GENERAL NOTES

I. GENERAL
A. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, SHOP DRAWINGS AND SPECIFICATIONS.
B. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO OBTAIN ALL CONTRACT DOCUMENTS AND LATEST ADDENDA AND TO SUBMIT TO ALL SUBCONTRACTORS AND SUPPLIERS PRIOR TO THE SUBMITTAL OF SHOP DRAWINGS.
C. THE GENERAL CONTRACTOR SHALL COMPARE ALL CONTRACT DRAWINGS AND REPORT ANY DISCREPANCY BETWEEN DISCIPLINES AND WITHIN A GIVEN DISCIPLINE TO THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION AND ERECTION.
D. IF A CONFLICT EXISTS AMONG THE STRUCTURAL DRAWINGS, GENERAL NOTES, OR THE SPECIFICATIONS, THE STRICTEST REQUIREMENTS, AS INDICATED BY THE ENGINEER, SHALL GOVERN.
E. THE CONTRACTOR SHALL COORDINATE ALL ELEVATIONS AND DIMENSIONS, INCLUDING BUT NOT LIMITED TO THOSE FOR OPENINGS IN WALLS AND IN ROOF AND FLOOR SYSTEMS, WITH THE ARCHITECTURAL, PLUMBING, ELECTRICAL, AND MECHANICAL PLANS.
F. ALL DIMENSIONS, ELEVATIONS, AND ANY OTHER CONDITIONS OF ANY EXISTING STRUCTURES OR OTHER FEATURES SHALL BE VERIFIED BY THE GENERAL CONTRACTOR AND ANY DISCREPANCIES WITH THE CONTRACT DRAWINGS REPORTED TO THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK. DURING THE CONSTRUCTION PROCESS, IT SHALL BE SOLELY THE CONTRACTOR'S RESPONSIBILITY TO MAINTAIN THE INTEGRITY OF THE EXISTING STRUCTURE AND TO PROTECT FROM DAMAGE ANY PORTIONS THAT ARE TO REMAIN.
G. THE COMPLETED LATERAL-FORCE RESISTING SYSTEMS AND DIAPHRAGMS ARE REQUIRED FOR THE STRUCTURE TO RESIST LATERAL LOADS AND PROVIDE STABILITY UNDER GRAVITY LOADS. DURING THE CONSTRUCTION PROCESS, THE CONTRACTOR SHALL PROVIDE ALL REQUIRED BRACING DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS UNTIL THE LATERAL-LOAD RESISTING OR STABILITY-PROVIDING SYSTEM IS COMPLETELY INSTALLED AND THE STRUCTURE IS COMPLETELY TIED TOGETHER.
H. UNLESS OTHERWISE NOTED, DETAILS SHOWN ON ANY DRAWING ARE TO BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS.
I. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS AND METHODS AND FOR SAFETY PRECAUTIONS AND PROGRAMS.
J. BRITT, PETERS & ASSOCIATES, INC. SHALL NOT BE RESPONSIBLE FOR THE ACTS OR OMISSION OF THE CONTRACTOR OR FOR THEIR FAILURE TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
K. THE BUILDING OWNER SHALL PROVIDE PERIODIC MAINTENANCE TO INSURE STRUCTURAL INTEGRITY. SUCH MAINTENANCE SHALL INCLUDE BUT NOT LIMITED TO PAINTING OF STEEL, PROTECTIVE COATING FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS.

II. DESIGN CRITERIA
A. THE CONTRACT DOCUMENTS ARE BASED ON THE REQUIREMENTS OF THE 2018 NORTH CAROLINA STATE BUILDING CODE.
B. DEAD LOADS
1. TYPICAL ROOF SYSTEMS: 15 PSF TOTAL
a. MEP*: 20 PSF
b. INSULATION & ROOFING: 10 PSF

* MISCELLANEOUS CEILING AND HANGING MECHANICAL LOADS SUCH AS DUCT WORK AND SPRINKLER PIPES.

C. LIVE LOADS*
1. LIVE LOADS ARE BASED ON THE MORE RESTRICTIVE OF THE UNIFORM LOAD LISTED BELOW OR THE CONCENTRATED LOAD LISTED ACTING OVER A 6.25 SQUARE FOOT AREA. LIVE LOADS HAVE BEEN REDUCED AS PRESCRIBED IN THE AFOREMENTIONED BUILDING CODE.

CATEGORY	UNIFORM LOAD (PSF)	CONCENTRATED LOAD (LBS)
a. DINING ROOMS/RESTAURANTS	100	
b. ROOFS		
ALL ROOF SURFACES SUBJECT TO WORKERS		300
ORDINARY ROOF	20	

*OR EQUIPMENT WEIGHT IF GREATER

D. DESIGN SNOW LOAD:

GROUND SNOW LOAD,	PG	15 PSF
FLAT ROOF SNOW LOAD,	PF	20 PSF
EXPOSURE FACTOR,	CE	1.0
SNOW THERMAL FACTOR,	CT	1.0
SNOW IMPORTANCE FACTOR,	I	1.0

E. DESIGN WIND LOADS:

BASIC WIND SPEED,	V_{ult}/V_{asd}	115 MPH / 90 MPH (3 SECOND GUST)
RISK CATEGORY		II
EXPOSURE		B
INTERNAL PRESSURE COEFF	GCPI	±0.18

Design Wind Pressure (psf):

			Effective Wind Area (sqft)					
Walls:			10	20	50	100	200	500
Interior	Area 4	+	18.2	17.4	16.3	16.0	16.0	16.0
		-	-19.7	-18.9	-17.8	-17.0	-16.2	-16.0
Edge	Area 5	+	18.2	17.4	16.3	16.0	16.0	16.0
		-	-24.3	-22.6	-20.5	-18.9	-17.3	-16.0
Roof:			10	20	50	100	200	500
Interior	Area 1	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-19.9	-19.4	-18.7	-18.2	-18.2	-18.2
Edge	Area 2	+	18.2	17.4	16.3	16.0	16.0	16.0
		-	-33.4	-29.8	-25.1	-21.6	-21.6	-21.6
Corner	Area 3	+	18.2	17.4	16.3	16.0	16.0	16.0
		-	-33.4	-29.8	-25.1	-21.6	-21.6	-21.6
Overhang:			10	20	50	100	200	500
Interior	Area 1	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-28.6	-28.1	-27.5	-27.0	-23.4	-18.6
Edge	Area 2	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-28.6	-28.1	-27.5	-27.0	-23.4	-18.6
Corner	Area 3	+	16.0	16.0	16.0	16.0	16.0	16.0
		-	-47.2	-37.0	-23.6	-16.0	-16.0	-16.0

Parapet Design Pressure (psf):

			Effective Wind Area (sqft)					
Parapet:			10	20	50	100	200	500
Edge	Area 2	+	56.7	51.2	44.1	38.6	37.6	36.3
		-	-39.7	-37.6	-35.0	-33.0	-31.0	-28.3
Corner	Area 3	+	56.7	51.2	44.1	38.6	37.6	36.3
		-	-45.3	-42.3	-38.3	-35.3	-32.3	[illegible]

CORNER AND EDGE ZONES ARE 4.0 FEET WIDE.

F. SEISMIC LOADS:

SHORT PERIOD SPECTRAL RESPONSE ACCELERATION,	SS	0.238g
1-SEC PERIOD SPECTRAL RESPONSE ACCELERATION,	S1	0.096g
SHORT PERIOD DESIGN SPECTRAL RESPONSE ACCELERATION,	SDS	[illegible].254g
1-SEC PERIOD DESIGN SPECTRAL RESPONSE ACCELERATION,	SD1	[illegible]54g
RISK CATEGORY		[illegible]
SEISMIC DESIGN CATEGORY		[illegible]
SITE CLASS		[illegible]

BASIC SEISMIC-FORCE RESISTING SYSTEM:
LIGHT FRAME WOOD BEARING SHEAR WALLS SHEATHED WITH WOOD STRUCTURAL PANELS

RESPONSE MODIFICATION FACTOR,	R	6.5
DEFLECTION AMPLIFICATION FACTOR,	CD	4.0
SEISMIC IMPORTANCE FACTOR	IE	1.0
SEISMIC RESPONSE COEFFICIENT	CS	0.034
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE		
DESIGN BASE SHEAR	V	3 KIPS

G. THE CONTRACTOR SHALL VERIFY ALL MECHANICAL EQUIPMENT WEIGHTS, LOCATIONS AND ASSOCIATED OPENINGS WITH THE MECHANICAL CONTRACTOR AND SUBMIT SUCH INFORMATION PRIOR TO FABRICATION OF THE SUPPORTING STRUCTURE. PROMPTLY NOTIFY THE ENGINEER IF THE ACTUAL WEIGHT EXCEEDS THE WEIGHT SHOWN ON THE STRUCTURAL DRAWINGS.
H. PROVISIONS SHALL BE MADE IN THE DETAILING, FABRICATION, AND ERECTION OF ALL CLADDING, PARTITIONS, WALLS, ETC. TO ACCOUNT FOR FLOOR TO FLOOR DEFLECTIONS AND LATERAL FRAME DEFLECTION.

III. FOUNDATIONS
A. BUILDING DESIGNED FOR AN ALLOWABLE SOIL BEARING CAPACITY OF 2,000 PSF HAS BEEN ASSUMED AND SHALL BE CONFIRMED BY A QUALIFIED SOILS ENGINEER PRIOR TO PLACEMENT OF CONCRETE.
B. THE CONTRACTOR SHALL OBTAIN A COPY OF THE SOILS REPORT AND ADHERE TO ALL RECOMMENDATION WITHIN, INCLUDING PREPARATION OF SOILS AT BUILDING PAD.
C. ALL SOILS WORK, INCLUDING BACKFILL OF UTILITY TRENCHES AND THE VERIFICATION OF BEARING CAPACITY OF SAME SHALL BE UNDER THE DIRECTION OF A QUALIFIED SOILS ENGINEER. PROXIMITY OF UTILITY TRENCHES TO BUILDING FOUNDATION SYSTEM SHALL BE AS APPROVED BY THE SOILS ENGINEER TO INSURE INTEGRITY OF THE BEARING SOILS.
D. ALL FOOTINGS SHALL BEAR ON UNDISTURBED EARTH OR ENGINEERED FILL AT ELEVATIONS SHOWN ON PLANS AND DETAILS. FLOOR SLABS SHALL BEAR ON 4 INCHES OF COMPACTED STONE. THE MOISTURE RETARDER SHALL BE PLACED BETWEEN THE STONE AND THE SLAB.
E. NO FOUNDATION CONCRETE SHALL BE INSTALLED UNTIL ALL FOUNDATION WORK HAS BEEN COORDINATED WITH UNDERGROUND UTILITIES. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD OF ALL CONFLICTS THAT EXIST BETWEEN FOOTINGS AND UTILITIES.
F. ALL FOUNDATIONS OR PORTIONS THEREOF, BELOW GRADE MAY BE EARTH FORMED BY NEAT EXCAVATIONS.
G. UNLESS OTHERWISE SHOWN, ALL FOOTINGS SHALL BE CENTERED ON WALLS AND/OR COLUMNS.
H. THE CONTRACTOR SHALL DETERMINE THE EXTENT OF CONSTRUCTION DEWATERING REQUIRED FOR THE EXCAVATION. THE CONTRACTOR SHALL SUBMIT TO THE GEOTECHNICAL ENGINEER FOR REVIEW THE PROPOSED PLAN FOR CONSTRUCTION DEWATERING, PRIOR TO EXCAVATION.
I. FOOTINGS SHALL NOT BE PLACED ON FROZEN SUBGRADE OR IN STANDING WATER.
J. HEAVY EQUIPMENT SHOULD NOT BE ALLOWED WITHIN 8 FEET OF ANY EARTH RETAINING WALL. USE ONLY HAND OPERATED VIBRATORY COMPACTORS FOR COMPACTING BEHIND RETAINING WALLS.
K. FOUNDATION TYPE
1. SPREAD FOOTING TOTAL LOAD: 2500 PSF NET PRESSURE.

IV. CONCRETE
A. CONCRETE SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
CONCRETE 28 DAY COMPRESSIVE STRENGTH AND DENSITY REQUIREMENTS:

USAGE	STRENGTH (PSI)	CONC. TYPE
a. FOOTINGS	3000	NWT
b. SLAB-ON-GRADE INTERIOR	3500	NWT

1. NWT = NORMAL WEIGHT CONCRETE
2. LWT = SAND-LIGHTWEIGHT CONCRETE (120 PCF MAX.)
3. ALL CONCRETE SHALL HAVE ALLOWABLE UNIT SHRINKAGE OF 0.045% AT 28 DAYS. (SEE ASTM C157)
4. ALL SLABS TO RECEIVE MOISTURE SENSITIVE FLOOR COVERINGS SHALL HAVE MAXIMUM WATER/ CEMENT RATIO OF 0.45.

B. CONCRETE SHALL CONFORM TO THE FOLLOWING DURABILITY REQUIREMENTS PER ACI-318 SECTION 4.2 & 4.3:

EXPOSURE/LOCATION	
a. FOOTINGS	F0, S0, P0, C1
b. INTERIOR SLAB	F0, S0, P0, C0

C. ALL CONCRETE CONSTRUCTION SHALL CONFORM TO THE CURRENT "ACI MANUAL OF CONCRETE PRACTICE".
D. PORTLAND CEMENT SHALL CONFORM TO ASTM C-150, TYPE I OR II.
E. ALL AGGREGATE FOR NORMAL WEIGHT CONCRETE SHALL MEET ASTM C 33.
F. ALL REINFORCEMENT SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS:
1. ALL REINFORCING, UNO: ASTM A615 GRADE 60
2. WELDED WIRE REINFORCEMENT (WWR):
a. SMOOTH WIRE: ASTM A 185 (65 KSI)
b. DEFORMED WIRE: ASTM A 497 (70 KSI)

G. REINFORCEMENT DETAILING:
1. REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315.
2. DEVELOPMENT AND SPLICE LENGTHS ARE IN TENSION UNLESS OTHERWISE INDICATED AND SHALL BE AS TABULATED IN THE SPLICE LENGTH TABLE, UNLESS OTHERWISE INDICATED.
3. LAP WWR ONE CROSSWIRE SPACING PLUS 2".
4. PROVIDE CORNER BARS AT ALL FOOTINGS AND WALL INTERSECTIONS TO MATCH HORIZONTAL REINFORCING SIZE AND SPACING. AT INTERSECTIONS OF CONTINUOUS SPREAD FOOTINGS EXTEND ALL BARS TO FAR SIDE OF INTERSECTING FOOTING
5. REINFORCEMENT SHALL BE SECURELY PLACED TO PREVENT DISPLACEMENT DURING CONCRETE PLACEMENT. PROVIDE THE FOLLOWING CONCRETE COVER FOR REINFORCING [ACI 318 SECTION 7.7 AND IBC TABLE 720.1], UNLESS SPECIFICALLY DETAILED OTHERWISE:
a. CAST AGAINST EARTH 3"
b. EXPOSED TO EARTH/WEATHER
#6 THRU #18 2"
#5 & SMALLER 1 1/2"
6. PROVIDE DOWELS TO MATCH REINFORCEMENT SIZE AND SPACING INDICATED FOR ALL STRUCTURAL ELEMENTS, UNLESS OTHERWISE INDICATED.

H. FOOTINGS SHALL BE CAST IN ALTERNATE PANELS NOT TO EXCEED 60'-0" IN LENGTH. SHEAR KEYS SHALL BE PROVIDED AT EACH CONSTRUCTION JOINT AND SHALL BE LOCATED AT 1/3 POINTS OF SPANS.
I. HORIZONTAL CONSTRUCTION JOINTS IN CONCRETE POURS SHALL NOT BE USED UNLESS SHOWN ON THE DRAWINGS. THE ARCHITECT/ENGINEER SHALL APPROVE ALL DEVIATIONS OR ADDITIONAL JOINTS IN WRITING.
J. CHAMFER ALL PERMANENTLY EXPOSED CONCRETE EDGES 3/4 INCH, UNLESS NOTED OTHERWISE.
K. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS OF OPENINGS AND SLEEVES IN CONCRETE WALLS AND SUPPORTED FLOORS. SPREAD REINFORCEMENT AT OPENINGS AND SLEEVES UNLESS OTHERWISE SHOWN. DO NOT CUT REINFORCEMENT. SEE TYPICAL REINFORCEMENT DETAILS FOR OPENINGS IN SLABS AND WALLS FOR ADDITIONAL REQUIREMENTS.
L. NO HOLES OR OPENINGS THROUGH FOUNDATION WALLS AND/OR FOOTINGS WITHOUT ENGINEER'S APPROVAL.
M. ALUMINUM SHALL NOT BE EMBEDDED IN ANY CONCRETE.

V. POST-INSTALLED ANCHORS:
A. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS.
B. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
C. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH EXISTING REBAR. HOLES SHALL BE DRILLED AND CLEANED PER THE MANUFACTURER'S INSTRUCTIONS. ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTALLATION INSTRUCTIONS AT NOT LESS THAN MINIMUM EDGE DISTANCES AND/OR SPACINGS INDICATED IN THE MANUFACTURER'S LITERATURE. CONTACT MANUFACTURER PRIOR TO ANCHOR INSTALLATION IF TRAINING IS REQUIRED.
D. UNLESS SPECIFIED OTHERWISE, ANCHORS SHALL BE EMBEDDED IN THE APPROPRIATE SUBSTRATE WITH A MINIMUM EMBEDMENT OF 8 TIMES THE NOMINAL ANCHOR DIAMETER OR THE EMBEDMENT REQUIRED TO SUPPORT THE INTENDED LOAD.
E. ADHESIVE ANCHOR DESIGN BOND STRENGTH HAS BEEN BASED ON CRACKED CONCRETE, ACI 355.4 TEMPERATURE CATEGORY [illegible] AND INSTALLATIONS INTO DRY HOLES DRILLED USING A HAMMER DRILL INTO CONCRETE THAT HAS CURED FOR AT LEAST 21 DAYS. ADHESIVE ANCHORS SHALL BE INSTALLED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER PER ACI 318-11, D.9.2.2 WHERE INDICATED ON THE CONTRACT DOCUMENTS. INSTALLATION REQUIRING CERTIFIED INSTALLERS SHALL BE INSPECTED PER ACI 318-11, D.9.[illegible]
F. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE LISTED BELOW, SHALL BE SUBMITTED TO THE ENGINEER WITH CALCULATIONS THAT ARE PREPARED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER SHOWING THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE REQUIRED BY THE BUILDING CODE.
G. ACCEPTABLE PRODUCTS ARE:
1. CONCRETE ADHESIVE ANCHORS:
a. HILTI HY 200

VI. PREFABRICATED WOOD TRUSSES
A. ALL PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED TO MEET THE LOADINGS SPECIFIED. FABRICATION AND ERECTION SHALL BE PER TRUSS PLATE INSTITUTE RECOMMENDATIONS AS CONTAINED IN THE APPROPRIATE PUBLICATIONS.
B. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT.
C. CONNECTIONS OF HIP TRUSSES SHALL BE WITH APPROPRIATE TRUSS HANGERS AS MANUFACTURER BY SIMPSON STRONG TIE CO., INC. OR AN APPROVED ALTERNATIVE FOR THE LOADS SPECIFIED.
D. COORDINATE TRUSS WEB CONFIGURATION WITH MECHANICAL DUCTWORK AS INDICATED ON MECHANICAL SHEETS. PROVIDE CLEAR SPACE BETWEEN WEBS AS REQUIRED TO INSTALL DUCTWORK.
E. PROVIDE ALL TEMPORARY AND PERMANENT TRUSS BRACING AND BRIDGING AS REQUIRED AND SHOWN ON THE TRUSS MANUFACTURERS SHOP DRAWINGS.
F. PROVIDE 2x4 DIAGONAL BRACING AT ROOF TRUSS VERTICALS WHERE INDICATED ON SECTIONS, DETAILS, OR TRUSS ELEVATION SCHEMATICS.
G. INSTALL STRONG BACKS, BRACING AND/OR BRIDGING PRIOR TO DECK INSTALLATION AND AS TRUSSES ARE ERECTED.
H. INSTALL 2x4 CONTINUOUS BOTTOM CHORD BRACING AT 6 FEET ON CENTER MAXIMUM AT ALL AREAS WHERE A RIGID CEILING IS NOT ATTACHED DIRECTLY TO THE TRUSS BOTTOM CHORD.
I. ALL HURRICANE TIES SHALL BE INSTALLED PRIOR TO SHEATHING.
J. REFER TO ARCHITECTURAL DRAWINGS FOR TRUSS PROFILES.
K. ALL TRUSS TO TRUSS CONNECTIONS SHALL BE DESIGNED BY THE DELEGATED TRUSS ENGINEER.

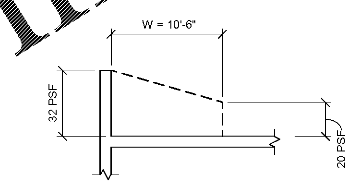
VII. WOOD FRAMING
A. SAWN CUT LUMBER:
1. UNLESS NOTED OTHERWISE, ALL LUMBER TO BE #2 KD SOUTHERN YELLOW PINE WITH A MAXIMUM MOISTURE CONTENT OF 19%.
2. ALL EXTERIOR WALLS TO BE FRAMED WITH #2 SOUTHERN YELLOW PINE 2X6 STUDS SPACED AT 16"ON CENTER.
3. PRESSURE (PRESERVATIVE) TREATED LUMBER
a. ALL LUMBER EXPOSED TO THE EXTERIOR ENVIRONMENT SHALL BE PRESSURE TREATED AND SHALL BEAR THE THIRD PARTY QUALITY MARK "ABOVE GROUND USE" AND MEET THE STANDARDS OF AWPA U1 USE CATEGORY UC3B (ABOVE GROUND, EXPOSED).
b. ALL LUMBER IN CONTACT WITH CONCRETE, MASONRY, OR SOIL SHALL BE PRESSURE TREATED AND SHALL BEAR THE THIRD PARTY QUALITY MARK "GROUND CONTACT" AND MEET THE STANDARDS OF AWPA U1 USE CATEGORY UC4A (GROUND CONTACT, GENERAL USE).
c. ACZA (AMMONIACAL COPPER ZINC ARSENATE) SHALL NOT BE USED AS A CHEMICAL FOR PRESSURE TREATED LUMBER.
4. AS A MINIMUM, FASTEN ALL WOOD FRAMING WITH COMMON NAILS TO COMPLY WITH THE "FASTENING SCHEDULE" OF THE AFOREMENTIONED BUILDING CODE.
5. ALL MULTIPLE PIECE WOOD BEAMS TO BE CONNECTED TOGETHER WITH (3) ROWS OF 16D NAILS @ 12"OC (UNLESS NOTED OTHERWISE).
6. PROVIDE SOLID BLOCKING BETWEEN JOISTS UNDER ALL LOAD BEARING PARTITIONS RUNNING PERPENDICULAR TO JOISTS.
7. PROVIDE SOLID BLOCKING BETWEEN JOIST AT ALL BEARING LOCATIONS.
8. THE DOUBLE TOP PLATES OF THE WALL SHALL RESIST THE CHORD FORCES IN THE ROOF DIAPHRAGM AND ACT AS DRAG STRUTS BETWEEN SHEAR WALL SEGMENTS. JOINTS SHALL BE LAPPED SPLICED WITHIN THE CENTER THIRD OF A WALL LENGTH AND THE MINIMUM LAP SHALL BE 4 FEET.
9. TIMBER CONNECTORS
a. TIMBER CONNECTORS CALLED FOR ON THE DRAWINGS ARE AS MANUFACTURED BY THE SIMPSON COMPANY. CONNECTORS BY OTHER MANUFACTURERS MAY BE USED IF THE LOAD CAPACITY IS EQUAL TO OR GREATER THAN THE CONNECTOR SPECIFIED. USE MANUFACTURER'S FURNISHED NAILS AND BOLTS.
b. CONNECTORS SHALL HAVE A MINIMUM CORROSION PROTECTION OF G90 GALVANIZATION.
c. CONNECTORS IN CONTACT WITH PRESSURE TREATED OR FIRE TREATED LUMBER SHALL BE MANUFACTURED FROM SIMPSON ZMAX (G185 GALVANIZED) STEEL.
d. CONNECTORS IN PROXIMITY TO SALT WATER SPRAY SHALL BE MANUFACTURED FROM TYPE 316L STAINLESS STEEL.
12. TIMBER FASTENERS
a. FASTENERS USED IN PRESSURE TREATED OR FIRE TREATED LUMBER SHALL BE GALVANIZED TO ASTM STANDARD B695 - CLASS 55 OR A153 - CLASS D.
b. FASTENERS USED IN PROXIMITY TO SALT WATER SPRAY SHALL BE MANUCATURED FROM TYPE 316 STAINLESS STEEL OR BE HOT DIP GALVANIZED TO ASTM STANDARD A153 - CLASS C.
13. ERECTION TOLERANCES
a. FRAMING MEMBERS WHICH WILL BE COVERED BY FINISHES SUCH AS WALLBOARD, PLASTER, OR CERAMIC TILE SET IN A MORTAR SETTING BED, SHALL BE WITHIN THE FOLLOWING LIMITS:
a.a. LAYOUT OF WALLS AND PARTITIONS: 1/4 " FROM INTENDED POSITION.
a.b. PLATES AND RUNNERS: 1/4" IN 8'-0" FROM A STRAIGHT LINE.
a.c. STUDS: $\frac{1}{4}$" IN 8'-0" OUT OF PLUMB, NOT CUMULATIVE.
a.d. FACE OF FRAMING MEMBERS: 1/4" IN 8'-0' FROM A TRUE PLANE.
b. FRAMING MEMBERS WHICH WILL BE COVERED BY CERAMIC TILE SET IN DRY-SET MORTAR, LATEX-PORTLAND CEMENT MORTAR, OR ORGANIC ADHESIVE SHALL BE WITHIN THE FOLLOWING LIMITS:
b.a. LAYOUT OF WALLS AND PARTITIONS: 1/4 " FROM INTENDED POSITION.
b.b. PLATES AND RUNNERS: 1/8" IN 8'-0" FROM A STRAIGHT LINE.
b.c. STUDS: 1/8" IN 8'-0" OUT OF PLUMB, NOT CUMULATIVE.
b.d. FACE OF FRAMING MEMBERS: 1/8" IN 8'-0' FROM A TRUE PLANE.

B. WALL AND ROOF SHEATHING
1. WALL SHEATHING SHALL BE MANUFACTURED BY A MEMBER OF AMERICAN PLYWOOD ASSOCIATION, SHALL BE LABELED WITH THE APA GRADE STAMP AND CONFORM TO THE FOLLOWING REQUIREMENTS:
a. PANEL GRADE — RATED SHEATHING
b. SPAN RATING — 16/32
c. EXPOSURE DURABILITY CLASSIFICATION — EXPOSURE 1
d. PRODUCT STANDARD — PS1
e. THICKNESS — 5/8
2. ROOF SHEATHING SHALL BE MANUFACTURED BY A MEMBER OF AMERICAN PLYWOOD ASSOCIATION, SHALL BE LABELED WITH THE APA GRADE STAMP AND CONFORM TO THE FOLLOWING REQUIREMENTS:
a. PANEL GRADE — RATED SHEATHING
b. SPAN RATING — 40/20
c. EXPOSURE DURABILITY CLASSIFICATION — EXPOSURE 1
d. PRODUCT STANDARD — PS1
e. THICKNESS — 5/8
3. ALL SHEATHING SHALL BE INSTALLED WITH THE STRENGTH (TYPICALLY FACE GRAIN) DIRECTION PERPENDICULAR TO THE SUPPORTING FRAMING WITH STAGGERED JOINTS.
4. ROOF SHEATHING SHALL BE INSTALLED WITH 5/8 PSCL SHEATHING CLIPS BY SIMPSON STRONG TIE, INSTALLED BETWEEN THE EDGED OF ALL ADJACENT PANELS MIDWAY BETWEEN SUPPORTING FRAMING MEMBERS THAT ARE SPACED MORE THAN 20-INCHES APART.
5. WALL SHEATHING SHALL BE FASTENED TO SUPPORTING FRAMING WITH 10D COMMON RING SHANK NAILS AT THE SPACING INDICATED BELOW UNLESS NOTED OTHERWISE IN THE SHEAR WALL SCHEDULE:
a. WALL EDGE — 6" OC
b. SUPPORTED PANEL EDGES AWAY FROM EDGE OF WALL — 6" OC
c. CENTER OF PANELS — 12" OC
6. ROOF SHEATHING SHALL BE FASTENED TO SUPPORTING FRAMING WITH 8D COMMON RING SHANK NAILS AT THE SPACING INDICATED BELOW:
a. ROOF EDGE — 4" OC, UNLESS NOTED OTHERWISE
b. SUPPORTED PANEL EDGES AWAY FROM EDGE OF ROOF — 6" OC
c. SUPPORTED PANEL EDGES BLOCKED DIAPHRAGM — 6" OC, UNLESS NOTED OTHERWISE
d. CENTER OF PANELS — 12" OC
7. WHERE EITHER 2-INCH OR 2 1/2-INCH FASTENER SPACINGS ARE USED WITH 2-INCH WIDE FRAMING MEMBERS FOR WOOD STRUCTURAL PANELS USED AT ROOF OR FLOOR, THE FRAMING MEMBER ADJOINING PANEL EDGES SHALL BE 3-INCH NOMINAL WIDTH AND NAILS AT PANEL EDGES SHALL BE STAGGERED IN TWO LINES.

C. LAMINATED VENEER LUMBER (LVL):
1. ALL LAMINATED VENEER LUMBER SHALL BE DESIGNED AND MANUFACTURED TO THE STANDARDS SET FORTH IN THE NER-126 REPORT.
2. ALLOWABLE UNIT STRESSES REQUIRED FOR DRY CONDITIONS OF USE FOR VENEER LAMINATED LUMBER SHALL BE AS FOLLOWS:
a. BENDING — 2600 PSI
b. COMPRESSION PARALLEL TO GRAIN — 2460 PSI
c. HORIZONTAL SHEAR — 285 PSI
d. COMPRESSION PERPENDICULAR TO GRAIN — 750 PSI
3. LAMINATED VENEER LUMBER MEMBER SIZES SHOWN ARE NET; OTHER MEMBER SIZES ARE NOMINAL.

VIII. SUBMITTALS
A. THE GENERAL CONTRACTORS SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING FOR REVIEW. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND/OR ENGINEER AND HAVE THE ENGINEER'S SHOP DRAWING STAMP AFFIXED PRIOR TO FABRICATION. FABRICATION AND ERECTION SHALL BE FROM REVIEWED SHOP DRAWINGS. PLEASE ALLOW 10 BUSINESS DAYS FOR REVIEW.
B. A RECORD SET OF APPROVED SHOP DRAWINGS SHALL BE KEPT IN THE FIELD BY THE GENERAL CONTRACTOR.
C. ANY DEVIATION FROM, ADDITION TO, SUBSTITUTION FOR, OR MODIFICATION TO THE STRUCTURE OR ANY PART OF THE STRUCTURE DETAILED ON THE CONTRACT DOCUMENTS SHALL BE SUBMITTED IN WRITING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SUBMITTED FOR REVIEW DO NOT CONSTITUTE "IN-WRITING" UNLESS IT IS CLEARLY NOTED THAT SPECIFIC CHANGES ARE BEING SUGGESTED.
D. THE CONTRACTOR SHALL PREPARE A LIST AND SCHEDULE OF ALL STRUCTURAL SUBMITTALS PRIOR TO CONSTRUCTION.
E. THE FOLLOWING SHOP DRAWINGS SHALL BE SUBMITTED BY THE CONTRACTOR FOR THE ENGINEER'S REVIEW:
1. MISCELLANEOUS STEEL
2. METAL AND FABRIC CANOPIES - CONNECTION TO BUILDING SHALL BE BY SUPPLIER (1, 3)
3. STRUCTURAL STEEL, SHOP AND ERECTION DRAWINGS (1, 3)
4. CONCRETE MIX DESIGNS
5. REINFORCING STEEL
6. PRE-ENGINEERED WOOD TRUSSES
F. ITEMS MARKED (1) SHALL HAVE SHOP DRAWINGS SEALED BY A REGISTERED ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. ITEMS MARKED (2) SHALL BE SUBMITTED TO ENGINEER FOR OWNER'S RECORD ONLY AND WILL NOT HAVE THE ENGINEER'S SHOP DRAWING STAMP AFFIXED. ITEMS MARKED (3) SHALL HAVE DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED.
1. THE OMISSION FROM THE SHOP DRAWINGS OF ANY MATERIALS REQUIRED BY THE CONTRACT DOCUMENTS TO BE FURNISHED SHALL NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY OF FURNISHING AND INSTALLING SUCH MATERIALS, REGARDLESS OF WHETHER THE SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED.
G. THE USE OF ELECTRONIC FILES OR REPRODUCTIONS OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT, AND OBLIGATES THEMSELVES TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT MAY OCCUR HEREON.

IX. SPECIAL INSPECTION AND TESTING (CHAPTER 17)
A. ALL TESTS AND INSPECTIONS SHALL BE PERFORMED BY AN INDEPENDENT TESTING AND INSPECTION AGENCY. THE SPECIAL INSPECTOR FROM THIS TESTING AGENCY SHALL OBSERVE THE WORK FOR CONFORMANCE TO THE DESIGN DRAWINGS AND SPECIFICATIONS.
B. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE ENGINEER OR ARCHITECT OF RECORD, AND ALL OTHER DESIGNATED INDIVIDUALS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF NOT CORRECTED, TO THE PROPER DESIGN AUTHORITY AND TO THE BUILDING OFFICIAL.
C. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS, SPECIFICATIONS, SOILS REPORT AND APPLICABLE WORKMANSHIP PROVISIONS OF THE INTERNATIONAL BUILDING CODE.



SNOW DRIFT DIAGRAM

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NORTH CAROLINA PROFESSIONAL SEAL 037779 ENGINEER BRET UNDERWOOD 03/22/19

PROJECT: *STARBUCKS SHELL BUILDING*
350 BLOWING ROCK BOULEVARD
LENOIR, NC 28645

DRAWING: GENERAL NOTES

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