

STRUCTURAL NOTES

A. GENERAL

- THE DRAWINGS ARE INTENDED TO SHOW THE GENERAL ARRANGEMENT, DESIGN AND EXTENT OF THE WORK AND ARE PARTIALLY DIAGRAMMATIC. THEY ARE NOT INTENDED TO BE SCALED FOR DIMENSIONS, MEASUREMENTS, OR TO SERVE AS SHOP DRAWINGS OR PORTIONS THEREOF.
- ALL DETAILS AND SECTIONS SHOWN OR INDICATED ARE INTENDED TO BE TYPICAL AND SHALL BE CONSIDERED TO APPLY TO ANY SIMILAR SITUATION UNLESS NOTED OTHERWISE ON THE DRAWINGS. A DIFFERENT DETAIL OR SECTION IS SHOWN.
- PROVIDE TO START OF CONSTRUCTION. THE CONTRACTOR AND ALL THE SUB-CONTRACTORS SHALL VERIFY ALL GRADES, LEVELS, DIMENSIONS AND COORDINATE EXISTING CONDITIONS AT THE JOB SITE WITH THE PLANS AND SPECIFICATIONS. THEY SHALL REPORT ANY INCONSISTENCIES OR ERRORS IN THE ABOVE TO THE ARCHITECT/ENGINEER BEFORE COMMENCING WORK. THE CONTRACTOR AND HIS SUB-CONTRACTORS SHALL LAY OUT THEIR WORK FROM ESTABLISHED REFERENCE POINTS AND BE RESPONSIBLE FOR ALL LINES, ELEVATIONS AND MEASUREMENTS IN CONNECTION WITH THEIR WORK.
- IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION SEQUENCE AND SEQUENCE OF WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING UTILITIES AND STRUCTURES WHICH MAY BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER THE COMPLETION OF THE PROJECT.
- IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.
- THE CONTRACTOR SHALL USE THE STRUCTURAL DRAWINGS AND SPECIFICATIONS TOGETHER WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND OTHER TRADE DRAWINGS AND SHOP DRAWINGS, TO CORRELATE ALL DETAILS, DIMENSIONS, ELEVATIONS, ETC. NOTIFY ARCHITECT/ENGINEER IN WRITING OF ANY POTENTIAL CONFLICTS BEFORE PROCEEDING WITH THE WORK.
- SHOULD ANY OF THE DETAILED INSTRUCTIONS SHOWN ON THE PLANS CONFLICT WITH THESE STRUCTURAL NOTES, THE SPECIFICATIONS, OR WITH EACH OTHER, THE STRICTEST PROVISION SHALL GOVERN.

B. GOVERNING CODE: 2012 - INTERNATIONAL BUILDING CODE WITH GEORGIA AMENDMENTS

- ROOF SNOW LOADS
 - GROUND SNOW LOAD: $P_g = 0$ PSF (FIGURE 1608.2)
 - FLAT-ROOF SNOW LOAD: $P_f = 0$ PSF (SECTION 1607.2.3)
 - SNOW EXPOSURE FACTOR: $C_e = 1.0$ (TABLE 1608.3.1)
 - SNOW LOAD IMPORTANCE FACTOR: $I_s = 1.0$ (TABLE 1608.4)
 - SNOW THERMAL FACTOR: $C_t = 1.0$ (TABLE 1608.3.2)
 - ALL APPLICABLE EFFECTS DUE TO SNOW DRIFTING (SECTION 1608)
- ROOF LIVE LOADS
 - MINIMUM ROOF LIVE LOAD = 20 PSF (SECTION 1607.1.2)
 - USE "PREPARATED WOOD TRUSSES" TO DESIGN TRUSS FOR ADDITIONAL LOADING INFORMATION.
- WIND LOADS
 - BASIC WIND SPEED = 120 MPH (FIGURE 1609)
 - WIND LOAD IMPORTANCE FACTOR: $I_w = 1.0$ (TABLE 1604.4)
 - WIND EXPOSURE CATEGORY "B" (SECTION 1609.4)
- SEISMIC DESIGN DATA
 - SEISMIC SITE CLASS - BASED ON SECTION 1615.1
 - SEISMIC IMPORTANCE FACTOR: $I_e = 1.0$ (TABLE 1604.5)
 - SITE (SOIL) CLASS - SECTION 1615.1.2
 - STRUCTURAL FRAMING AND SEISMIC RESISTING SYSTEM: LIGHT-FRAME WALLS WITH SHEAR WALLS (TABLE 1617.8.2, ITEM 1 K)

C. FOUNDATION

- FOUNDATIONS ARE DESIGNED TO BEAR ON NATURAL GRADE OR FILL, WELL COMPACTED OF AN ALLOWABLE BEARING CAPACITY, INDICATED ON THE FOUNDATION PLAN.
- A CERTIFIED TESTING LABORATORY SHALL BE ENGAGED BY THE OWNER TO PERFORM SOIL BORINGS, PROVIDE A FOUNDATION REPORT AND VERIFY THAT THE REQUIRED BEARING CAPACITY WILL BE MAINTAINED.
- SOIL CAPACITY SHALL BE DETERMINED AND TESTED BY A REGISTERED FOUNDATION ENGINEER, PRIOR TO CASTING OF CONCRETE IN THE FOOTINGS.
- BOTTOM OF FOOTING ELEVATION TO BE DETERMINED BY THE SOIL CONDITIONS AND FROST-LINE DEPTH.
- ALL LONGITUDINAL REBARS IN THE WALL FOOTINGS SHALL BE CONTINUOUS AND SPICED AS SHOWN. CONTINUE ALL HORIZONTAL REBARS AT BEAMS AND CORNERS BY BENDING THE REBARS 45 DEGREES AROUND THE CORNERS OR ADDING MATCHING CORNER BARS, EXTENDING 48 BAR-DIAMETERS INTO FOOTING EACH SIDE OF CORNER OR BENT.

D. REINFORCED CONCRETE

- MATERIALS
 - SPECIFICATIONS IN GENERAL, COMPLY WITH AC 308 (LATEST EDITION) "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS".
 - STRUCTURAL CONCRETE:

CLASS	LOCATION	f _c
I	FOOTINGS, CHAIRS & GRADE BEAMS	3,500
II	INTERIOR SLABS ON GRADE, AND ALL CONCRETE NOT OTHERWISE IDENTIFIED	4,000
III	PIERS PLACED INTEGRALLY WITH WALLS, EXTERIOR SLABS ON GRADE, AND ALL EXTERIOR CONCRETE (WITH AIR) NOT OTHERWISE IDENTIFIED	4,000
IV	BACKFILL BELOW FOOTINGS AND GRADE BEAMS	1,500
 - ALL DEFORMED REINFORCING BARS: F_y = 60,000.
 - GALVANIZED WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 (LATEST EDITION). USE SHEET FORM, NOT WALK.
- FIELD MANUAL: PROVIDE AT LEAST ONE COPY OF THE ACI FIELD REFERENCE MANUAL, SP-13, IN THE FIELD OFFICE AT ALL TIMES.
- CONTINGENCIES: PROVIDE SUPPORTS AS REQUIRED TO MAINTAIN ALIGNMENT AND COVER OVER THE REINFORCING.
- FOOTINGS:
 - VERTICAL DOWNLAYS IN FOOTINGS TO MATCH VERTICAL WALL REINFORCING.
 - PROVIDE LEAN CONCRETE (CLASS 10) UNDER FOOTINGS FOR ACCIDENTAL OVER-EXCAVATION, SOFT SPOTS AND TRENCHES.
- SPLICES: UNLESS NOTED OTHERWISE, MINIMUM LAP SPLICE LENGTHS TO BE AS FOLLOWS:
 - VERTICAL BARS IN WALLS, PIERS, OR COLUMNS (INCLUDING DOWNLAYS): 35 DIAMETER
 - HORIZONTAL BARS IN SLABS & FOOTING: 35 DIAMETER
 - HORIZONTAL BARS IN WALL: 45 DIAMETER
- SAW-CUT & CONSTRUCTION JOINTS: PROVIDE JOINTS IN ALL SLABS-ON-GRADE, AS INDICATED ON THE FOUNDATION PLAN.
- CONCRETE COVER: UNLESS NOTED OTHERWISE, DETAIL REINFORCING TO PROVIDE CONCRETE COVER AS FOLLOWS:
 - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 INCHES
 - CONCRETE EXPOSED TO EARTH OR WEATHER:

#5 BARS AND SMALLER	1-1/2 INCHES
OTHER	2 INCHES
 - CONCRETE NOT EXPOSED TO EARTH OR WEATHER:

BEAM AND COLUMN BARS INCLUDING REINFORCING	1-1/2 INCHES
SLABS, WALLS, JOISTS	1 INCH
OTHER	1-1/2 INCHES

E. STRUCTURAL STEEL

- MATERIALS
 - STRUCTURAL STEEL: ASTM A572 - 50 OR ASTM A588 - 80
 - ANCHOR BOLTS: ASTM A307 OR A330 - ELECTRODES: SERIES 70
 - EXPANSION BOLTS: HELIX TOWER BOLTS OR APPROVED EQUAL.
- SPECIFICATIONS: WELDING PERSONNEL AND PROCEDURES ARE TO BE QUALIFIED PER AWS D1.1. UNLESS SPECIFICALLY NOTED OTHERWISE, DESIGN, FABRICATION AND ERECTION TO BE GOVERNED BY: (ALL) THE LATEST EDITION OF:
 - AWSD D1.1 - WELDING SPECIFICATIONS FOR STEEL BOULDS.
 - AWSD D1.5 - WELDING SPECIFICATIONS FOR STEEL BOULDS.
 - AWSD D1.6 - WELDING SPECIFICATIONS FOR STEEL BOULDS.
 - AWSD D1.7 - WELDING SPECIFICATIONS FOR STEEL BOULDS.
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 - AWSD D1.100 - WELDING SPECIFICATIONS FOR STEEL BOULDS.
- CONNECTIONS: CONNECTIONS TO BE DESIGNED BY THE FABRICATOR TO DEVELOP FULL STRENGTH OF MEMBER OR FORCES SHOWN ON THE PLANS. WELDING GOVERNS. FOLLOW INSTRUCTIONS ON DRAWINGS FOR GENERAL ARRANGEMENT OR PARTICULAR DETAILS.
- GALVANIZING: ALL SHEET ANGLE UNITS IN EXTERIOR WALLS AND ALL EXTERIOR STEEL EXPOSED TO THE ELEMENTS SHALL BE HOT DIP GALVANIZED.
- MISCELLANEOUS:
 - PROVIDE HOLES FOR OTHERS. IF OPENING IS NOT SHOWN ON THE STRUCTURAL DRAWINGS, OBTAIN PRIOR APPROVAL.
 - STEEL SUPPORTING OR CONNECTED TO HVAC AND OTHER EQUIPMENT AND ROOF DRUMS AS SHOWN ON THE DRAWINGS IS SHOWN FOR BEARING PURPOSES ONLY. CONTRACTOR SHALL RECONCILE EXACT SIZE AND LOCATION BEFORE PROCEEDING WITH THE WORK.
 - CROCK UNDER BEARING PLATES, BASE PLATES, AND SETTING PLATES TO BE NON-SHRINKING TYPE.
 - STEEL BELOW GRADE TO BE PROTECTED BY A MIN. OF 3 INCHES OF CONCRETE.
 - PROVIDE HEAVY WASHERS AT ALL ANCHOR BOLTS.
 - EMBEDMENT LENGTH OF EXPANSION BOLTS INTO SOLID MASONRY OR CONCRETE SHALL BE AS FOLLOWS:

1/2 INCH DIAMETER BOLTS	3 1/2 INCHES EMBEDMENT
3/4 INCH DIAMETER BOLTS	5 INCHES EMBEDMENT

F. MASONRY

- MATERIALS
 - CONCRETE BLOCK: ASTM D90 (HOLLOW) ASTM D145 (SOLID)
 - MORTAR: ASTM C270 TYPE "S", AVERAGE COMPRESSIVE STRENGTH: 1800 PSI MINIMUM (AT 28 DAYS)
 - JOINT BEAM AND CORE FILL: ASTM C474, COARSE TYPE
 - JOINT REINFORCING: MILD GALVANIZED FIBER, 9 GAUGE MINIMUM SIDE WIRES AND CROSS WIRES (LAGER OR RUBS TYPE)
 - BAR REINFORCING: ASTM A615, GRADE 60.
- REINFORCED MASONRY, WHERE VERTICAL BARS ARE TO BE GROUDED INTO CORES, THE FOLLOWING REQUIREMENTS APPLY:
 - PROVIDE CORES FROM FOOTING, SAME SIZE AND SPACING AS WALL BARS. LAP 12 INCHES MINIMUM WITH WALL BAR. EMBED INTO FOOTING 9 INCHES.
 - PROVIDE A CONTINUOUS VERTICAL CAVITY, AT LEAST 2 1/2" IN SIZE, FREE OF MORTAR DROPPINGS.
 - SPACES IN VERTICAL BARS, PROVIDE MECHANICAL COUPLERS OR 48 DIAMETER LAP.
 - ALL REINFORCEMENT MUST BE INSTALLED AND SECURELY ANCHORED IN PLACE PRIOR TO PLACEMENT OF GROUT.
- MISCELLANEOUS:
 - FILL CORE SOLID AROUND ANCHOR BOLTS.
 - PROVIDE SOLID BLOCKS OR SOLID FILL HOLLOW BLOCKS AT ALL EXPANSION BOLT LOCATIONS.
 - HOLLOW MASONRY UNITS TO BE LAD WITH FULL NORTHER COARSEM OR HORIZONTAL AND VERTICAL FACE SHIELDS. WEBS SHALL ALSO BE BEDDED IN ALL COURSES IN THE STARTING COURSE ON FOOTINGS, AND WHEN ADJACENT TO CELLS OR CAVITIES TO BE REINFORCED OR FILLED WITH CONCRETE OR GROUT. SOLID UNITS TO BE LAD WITH FULL HEAD AND END JOINTS.
 - JOINT REINFORCING AT 18 INCHES EXCEPT AS NOTED.
 - LAP JOINT REINFORCING 8 INCHES FOR STANDARD, 15 INCHES FOR HEAVY WEIGHT.
 - WHERE MASONRY UNITS ARE USED ABOVE HOLLOW UNITS A DIFFERENT THICKNESS, PROVIDE A CONTINUOUS COURSE OF SOLID MASONRY (OR SOLID GROUDED BLOCK) AT LEAST 8 INCHES HIGH BEYOND TRANSITION.

H. STRUCTURAL LUMBER

- STUDS: STRUCTURAL LUMBER: DOUGLAS FIR-LARCH #2 OR APPROVED EQUAL.

SP	1,500	825	90	565	1,500	1,600,000
248	1,250	725	90	565	1,500	1,600,000
248	1,250	800	90	565	1,500	1,600,000
2410	1,000	800	90	565	1,500	1,600,000
2412	975	800	90	565	1,500	1,600,000
- ROOF/WALL: ORIENTED STRAND BOARD: STRUCTURAL I, EXPOSURE 1, EXTERIOR GUE FOR ROOF. ROOF WALLS PANEL IDENTIFICATION INDEX 24/16 - 5/8 INCH OR 24/16 - 1/2 INCH (WITH PLYWOOD CLIPS AT ROOF).
- ROOF/WALL: PLYWOOD: C-PLYWOOD, STRUCTURAL I, EXPOSURE 1, EXTERIOR GUE FOR ROOF AND WALL PANEL IDENTIFICATION INDEX 24/75 - 5/8 INCH OR 24/75 - 1/2 INCH (WITH PLYWOOD CLIPS AT ROOF).
- SILL PLATES: NO. 2 SPRUCE-PINE-FIR, OR EQUAL: FC-675 PSI, F_y-70 PSI, E = 1,200,000 PSI.
- SPECIFICATIONS: UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION AND ERECTION SHALL BE GOVERNED BY THE LATEST REVISIONS OF:
 - NATIONAL SPECIFICATION FOR STEEL-GRANULE LUMBER AND ITS FASTENINGS.
 - U.S. PRODUCT STANDARD PS-1 FOR SOFTWOOD PLYWOOD - CONSTRUCTION AND INDUSTRIAL.
- CONNECTIONS:
 - JOISTS TO BEAMS - 16 GA. GALVANIZED STD. JOIST HANGERS, UNLESS SHOWN OTHERWISE.
 - PLYWOOD TO ROOF TRUSSES OR RAFTERS - NAIL - USE 8d RING SHANK NAILS AT 6 INCHES O/C AT PANEL EDGES AND 12 INCHES O/C AT INTERMEDIATE SUPPORTS. PROVIDE PLYWOOD CLIPS AT MID-SPAN OF PLYWOOD BETWEEN SUPPORTS.
- ALL STRUCTURAL WOOD TO BE SURFACED FOUR (4) SIDES (S-4-S) A AND MAXIMUM MOISTURE CONTENT OF 19 PERCENT.
- ALL LUMBER AND PLYWOOD IN CONTACT WITH CONCRETE, STUCCO, MASONRY OR OTHER CEMENTITIOUS MATERIALS SHALL BE TREATED WITH AN EPA ACCEPTABLE WOOD PRESERVATIVE (SUCH AS "ACQ" - ALKALINE-COPPER-QUATERNARY OR "CSA-A" COPPER AZOLE TYPE A & B).
- ALL WOOD CONNECTORS SHALL BE GALVANIZED STEEL OR RUST-PROOF PAINTED STEEL (U.O.S.). ALL GALVANIZED METAL CONNECTORS IN CONTACT WITH TREATED WOOD (ITEM #3) SHALL BE "TRIPLE-DIP Q-165" GALVANIZED. ANY FELD WELDS (INTERIOR OR EXTERIOR) OF SUCH CONNECTORS SHALL BE WIRE BRUSH CLEANED AND RUST PROOF PAINTED.
- MISCELLANEOUS:
 - USE ONE LINE OF SOLID BLOCKING OR CROSS BRIDGING AT 8" - 12" O/C MAX. FOR ALL JOISTS AND RAFTERS. USE SOLID BLOCKING AT JOIST AND RAFTER T.O.P. TO PREVENT END-RUPTURE.
 - USE SOLID BLOCKING AT MID-HEIGHT FOR ALL EXTERIOR STUD WALLS AND INTERIOR BEARING PARTITIONS.
 - USE DOUBLE STUDS UNDER BEAM AND UNTEL BEARING, UNLESS SHOWN OTHERWISE.

I. PREFABRICATED WOOD TRUSSES

- MATERIALS
 - LUMBER: SEE "STRUCTURAL LUMBER" SECTION FOR WOOD INFORMATION.
 - METAL CONNECTOR PLATES: GALVANIZED SHEET STEEL, ASTM A448 (LATEST EDITION) GRADE "A", COATING CLASS 600 FOR ASTM A575 (LATEST EDITION), MANUFACTURED WITH HOLES, PLUGS, TEETH, OR PRONGS UNIFORMLY SPACED AND FORMED.
 - SEE "STRUCTURAL LUMBER" SECTION FOR GALVANIZED CONNECTIONS FOR TREATED WOOD.
- DESIGN CRITERIA:
 - LOADING:

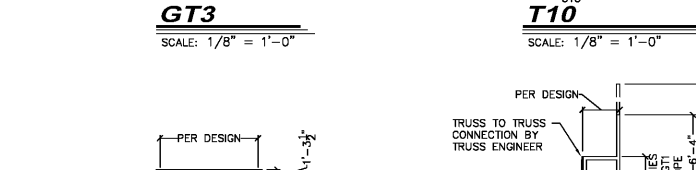
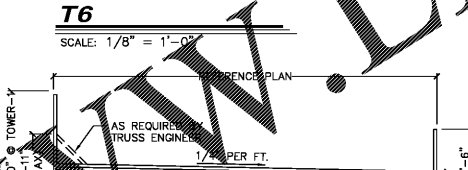
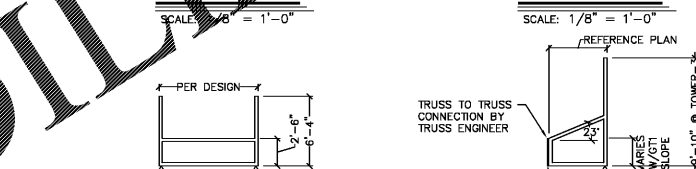
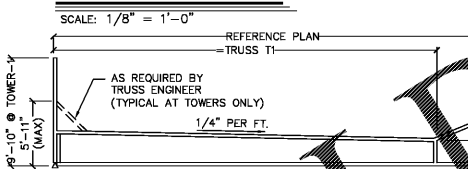
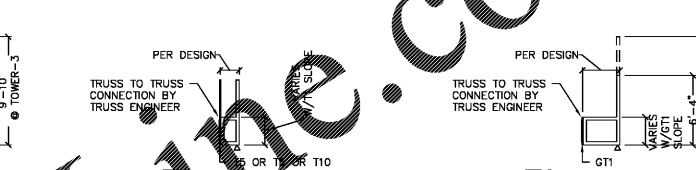
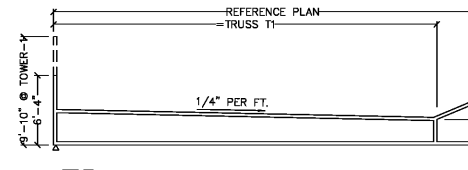
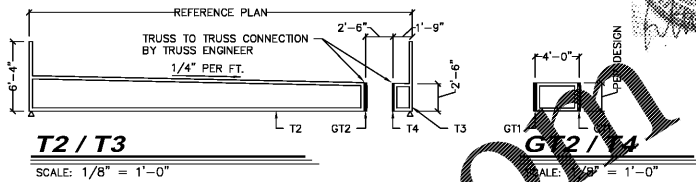
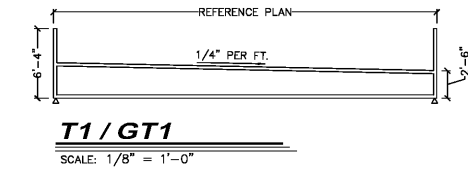
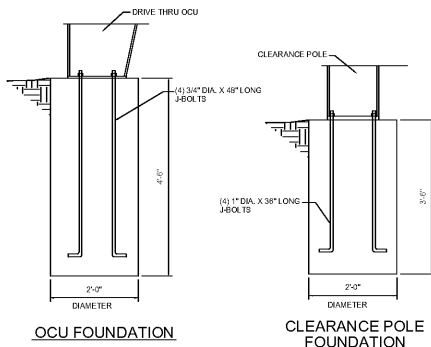
TOP CHORD LIVE LOAD:	20 PSF
TOP CHORD DEAD LOAD:	20 PSF + MECH. EQUIP.
BOTTOM CHORD DEAD LOAD:	10 PSF
NET WIND UPLIFT:	18 PSF
 - DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER, REGISTERED IN THE STATE OF THIS PROJECT, EXPERIENCED IN SIMILAR DESIGN, RETAINED BY THE TRUSS MANUFACTURER.
 - SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE ENGINEER RESPONSIBLE FOR THE TRUSS DESIGN. IN ADDITION, SIGNED AND SEALED DESIGN CALCULATIONS FOR THESE TRUSSES SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.
 - MEMBER SIZES SHOWN ARE MINIMUM SIZES.
 - MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/240.
 - MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.
- MISCELLANEOUS:
 - BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 1/2" BOLTS AT 4'-0" O.C. USE W/8 MEMBERS TOGETHER WITH 1/2" BOLTS AT 2'-0" O.C. AT CONCENTRATED LOADS, UNLESS OTHERWISE SPECIFIED BY THE TRUSS DESIGN ENGINEER. VERIFY ALL DIMENSIONS, ELEVATIONS AND SLOPE PRIOR TO MANUFACTURING. REPORT ANY DISCREPANCIES IMMEDIATELY TO THE ARCHITECT.
 - WOOD TRUSSES SHALL BE DESIGNED AND FABRICATED TO CONFORM TO THE REPORT AND SHOP DRAWINGS. ALL TRUSS MANUFACTURING AND FABRICATION SHALL BE DETAILED AS SPECIFICALLY SHOWN ON THE DRAWINGS. WELD CONNECTIONS ARE TO BE DETAILED AS SPECIFICALLY SHOWN ON THE DRAWINGS.
 - PROVIDE 2x4 BOTTOM CHORD BRIDGING AT A MAXIMUM SPA OF 17'-0" C.

J. ABBREVIATIONS:

- | | | | |
|--------|-----------------------|--------|--------------------------|
| T | TOP | W | WALL ELEVATION |
| B | BOTTOM | W | WALL ON GRADE |
| C | CONCRETE MASONRY UNIT | W | WELDED WIRE FABRIC |
| E.F. | EACH FACE | U.S. | UNLESS NOTED |
| E.W. | EACH WAY | T.P. | TOP OF FINISH |
| E.E. | EACH END | O.C. | ON CENTER |
| T.P. | TOP OF FINISH | T.B. | TOP OF BEARING ELEVATION |
| O.C. | ON CENTER | L.L.V. | LONG LEG VERTICAL |
| A.N.C. | ANCHOR | L.H. | LONG LEG HORIZONTAL |
| T.O.S. | TOP OF SUBSTRATION | | |

DESCRIPTION OF SPECIAL INSPECTION SERVICES	SERVICE	EXTENT
CONCRETE CONSTRUCTION	FIELD INSPECTION	PERIODIC
INSTALLATION OF REINFORCING BARS	FIELD INSPECTION	CONTINUOUS
INSTALLATION OF CAST-IN-PLACE BARS	FIELD INSPECTION	PERIODIC
FRESH CONCRETE TAMPING	FIELD TESTING	CONTINUOUS
CONCRETE CURING OPERATIONS	FIELD REVIEW	PERIODIC
EVALUATION OF CONCRETE STRENGTH	FIELD TESTING AND REVIEW	PERIODIC
TRUSS STRUCTURAL WOOD	FIELD INSPECTION	PERIODIC
VERIFY FABRICATION QUALITY CONTROL PROCEDURES	INDEPENDENT REVIEW	PERIODIC
INSPECTION OF WELDED JOINTS	SHOP AND FIELD INSPECTION	PERIODIC
ANCHORING AND OTHER FASTENING OF COMPONENTS	FIELD INSPECTION	PERIODIC
ROCK BOLTS AND SHEAR WALLS	FIELD INSPECTION	PERIODIC
VERIFY SITE PREPARATION COMPLIES WITH APPROVED SOILS REPORT	FIELD INSPECTION	CONTINUOUS
VERIFY ERECTION AND COMPACTION OF FILL MATERIALS COMPLIES WITH APPROVED SOILS REPORT	FIELD INSPECTION	CONTINUOUS
VERIFY DRY DENSITY OF COMPACTED FILL COMPLIES WITH APPROVED SOILS REPORT	FIELD TESTING	CONTINUOUS

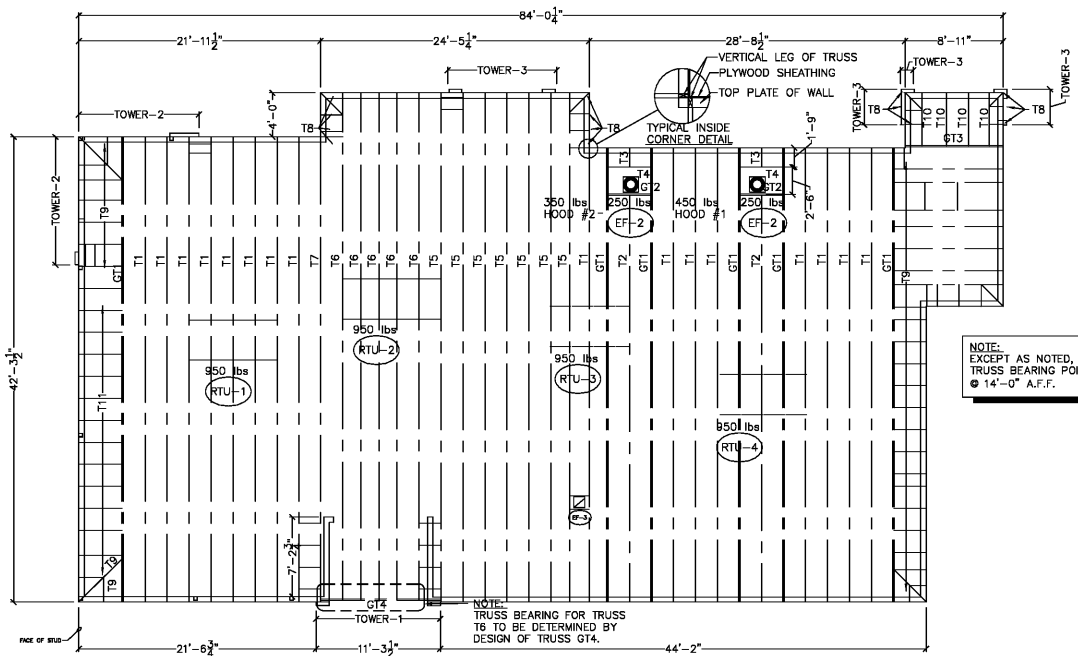
NOTE: THE INSPECTION AND TESTING AGENCIES SHALL BE ENGAGED BY THE OWNER OR THE OWNER'S AGENT AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR UNLESS WORK IS TO BE INSPECTED OR TESTED. ANY CONTACT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL PRIOR TO COMMENCING WORK. THE QUALIFICATIONS OF THE INSPECTION AGENCY MUST BE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL. SPECIAL INSTRUCTIONS, REPORTS AND A FINAL REPORT IN ACCORDANCE WITH SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF THE WORK IS APPROVED FOR OCCUPANCY.



- TRUSS PROFILE: PROVIDE FOR DESIGN INTENT ONLY. FIELD MEASUREMENTS REQUIRED FOR SITE SPECIFIC PROJECT.
- TRUSS DESIGN MEMBER SIZES TO BE DETERMINED BY THE TRUSS ENGINEER BASED ON LOCAL DESIGN LOAD REQUIREMENTS.
- REFER TO GENERAL NOTES FOR ADDITIONAL INFORMATION.

TRUSS PROFILES

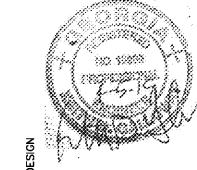
SCALE: 1/8" = 1'-0"



TRUSS LAYOUT

SCALE: 1/8" = 1'-0"

KEITH D. GALLOWAY, PE
STRUCTURAL ENGINEERING
1803 First Ave South
Pell City, AL 35125
205.338.4533 P



DATE: 03/26/19
CHECKED BY: E.H. MRW
REVISION
RELEASED FOR CONSTRUCTION
NO. DATE
04.5.19

BURGER KING

PREMIER KINGS, INC.
5529 CARMICHAEL ROAD
MONTGOMERY, ALABAMA 36117
PHONE: (205) 322-1778
www.inplata.com
LINSEY@BKBALABAMA.COM

2126 Morris Avenue, #103
Montgomery, AL 36117
Phone: (205) 322-1778
Fax: (205) 322-1778
email: info@inplata.com
www.inplata.com

james m. huckestein architect AIA
architecture • planning • interior design

PROJECT #: 18026-24
RCC-60 TALL 20/20 IMAGE: NOVEMBER 2018 DESIGN RELEASE
BURGER KING RESTAURANT
NEAR CORNER OF HWY 62 & JACKSON ST.
NAHANTA, GA

STRUCTURAL NOTES AND DETAILS

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