BUILDING CODES USED FOR DESIGNste

1. 2015 INTERNATIONAL BUILDING CODE

DESIGN LOADS

1. DESIGN LIVE LOADS:

   ROOF LIVE LOAD: 20 PSF

2. DESIGN SNOW LOADS:

   GROUND SNOW LOAD, Pg: 5 PSF
   FLAT ROOF SNOW LOAD, Pf: 3.5 PSF
   SNOW EXPOSURE FACTOR, Ce: 1.0
   SNOW LOAD IMPORTANCE FACTOR, I: 1.0
   THERMAL FACTOR, Ct: 1.0

3. WIND LOAD DESIGN CRITERIA:

   DESIGN WIND SPEED: 115 MPH
   WIND IMPORTANCE FACTOR: 1.0
   WIND EXPOSURE CATEGORY: C
   GCpi: +/- 0.55

4. SEISMIC LOAD DESIGN CRITERIA:

   SEISMIC IMPORTANCE FACTOR, I: 1.0
   SITE CLASS: D
   SPECTRAL RESPONSE ACCELERATION: Ss=0.198 g S1= 0.093 g
   SPECTRAL RESPONSE COEFFICIENTS: Sds=0.211 g Sd1= 0.149 g
   SEISMIC DESIGN CATEGORY: C
   BASIC SEISMIC-FORCED-RESISTING SYSTEM: LIGHT-FRAMED WALLS SHEATHED w/ WOOD STRUCTURAL PANELS
   SEISMIC RESPONSE COEFFICIENT, Cs: 0.032
   RESPONSE MODIFICATION FACTOR, R: 6.5
   SYSTEM OVER-STRENGTH FACTOR, OMEGAo: 2.5
   DEFLECTION AMPLIFICATION FACTOR Cd: 4
   ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE

GENERAL STRUCTURAL NOTES

1. THIS DRAWING SET IS TO BE VIEWED AS A WHOLE. ALL WORK PERTAINING TO A SPECIFIC CONTRACTOR MAY OR MAY NOT BE SHOWN ON SPECIFIC DRAWING SECTIONS. IT IS EACH SUBCONTRACTOR'S RESPONSIBILITY TO PREPARE HIS BID FROM A COMPLETE SET OF PLANS.

2. THE CONTRACTOR SHALL FOLLOW WRITTEN DIMENSIONS ONLY. DO NOT SCALE DRAWINGS. DIMENSIONS NOT SHOWN ON PLAN TO BE COORDINATED WITH ARCHITECTURAL PLANS.

3. ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY AT ANY SIMILAR SITUATION ELSEWHERE ON THE JOB EXCEPT WHERE A DIFFERENT DETAIL OR SECTION IS SHOWN.

4. THE STRUCTURE SHALL BE ADEQUATELY BRACED AND SHORED DURING ERECTION AGAINST WIND AND ERECTION LOADS. STRUCTURAL MEMBERS ARE DESIGNED FOR "IN-PLACE" LOADS ONLY.

5. THE GENERAL CONTRACTOR SHALL VERIFY ALL OPENING SIZES, PAD SIZES AND LOCATIONS WITH THE RESPECTIVE CONTRACTORS.

6. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES BETWEEN CONSTRUCTION DOCUMENTS AND ACTUAL FIELD CONDITIONS.

7. ALL HOLES THROUGH EXISTING CONSTRUCTION SHALL BE CORE DRILLED OR SAWCUT.

8. THE VARIOUS SUBCONTRACTORS ARE RESPONSIBLE FOR PLACING OF SLEEVES, OUTLET BOXES, ANCHORS, VENT OPENINGS, ETC. THAT MAY BE REQUIRED IN FOUNDATION WALLS. CONSTRUCTION MANAGER SHALL COORDINATE ALL PLACEMENT OF ITEMS IN FOUNDATION WALLS.

9. SEE ARCHITECTURAL PLANS FOR ADDITIONAL DETAILS AND INFORMATION.

10. ALL ELEVATIONS GIVEN ARE REFERENCED TO FINISHED FLOOR ELEVATIONS AT 100'-0".

EXCAVATION AND EARTHWORK

1. WATER LEVELS INDICATED ON THE BORING LOGS MAY BE SUBJECT TO SEASONAL AND/OR ANNUAL VARIATIONS. A DEWATERING SYSTEM OF SUFFICIENT CAPACITY SHALL BE INSTALLED AND OPERATED TO MAINTAIN THE CONSTRUCTION AREA FREE OF WATER AT ALL TIMES.

2. THE BEARING VALUE OF THE SOIL IS PER REPORT BY: **EARTH SCIENCE, LLC DATED AUGUST 28, 2018** IS BASED ON THE FOLLOWING NET ALLOWABLE BEARING PRESSURE:

   CONT. WALL FOOTINGS 2,000 PSF

3. ALL FOOTING EXCAVATIONS SHALL BE INSPECTED, PRIOR TO CONCRETE PLACEMENT, BY A SOILS ENGINEER TO VERIFY SUITABLE BEARING MATERIAL OF CAPACITY AS SPECIFIED.

4. NOTIFY THE OWNER'S REPRESENTATIVE WHEN ADDITIONAL EXCAVATION IS REQUIRED TO REACH SUITABLE BEARING MATERIAL.

5. THE SOILS ENGINEER SHALL CERTIFY IN WRITING THAT ALL FOUNDATIONS WERE PLACED ON SOIL WITH THE BEARING VALUE AS SPECIFIED.

6. WITHIN THE EXCAVATION AREA OF THE FOUNDATIONS, ALL VEGETATION, TOPSOIL, PREVIOUSLY PLACED FILL AND UNSUITABLE SOILS SHALL BE REMOVED. ALL FOOTINGS TO BEAR ON VIRGIN SOIL OR PROPERLY PLACED AND COMPACTED ENGINEERED FILL.

7. FOUNDATION DESIGN DOES NOT ACCOUNT FOR WINTER CONSTRUCTION. ANY UNENCLOSED/UNHEATED SPACES SHALL BE ADEQUATELY PROTECTED AGAINST FROST DURING WINTER CONSTRUCTION BY CONTRACTOR.

8. IF ANY SOFT SPOTS OR AREAS QUESTIONABLE FOR ANY REASON ARE ENCOUNTERED BY THE CONTRACTOR, ARCHITECT/ENGINEER SHALL BE ADVISED IMMEDIATELY SO THAT ANY REQUIRED ACTION MAY BE TAKEN PRIOR TO CONTINUATION OF CONSTRUCTION IN THAT AREA.

SPECIAL INSPECTIONS

1. REFER TO THE "STATEMENT OF SPECIAL INSPECTIONS" FOR THE LIST OF ELEMENTS OF CONSTRUCTION THAT SHALL REQUIRE SPECIAL INSPECTION PER IBC CODE SECTION 1704. THE OWNER SHALL EMPLOY A SPECIAL INSPECTION AGENCY APPROVED BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO THE START OF WORK. COPIES OF ALL INSPECTION REPORTS SHALL BE SUBMITTED TO THE ARCHITECT OF RECORD, STRUCTURAL ENGINEER OF RECORD, AND BUILDING INSPECTOR IN A TIMELY MANNER.

2. THE SPECIAL INSPECTIONS IDENTIFIED ON THE PLANS ARE IN ADDITION TO, AND NOT A SUBSTITUTE FOR, THOSE INSPECTIONS REQUIRED TO BE PERFORMED BY A BUILDING INSPECTOR.

3. CONTINUOUS INSPECTION SHALL BE PROVIDED DURING THE PERFORMANCE OF WORK REQUIRING SPECIAL INSPECTION, UNLESS OTHERWISE NOTED. WHEN WORK IN MORE THAN ONE CATEGORY OF WORK REQUIRING SPECIAL INSPECTION IS TO BE PERFORMED SIMULTANEOUSLY, OR THE GEOGRAPHIC LOCATION OF THE WORK IS SUCH THAT IT CANNOT BE CONTINUOUSLY OBSERVED, IT SHALL BE THE RESPONSIBILITY OF THE AGENT TO EMPLOY A SUFFICIENT NUMBER OF SPECIAL INSPECTORS TO ASSURE THAT ALL WORK IS CONTINUOUSLY INSPECTED IN ACCORDANCE WITH THOSE PROVISIONS.

CONCRETE

1. ALL CONCRETE WORK INCLUDING FORMING, REINFORCING, MIXING, PLACING AND CURING SHALL BE DONE IN ACCORDANCE WITH THE ACI MANUAL OF CONCRETE PRACTICE INCLUDING "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", ACI 318 AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE", ACI 301.

2. ALL CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH AND DENSITY, IN ACCORDANCE WITH THE FOLLOWING:

INTENDED USE	MINIMUM 28 DAY STRENGTH f'c (ksi)	CONCRETE DENSITY	MAXIMUM W/C RATIO (INCLUDING FLY ASH)	MINIMUM CEMENT MATERIAL (#/CY INCLUDING FLY ASH)	MAXIMUM AGGREGATE (in) (A)	SLUMP LIMITS (in) (+0", -2")	TOTAL AIR LIMITS (%0) (B)	REQUIRED ADMIXTURES (C)
FOOTINGS	4	145	0.48	564	1	4	-	-
CONCRETE EXPOSED TO DEICERS	4	145	0.40	564	3/4	4	6	AE, WR
INTERIOR SLAB ON GRADE	4	145	0.50	564	1	4	N	WR
ALL CONCRETE NOT OTHERWISE SPECIFIED	4	145	0.40	564	3/4	4	6	AE

(A) FOR MAXIMUM COARSE AGGREGATE SIZE INDICATED, USE THE FOLLOWING AGGREGATE SIZE NUMBERS PER A.S.T.M. C33:
   3/8" - #8 AGGREGATE   1" - #57 AGGREGATE
   3/4" - #67 AGGREGATE   1 1/2" - #467 AGGREGATE

(B) TOTAL AIR CONTENT LIMITS INCLUDE BOTH ENTRAINED AND ENTRAPPED AIR +/- 1 1/2%. "N" IN COLUMN INDICATES THE ADDITION OF ENTRAINED AIR IS NOT PERMITTED.

(C) ABBREVIATIONS FOR REQUIRED ADMIXTURES AS FOLLOWS: AE = AIR- ENTRAINING ADMIXTURE, WR = WATER REDUCING ADMIXTURE.

3. FLY ASH ALLOWANCES:

   20% MAXIMUM BY WEIGHT IN FOOTINGS
   15% MAXIMUM BY WEIGHT IN SLABS

4. COORDINATE CONCRETE WORK WITH THAT OF OTHER TRADES TO ALLOW FOR SETTING OF SLEEVES, ACCESSORIES, ETC.

5. ALL REINFORCING STEEL, ANCHOR BOLTS (SHALL NOT BE HAND SET), DOWELS AND INSERTS SHALL BE WELL SECURED IN POSITION PRIOR TO PLACING CONCRETE.

6. TEST CYLINDERS WILL BE REQUIRED, AND RECORDS OF RESULTS SHALL BE TRANSMITTED TO ENGINEER. SLUMP TESTS ARE RECOMMENDED.

7. PROVIDE A 3/4" CHAMFER ON ALL EXPOSED CORNERS UNLESS OTHERWISE NOTED ON THE DRAWINGS.

8. ALL COLD JOINTS SHALL BE ROUGHENED AND CLEANED PRIOR TO PLACING CONCRETE.

9. CONSTRUCTION JOINTS IN CONCRETE INDICATED WITH A ROUGH, CLEAN SURFACE SHALL HAVE A 1/4" MAXIMUM AMPLITUDE.

PRE-FABRICATED WOOD TRUSSES

DESIGN CRITERIA:

   TOP CHORD: 20 P.S.F. LIVE LOADS, 10 P.S.F. DEAD LOAD
   BOTTOM CHORD: 10 P.S.F. DEAD LOAD
   LOAD DURATION FACTOR: 125% LUMBER ONLY
   TRUSS SPACING: 24"oc MAXIMUM
   DEFLECTION: L/240 MAXIMUM LIVE LOAD DEFLECTION
   CAMBER: DEAD LOAD

1. SUBMITTALS: SUBMIT TRUSS CALCULATIONS, SHOP DRAWINGS AND ERECTION PLANS FOR REVIEW. CALCULATIONS SHALL BE SEALED AND SIGNED ACROSS THE SEAL BY A REGISTERED ENGINEER. SHOW DESIGN LOADS, STRESS DIAPHRAGMS, AND DESIGN OF EACH MEMBER.

2. DESIGN AND FABRICATION SHALL BE IN ACCORDANCE WITH THE TRUSS PLATE INSTITUTE PUBLICATION "DESIGN SPECIFICATION FOR METAL PLATE CONNECTED WOOD TRUSSES", LATEST EDITION.

3. BRACING: PROVIDE ALL PERMANENT TRUSS BRACING INDICATED ON DRAWINGS OR SPECIFIED BY TRUSS MANUFACTURER. IN ADDITION, PROVIDE TEMPORARY BRACING AS INDICATED IN THE TRUSS PLATE INSTITUTE BOOKLET "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS BWT-76".

4. ANCHORAGE: PROVIDE SIMPSON FRAMING ANCHOR H2.5 UNLESS NOTED OTHERWISE TO RESIST UPLIFT AT EACH ROOF TRUSS BEARING POINT.

5. FIELD MODIFICATIONS: NO FIELD MODIFICATIONS OF TRUSSES IS PERMITTED UNLESS FABRICATOR PROVIDES CALCULATIONS AND DRAWINGS DETAILING THE MODIFICATIONS. CALCULATIONS AND DRAWINGS SHALL BE SIGNED AND SEALED BY A REGISTERED ENGINEER.

6. REFER TO DEFERRED SUBMITTALS FOR ADDITIONAL REQUIREMENTS.

WOOD FRAMING

1. WOOD FRAMING SHALL CONFORM TO THE "LUMBER SCHEDULE" UNLESS OTHERWISE NOTED. GRADING SHALL BE IN ACCORDANCE WITH THE 2015 EDITION OF THE INTERNATIONAL BUILDING CODE.

2. FOR STRUCTURAL GLUE-LAMINATED TIMBER MEMBERS, AN AITC CERTIFICATION OF CONFORMANCE ISSUED BY A CURRENT ICC-APPROVED QUALITY CONTROL AGENCY SHALL BE SUBMITTED TO THE BUILDING INSPECTOR PRIOR TO INSTALLATION.

3. MINIMUM NAILING AND BOLTING OF WOOD FRAMING MEMBERS AND WOOD SHEATHING SHALL BE AS INDICATED IN THE "WOOD FRAMING CONNECTIONS SCHEDULE" UNLESS OTHERWISE NOTED.

4. ALL NAILS SHALL BE **GALVANIZED COMMON WIRE NAILS** UNLESS OTHERWISE NOTED. NAILS IN CONTACT WITH FIRE RETARDANT TREATED OR PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED (ASTM A153) OR STAINLESS STEEL (TYPE 304 OR 316). WHEN REQUIRED TO PREVENT SPLITTING, PRE-DRILL FOR NAILS WITH 1/8" DIAMETER DRILL BIT.

5. BOLTS AND LAG SCREWS SHALL CONFORM TO ASTM A307 AND ANSI/ASME STANDARD B18.2.1-1981, **AND SHALL BE GALVANIZED**. BOLTS AND LAG SCREWS IN CONTACT WITH FIRE RETARDANT TREATED OR PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED (ASTM A153) OR STAINLESS STEEL (TYPE 304 OR 316). STANDARD WASHERS SHALL BE PROVIDED UNDER HEAD AND NUT OF ALL BOLTS IN WOOD FRAMING. BOLT THREADS SHALL NOT BEAR ON WOOD. DRILLED HOLES FOR BOLTS SHALL BE 1/16" LARGER IN DIAMETER THAN BOLT.

6. ALL BOLTS SHALL BE RETIGHTENED IMMEDIATELY PRIOR TO CLOSING IN FRAMING.

7. METAL FRAMING CONNECTORS SHALL BE "SIMPSON" BRAND OR ENGINEER APPROVED EQUIVALENT **AND SHALL BE GALVANIZED** . METAL FRAMING CONNECTORS IN CONTACT WITH FIRE RETARDANT TREATED OR PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED (ASTM A123) OR STAINLESS STEEL (TYPE 316L). METAL FRAMING CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S LATEST PUBLISHED INSTALLATION INSTRUCTIONS USING THE LARGER SIZE AND QUANTITY OF FASTENERS INDICATED, UNLESS OTHERWISE NOTED

REINFORCING STEEL

1. NON-WELDED STEEL BAR REINFORCING SHALL CONFORM TO ASTM A615, GRADE 60. WELDED STEEL BAR REINFORCING SHALL CONFORM TO ASTM A706.

2. WELDING OF REINFORCING STEEL SHALL BE PERFORMED BY A.W.S. QUALIFIED WELDERS IN CONFORMANCE WITH A.W.S. D1.1 USING E90 SERIES ELECTRODES UNLESS OTHERWISE NOTED ON THE DRAWINGS.

3. BAR LAP SPLICE LENGTHS FOR BARS INSTALLED IN CONCRETE SHALL BE 48 BAR DIAMETERS UNLESS OTHERWISE NOTED ON THE DRAWINGS. BAR LAP SPLICE LENGTHS FOR BARS INSTALLED IN MASONRY SHALL BE AS INDICATED IN THE "TYPICAL REINFORCING BAR SPLICES IN MASONRY" DETAIL PROVIDED ON THE STRUCTURAL DRAWINGS.

4. MINIMUM CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED ON THE DRAWINGS:

   CONCRETE CAST AGAINST EARTH 3"
   FORMED CONCRETE AGAINST EARTH 2"
   CONCRETE EXPOSED TO WEATHER 2"
   ALL OTHER 1 1/2"

5. ALL DETAILING, FABRICATION AND ERECTION OF REINFORCING STEEL SHALL CONFORM TO THE LATEST EDITION OF ACI 315, DETAILS AND DETAILING OF CONCRETE REINFORCEMENT.

TENSION LAP SPLICE LENGTH-60 KSI REBAR NOTES

1. FOR HORIZONTAL BARS, VALUES IN THE TABLE SHALL BE MULTIPLIED BY 1.3 WHERE MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST BELOW THE BAR.

2. VALUES IN THE TABLE SHALL BE MULTIPLIED BY 1.5 FOR EPOXY COATED BARS WITH CLEAR COVER LESS THAN 3 BAR DIAMETERS OR CLEAR SPACING LESS THAN 6 BAR DIAMETERS. MULTIPLY VALUES IN TABLE BY 1.2 FOR ALL OTHER EPOXY COATED BARS.

3. VALUES IN TABLE NEED NOT TO BE MULTIPLIED BY MORE THAN 1.7 DUE TO THE INCREASE FROM NOTES 1 AND 2.

4. VALUES IN THE TABLE SHALL BE MULTIPLIED BY 1.33 WHERE LIGHT WEIGHT CONCRETE IS USED.

5. LAP SPLICES IN TENSION ARE NOT PERMITTED FOR BAR LARGER THAN #11. A FULL MECHANICAL OR FULL WELDED SPLICE SHALL DEVELOPE AT LEAST 1.25fy OF THE BAR.

6. WHERE CLEAR SPACING OF BARS BEING SPLICED IS AT LEAST 2 BAR DIAMETERS AND CLEAR COVER AT LEAST 1 BAR DIAMETER, USE CASE 1. FOR ALL OTHER BAR ARRANGEMENTS, USE CASE 2.

7. VALUES IN THE TABLE ARE BASED ON 60ksi REBAR. FOR OTHER REBAR YIELD STRENGTHS, MULTIPLY VALUES IN THE TABLE BY THE SPECIFIED YIELD STRENGTH DIVIDED BY 60.

8. WHERE BARS OF DIFFERENT SIZES ARE SPLICED, PROVIDE THE LAP LENGTH OF THE LARGER BAR.

9. WELDED WIRE REINFORCEMENT (DEFORMED OR PLAIN WIRE) SHALL BE LAPPED ONE FULL MESH SQUARE PLUS 2 INCHES MINIMUM, BUT NOT LESS THAN 8 INCHES.

10. REBAR IN ALL CONCRETE MEMBERS SHALL BE SPLICED IN ACCORDANCE WITH "TENSION LAP SPLICE LENGTH" TABLE, UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS.

TENSION LAP SPLICE LENGTH (INCHES) - 60 KSI REBAR TABLE

f'c =	3,000psi	3,000psi	3,500psi	3,500psi	4,000psi	4,000psi	5,000psi	5,000psi
BAR SIZE	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2
#3	22	33	20	30	19	28	17	25
#4	29	43	27	40	25	37	23	34
#5	36	54	33	50	31	47	28	42
#6	43	65	40	60	37	56	34	50
#7	63	94	58	87	54	81	49	73
#8	72	107	66	99	62	93	56	83
#9	81	121	75	112	70	105	63	94
#10	91	136	84	126	79	118	71	106
#11	101	151	93	140	87	131	78	117

REINFORCED MASONRY

1. ALL MASONRY SHALL CONFORM TO ACI 530, "BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES". CONCRETE BLOCK SHALL CONFORM TO ASTM C90, TYPE1, GRADE N, NORMAL WEIGHT CMU, MINIMUM PRISM STRENGTH (f'm) = 2,000 PSI. GROUT SHALL COMPLY WITH REQUIREMENTS OF ASTM C476. MORTAR SHALL COMPLY WITH REQUIREMENTS OF ASTM C270.

2. REINFORCED MASONRY REQUIRES CONTINUOUS INSPECTION.

3. ALL MASONRY WALLS SHALL HAVE HORIZONTAL REINFORCING CONSISTING OF GALVANIZED HEAVY WEIGHT. "DUR-O-WAL" OR EQUAL. ALL REINFORCING SHALL BE LOCATED AT 8"oc UNLESS NOTED OTHERWISE ON PLAN AND/OR SECTIONS.

4. SUPPLY VERTICAL REINFORCING IN MINIMUM 4'-0" PLUS 48 BAR DIAMETER LAP LENGTHS.

5. DOWELS AS SHOWN SHALL MATCH SIZE AND NUMBER OF WALL BARS REINFORCING UNLESS OTHERWISE NOTED. HOOK INTO FOOTING 1'-0" MINIMUM AND LAP 48 BAR DIAMETERS WITH MAIN STEEL.

6. WALL CONSTRUCTION LIFTS FOR REINFORCING BARS AND INSULATION FILL SHALL BE PER ACI 530.

7. TYPE "S" MORTAR REQUIRED FOR ALL WALLS UNLESS NOTED OTHERWISE.

8. ALL BOLTS, ANCHORS, ETC., INSERTED IN THE WALLS, SHALL BE GROUTED SOLID INTO POSITION WITH 6" MINIMUM EDGE DISTANCE FROM ANCHOR TO EDGE OF GROUTED PORTION OF CMU IN ALL DIRECTIONS.

9. REINFORCING SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED ON DRAWINGS.

10. DOWELS FROM C.I.P. CONCRETE SHALL MATCH THE VERTICAL REINFORCEMENT IN THE WALL ABOVE UNLESS NOTED OTHERWISE. SUCH DOWELS SHALL BE FURNISHED BY THE CONCRETE CONTRACTOR.

11. REINFORCING ENTIRELY WITHIN THE MASONRY SHALL BE FURNISHED BY THE MASONRY CONTRACTOR.

12. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL BLOCK CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN 6 VERTICAL. DOWEL MAY BE GROUTED INTO A CELL IN VERTICAL ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING, AS LONG AS THE CENTER-TO-CENTER SPACE BETWEEN THE WALL REINFORCING AND THE DOWEL DOES NOT EXCEED 5 FEET.

13. SPLICED REINFORCING SHALL BE LAPPED 48 BAR DIAMETERS OR 24 INCHES, WHICHEVER IS GREATER, SPLICED BARS SHALL BE WIRED TOGETHER. WHERE ADJACENT SPLICES OCCUR IN THE SAME CELL, SPLICE LENGTH SHALL BE INCREASED TO 52 BAR DIAMETERS.

14. VERTICAL BARS SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 192 DIAMETERS OF THE REINFORCING OR 10'-0".

15. VERTICAL REINFORCING BARS SHALL HAVE A MINIMUM CLEARANCE OF 3/4 OF AN INCH FROM THE MASONRY AND 1 INCH TO ADJACENT BARS AND NOT LESS THAN ONE BAR DIAMETER BETWEEN BARS NOT SPLICED.

16. REINFORCING STEEL SHALL BE SECURELY IN PLACE AND INSPECTED BEFORE GROUTING STARTS.

17. VERTICAL GROUTING MAY BE EITHER 'LOW LIFT' OR 'HIGH LIFT' AT THE CONTRACTOR'S OPTION.

18. VERTICAL CELLS THAT WILL BE GROUTED SHALL HAVE VERTICAL ALIGNMENT TO MAINTAIN A CONTINUOUS UNOBSTRUCTED CELL AREA NOT LESS THAN 2"x3".

19. GROUTING SHALL BE STOPPED 1 1/2" BELOW THE TOP OF A COURSE, AND 1/2" BELOW THE TOP OF A BOND BEAM, SO AS TO FORM A KEY AT THE POUR JOINT.

DEFERRED SUBMITTALS

THE FOLLOWING MUST BE SIGNED & SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THIS PROJECT IS LOCATED AND SUBMITTED TO ENGINEER OF RECORD;

1. PRE-FABRICATED WOOD TRUSS CALCULATIONS AND FABRICATION DRAWINGS. EACH TRUSS SHALL BE LEGIBLY BRANDED, MARKED OR OTHERWISE HAVE PERMANENTLY AFFIXED THERE TO THE FOLLOWING INFORMATION LOCATED WITHIN 2 FEET OF THE CENTER OF THE SPAN ON THE ACE OF THE BOTTOM CHORD

   A. IDENTITY OF THE COMPANY MANUFACTURING THE TRUSS
   B. THE DESIGN LOADS
   C. THE SPACING OF THE TRUSSES
   D. PLAN AND DETAILS FOR THE LOCATIONS OF ALL ERECTION/TEMPORARY AND PERMANENT LATERAL AND DIAGONAL BRACING AND/OR BLOCKING REQUIRED IN THE TOP CHORD, WEB, AND BOTTOM CHORD PLANES (DIAPHRAGMS EXCLUDED).
   E. FRAMING PLAN LAYOUT (DIMENSIONED/TO SCALE)

SHOP DRAWINGS

1. SHOP DRAWINGS, UNLESS OTHERWISE NOTED, SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

2. PRIOR TO SUBMITTAL, THE CONTRACTOR SHALL REVIEW THE SHOP DRAWINGS AND MAKE ANY CORRECTIONS REQUIRED. THE CONTRACTOR SHALL STAMP AND SIGN THE DRAWINGS THAT HE HAS REVIEWED THEM.

3. SHOP DRAWINGS SHALL BE FURNISHED FOR ALL STRUCTURAL COMPONENTS.

4. STRUCTURAL DRAWINGS ARE THE SOLE PROPERTY OF AEDIFICA CASE ENGINEERING. REPRODUCTION OF STRUCTURAL DRAWINGS FOR USE IN SHOP DRAWING SUBMITTALS IS NOT ACCEPTABLE.

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Revisions

02/26/18 - Prototype Update

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Sheet Title: GENERAL NOTES

Aedifica | case engineering
St. Louis - Montréal
mechanical electrical plumbing fire protection structural

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S-1
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