PIPES SPANNING PARALLEL TO THE STEEL JOIST:
 - 6 INCH DIAMETER PIPE HANGERS SHALL NOT BE PLACED CLOSER THAN 12 INCHES TO ANY STEEL JOIST.

H. METAL ROOF DECK:

- METAL ROOF DECK SHALL COMPLY WITH THE REQUIREMENTS OF THE STEEL DECK INSTITUTE "SPECIFICATIONS AND COMMENTARY FOR STEEL ROOF DECK" (LATEST EDITION).
- METAL ROOF DECK SHALL BE CONFIGURATION, DEPTH AND MINIMUM GAGE AS SHOWN ON THE DRAWINGS. MINIMUM YIELD STRENGTH SHALL BE 33,000 PSI. ATTACHMENT TO THE SUPPORTING STRUCTURE SHALL BE AS SHOWN ON THE DRAWINGS. SEE ROOF PLAN NOTES.
- STEEL ROOF DECK AND CONNECTORS SHALL CONFORM TO FACTORY MUTUAL PUBLICATION I-28 AND I-28S, CLASS I-60, WINDSTORM RESISTANCE. INSTALLATION SHALL BE BY AN OSHA CERTIFIED OPERATOR IN ACCORDANCE WITH THE FASTENER MANUFACTURER'S INSTRUCTIONS AND INITIAL FASTENING UNDER THE SUPERVISION OF THE MANUFACTURER'S REPRESENTATIVE. STEEL ROOF DECK SHALL BE ATTACHED AS FOLLOWS:
 - AT SUPPORTS: 5/8" DIAMETER WELDS.
 - AT SIDELAPS: BUILDEX TRAXX #10-14 STANDARD HEX WASHER HEAD SCREWS.
- DO NOT HANG OR SUPPORT ANY LOADS FROM METAL ROOF DECK.
- METAL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS.
- AT ANGLE FRAMING AT OPENINGS, ADD ADDITIONAL SPLICES AS REQUIRED TO ACHIEVE PROPER BEARING ON THE SUPPORT WITHOUT FORCING THE DECK TO BEAR ON THE SUPPORT.

I. SHEATHING

- STRUCTURAL PANEL ON WALL SHALL BE 1/2" APA RATED INSULATED STRUCTURAL PANEL (OSB BOARD) EXPOSED TO EXTERIOR GRADE APA RATED WITH THE CLASS SHOWN ON PLANS. PANEL SHALL BE NAIL WITH 8D COMMON NAILS @ 6" O.C. AT EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS UNLESS NOTED.
- STRUCTURAL PANEL ON ROOF SHALL BE 1/2" T&G APA RATED INSULATED STRUCTURAL PANEL (PLYWOOD/OSB BOARD) EXPOSED TO EXTERIOR GRADE. PANEL SHALL BE NAIL WITH 8D COMMON NAILS AT 6" O.C. AT EDGES, AT 12" O.C. AT INTERMEDIATE SUPPORTS, AND AT 6" O.C. AT DIAPHRAGM BOUNDARY UNLESS NOTED OTHERWISE.
- ROOF PANEL SHALL BE PLACED WITH FACE GRAIN ACROSS RAFTERS AND STAGGERED SO CONTINUOUS PANEL JOISTS OCCUR ONLY IN ONE DIRECTION, PERPENDICULAR TO THE SPAN OF THE RAFTER. PROVIDE TONGUE AND GROOVE SHEATHING OR BLOCKING.

K. FRAMING LUMBER

- ALL LUMBER SHALL BE GRADED IN ACCORDANCE WITH AF&PA STANDARDS & NDS 2005.
- STUDS SHALL BE SOUTHERN YELLOW PINE OR DOUGLAS FIR LARCH OR APPROVED EQUAL STUD GRADE AS NOTED ON PLANS AT 19% MAX. MOISTURE CONTENT.

S.Y.P. #1	Fy = 1000 P.S.I.	E = 1,400,000 P.S.I.
D.O.G. #1	Fb = 900 P.S.I.	E = 1,600,000 P.S.I.
- RAFTERS, COLUMNS, AND JOISTS SHALL BE SOUTHERN YELLOW PINE OR DOUGLAS FIR LARCH OR APPROVED EQUAL JOIST/RAFTER GRADE (AS NOTED ON PLANS) AT 19% MAX. MOISTURE CONTENT.

S.Y.P. #2	Fy = 925 P.S.I.	E = 1,400,000 P.S.I.
D.O.G. #2	Fb = 900 P.S.I.	E = 1,600,000 P.S.I.
- WOOD CONNECTORS SHOWN ON THE DRAWINGS SHALL BE SIMPSON STRONG-TIE CONNECTORS AS MANUFACTURED BY THE SIMPSON CORPORATION. ALL CONNECTORS EXPOSED TO THE WEATHER SHALL BE GALVANIZED.
- ALL SHEATHING & DRYWALL PANEL EDGES SHALL BE SUPPORTED WITH MIN. 2x6 BLOCKING TYP.
- ALL FRAMING LUMBER EXPOSED TO WEATHER SHALL BE PRESSURE TREATED.

L. COLD-FORMED METAL FRAMING (METAL STUDS):

- METAL STUDS SHOWN ON THE DRAWINGS HAVE BEEN SPECIFIED USING AMERICAN IRON & STEEL INSTITUTE (AISI) STANDARD DESIGNATIONS.
- DESIGN, FABRICATIONS AND ERECTION SHALL CONFORM TO AISI SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS, LATEST EDITION. THE CONTRACTOR SHALL SUBMIT FABRICATION DRAWINGS FOR REVIEW SIGNED AND SEALED BY A PROFESSIONAL STRUCTURAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.
- GALVANIZED MATERIALS:
 - ALL GALVANIZED STUDS AND ACCESSORIES 12, 14 AND 16 GAUGE, SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE MINIMUM REQUIREMENTS OF A.S.T.M. A653, GRADE D WITH A MINIMUM YIELD OF 50,000 PSI.
 - ALL GALVANIZED 18 AND 20 GAUGE STUDS, TRACK, BRIDGING, END CLOSURES AND ACCESSORIES SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF A.S.T.M. A653, GRADE A WITH A MINIMUM YIELD OF 33,000 PSI.
 - ALL GALVANIZED STUDS, TRACK, BRIDGING AND ACCESSORIES SHALL BE FORMED FROM STEEL HAVING A G-60 GALVANIZED COATING MEETING THE REQUIREMENTS OF A.S.T.M. A653.
- UNLESS NOTED, ALL SCREWS OR PINS SHALL BE NON-CORROSIVE NO. 8-18 (D=125") OR LARGER. (DO NOT USE STAINLESS STEEL OR COPPER COATED FASTENERS). UNLESS NOTED, ALL PINS FOR STUDS (1/4 GAUGE TO 22 GAUGE) ATTACHMENT TO SHEATHING SHALL BE 0.1 INCH DIAMETER STEEL BY ERICO TOOL & FASTENERS, INC.
- UNLESS NOTED, TRACKS SHALL BE SAME DEPTH AS JOISTS AND EQUAL OR THICKER GAUGE THAN JOISTS. TRACKS SHALL BE CONNECTED TO SUPPORTS AT 16" O.C. MAXIMUM. STUDS SHALL BE CONNECTED TO TRACKS AT EACH SIDE.
- THE QUANTITY OF STUDS DISPLACED OR CUT FOR OPENING SHALL BE PLACED HALF ON EACH SIDE OF OPENING.
- BRIDGING FOR WALL STUDS SHALL BE PER MANUFACTURER RECOMMENDATIONS.

M. MASONRY

- REINFORCED MASONRY SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH, F'M, OF 1500 PSI. MASONRY UNITS SHALL BE NORMAL WEIGHT BLOCK CONFORMING TO ASTM C90, GRADE N, AND SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 900 PSI. MORTAR SHALL CONFORM TO ASTM C270, TYPE S, U.N.O. GROUT SHALL CONFORM TO ASTM C476. GROUT SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2000 PSI AND MAXIMUM DENSITY OF 115 PCF. SLUMP AT POINT OF PLACEMENT SHALL BE 9" ± 1".
- REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
- CONTINUOUS WIRE REINFORCING (JOINT REINFORCING) SHALL BE GALVANIZED TRUSS OR LADDER TYPE FORMED FROM 9 GAUGE COLD-DRAWN STEEL WIRE COMPLYING WITH ASTM A82. JOINT REINFORCING SHALL BE SPACED AT 16" O.C. VERTICALLY IN ALL MASONRY WALLS.
- ALL REINFORCED CELLS, ALL CELLS BELOW GRADE AND ALL CELLS BELOW FINISH FLOOR SHALL BE GROUTED SOLID.
- WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL BLOCK CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN 6 VERTICAL. DOWELS MAY BE GROUTED INTO A CELL IN VERTICAL ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING.
- REINFORCING STEEL SHALL BE SECURED IN PLACE BEFORE GROUTING STARTS.
- SPLICED REINFORCING SHALL BE LAPPED 48 BAR DIAMETERS OR 24 INCHES, WHICHEVER IS GREATER. SPLICED BARS SHALL BE WIRED TOGETHER.
- VERTICAL BARS SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 200 DIAMETERS OF THE REINFORCING, NOR 10 FEET. BARS SHALL BE IN PLACE PRIOR TO GROUTING.
- VERTICAL REINFORCING BARS SHALL HAVE A MINIMUM CLEARANCE OF 1/4 OF AN INCH FROM THE MASONRY FOR FINE GROUT AND 1/2 INCH FOR COARSE GROUT.
- VERTICAL CELLS THAT WILL BE GROUTED SHALL HAVE A VERTICAL ALIGNMENT TO MAINTAIN A CONTINUOUS UNOBSTRUCTED CELL AREA NOT LESS THAN 3/4".
- GROUTING SHALL BE STOPPED 1/2" BELOW THE TOP OF A COURSE SO AS TO FORM A KEY AT THE POOR JOINT.
- ALL BOLTS, ANCHORS, ETC., INSERTED IN THE WALLS, SHALL BE GROUTED SOLID INTO POSITION.

N. MISCELLANEOUS:

- THESE GENERAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- STRUCTURAL DRAWINGS ARE INTENDED TO BE USED WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL & FIRE SPRINKLER DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING SUCH REQUIREMENTS INTO THEIR SHOP DRAWINGS AND CONSTRUCTION.
- NO OPENINGS SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE PROFESSIONAL OF RECORD.
- NO CHANGE IN SIZE, MATERIAL OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE PROFESSIONAL OF RECORD.
- OPENINGS 1'-4" AND LESS ON A SIDE ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SUCH OPENINGS.
- THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
- THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND SUPPORTS THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND SEQUENCES.
- DO NOT SCALE THESE DRAWINGS FOR THESE DIMENSIONS.
- CONTRACTOR CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
- THE CONTRACTOR SHALL INFORM THE PROFESSIONAL OF RECORD IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY OF SUCH DEVIATION BY THE PROFESSIONAL OF RECORD REVIEW OF SHOP DRAWINGS, PRODUCT DATA, ETC., UNLESS THE CONTRACTOR HAS SPECIFICALLY INFORMED THE PROFESSIONAL OF RECORD OF SUCH DEVIATION AT THE TIME OF SUBMISSION, AND THE PROFESSIONAL OF RECORD HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.
- CONNECTIONS OF ALL ITEMS SUPPORTED BY THE STRUCTURE ARE THE RESPONSIBILITY OF THE DISCIPLINES WHO ARE MAKING THESE ATTACHMENTS. THESE ATTACHMENTS SHALL BE DESIGNED TO RESIST ALL GRAVITY, WIND, WIND SIFT, THERMAL LOADS, ECT.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS.
- UNLESS NOTED, SUBMIT SHOP DRAWINGS OF ALL FABRICATED MATERIALS FOR REVIEW. DESIGN DRAWINGS SHALL NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS. SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS THEY WERE CHECKED, BEAR THE INITIAL OF THE CHECKER AND ARE STAMPED "APPROVED" BY THE GENERAL CONTRACTOR.

O. SPECIAL INSPECTIONS: - UNLESS NOT REQUIRED AS WARRANTED BY CONDITIONS IN THE JURISDICTION AS APPROVED BY THE BUILDING OFFICIAL:

- SPECIAL INSPECTOR (SI) SHALL BE RETAINED AND PAID BY THE OWNER.
 - SPECIAL INSPECTOR SHALL BE FULLY QUALIFIED, APPROVED BY THE BUILDING OFFICIAL, REGISTERED BY APPLICABLE REGISTRATION BOARD IF REQUIRED AND ACCEPTABLE TO THE ARCHITECT.
 - THE DUTIES OF THE SPECIAL INSPECTOR SHALL INCLUDE, BUT ARE NOT LIMITED TO, VERIFICATION OF CONSTRUCTION QUALITY CONTROL, TESTING, COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS, BUILDING CODE REQUIREMENTS, AND LOCAL BUILDING DEPARTMENT REQUIREMENTS.
 - IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE PROPER NOTIFICATION TO THE SPECIAL INSPECTOR AND PROCEED WITH THE CONSTRUCTION ONLY AFTER THE SPECIAL INSPECTOR'S APPROVAL.
- SPECIAL INSPECTORS SHALL KEEP RECORDS OF ALL INSPECTIONS AND TESTING. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE CODE OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL OF RECORD. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CODE OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL OF RECORD. A FINAL REPORT OF INSPECTIONS DOCUMENTING COMPLETION AND COMPLIANCE OF ALL REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY. INTERIM REPORTS SHALL BE SUBMITTED PERIODICALLY WITH MINIMUM FREQUENCY OF TWO WEEKS.
- SPECIAL INSPECTIONS ARE REQUIRED FOR, BUT NOT LIMITED TO, THE FOLLOWING ACTIVITIES:
 - SLAB ON GRADE CONCRETE AND REINFORCING. VERIFICATION IS REQUIRED FOR PROPER LOCATION OF SLAB-ON-GRADE REINFORCING AND USAGE OF PROPER REINFORCING SUPPORTS.
 - MASONRY AND REINFORCING
 - FOOTING EXCAVATION AND FILL (GEOTECHNICAL ENGINEER CERTIFICATIONS IS REQUIRED PRIOR TO POURING CONCRETE).
 - D. ALL FIELD WELDING.
 - ALL HIGH STRENGTH BOLTING, ANCHORING SYSTEMS & POST-INSTALLED CONCRETE ANCHORS.
 - METAL ROOF DECK WELDS AND ATTACHMENTS.
- FAILURE TO NOTIFY THE SPECIAL INSPECTOR MAY RESULT IN CONTRACTOR HAVING TO REMOVE WORK FOR THE PURPOSE OF INSPECTION AT CONTRACTOR'S EXPENSE.
- PREMATURE NOTIFICATION FOR INSPECTIONS MAY RESULT IN AN ADDITIONAL INSPECTION WITH ALL EXPENSES AND FEES PAID BY THE CONTRACTOR.

ABBREVIATIONS		E.J.	EXPANSION JOINT	P.S.F.	POUNDS PER SQUARE FOOT
AB	ANCHOR BOLT	ELEV.	ELEVATION	P.S.I.	POUNDS PER SQUARE INCH
ANCH.	ANCHOR	E.W.	EACH WAY	REINF.	REINFORCED
ARCH.	ARCHITECT/ARCHITECTURAL	EXIST.	EXISTING	REQ'D.	REQUIRED
BLDG.	BUILDING	EXP.	EXPANSION	SECT.	SECTION
BLKG.	BLOCKING	EXT.	EXTERIOR	SIM.	SIMILAR
BM	BEAM	FDN.	FOUNDATION	STL.	STEEL
BOT.	BOTTOM	F.F.E.	FINISHED FLOOR ELEVATION	STRUCT.	STRUCTURAL
BRT.	BEARING	FRMG.	FRAMING	T.B.E.	TRUSS BEARING ELEVATION
CJ	CONTROL JOINT/ CONSTRUCTION JOINT	FV.	FIELD VERIFY	T.F.E.	TOP OF FOOTING ELEVATION
CL	CENTER LINE	FTG.	FOOTING	T.S.E.	TOP OF STEEL ELEVATION
C.M.U.	CONCRETE MASONRY UNIT	HORIZ.	HORIZONTAL	TYP.	TYPICAL
COL.	COLUMN	J.B.E.	JOIST BEARING ELEVATION	VERT.	VERTICAL
CONC.	CONCRETE	JT.	JOINT	W/	WITH
CONT.	CONTINUOUS	L.L.	LIVE LOAD	WWF	WELDED WIRE FABRIC
DIA.	DIAMETER	L.F.H.	LONG LEG HORIZONTAL		
D.L.	DEAD LOAD	L.L.V.	LONG LEG VERTICAL		
DWG.	DRAWING(S)	MAX	MAXIMUM		
E.A.	EACH	MFCR.	MANUFACTURER		
E.F.	EACH FACE	MIN.	MINIMUM		
E.J.	EXPANSION JOINT	M.O.	MASONRY OPENINGS		
		O.C.	ON CENTER		
		P	PLATE		

GreenbergFarrow

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ISSUE/REVISION RECORD

DATE	DESCRIPTION
05/28/19	COORDINATION SET
02/20/19	HEALTH SUBMITAL
04/24/19	HEALTH RESPONSE
05/03/19	PERMIT/BD SET

PROFESSIONAL SEAL

PROFESSIONAL IN CHARGE

B. BORNHORN

PROJECT MANAGER

M. HWS

QUALITY CONTROL

M. HWS

DRAWN BY

C. YLA

PROJECT NAME

BUBBA'S 33

GASTONIA NORTH CAROLINA

3287 E. FRANKLIN BLVD



PROJECT NUMBER

20181046.0

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

S1