Roof

Area	GCp +/- GCpi 10 sf	50 sf	100 sf	500 sf	Surface Pressure (psf) 10 sf	50 sf	100 sf	500 sf
Negative Zone 1	-1.18	-1.11	-1.08	-1.08	-31.4	-29.6	-28.8	-28.8
Negative Zone 2	-1.98	-1.49	-1.28	-1.28	-52.8	-39.7	-34.1	-34.1
Negative Zone 3	-1.98	-1.49	-1.28	-1.28	-79.4	-47.7	-34.1	-34.1
Positive Zone 1	0.48	0.41	0.38	0.38	16.0	16.0	16.0	16.0
Positive Zones 2 & 3	1.08	0.97	0.92	0.81	28.8	25.8	24.5	21.6
Overhang Zone 1&2	-1.7	-1.63	-1.6	-1.1	-45.3	-43.4	-42.6	-29.3
Overhang Zone 3	-1.7	-1.63	-1.6	-1.1	-45.3	-43.4	-42.6	-29.3

Negative zone 3 = zone 2 if parapet height >= 3ft.
Overhang pressures in the table above assume an internal pressure coefficient (Gcpi) of 0.0
Overhang soffit pressure equals adj wall pressure (which includes internal pressure of 4.8 psf)

Parapet

qp = 27.9 psf

Parapet Face Pressure (psf)		10 sf
Windward	Zone 4 :	30.1
	Zone 5 :	30.1
Leeward	Zone 4 :	-55.2
	Zone 5 :	-83.0**

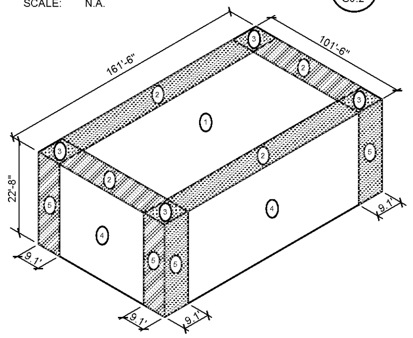
**If parapet height is 3 feet or greater, Leeward Zone 5 pressure is equal to Leeward Zone 4 pressure

Walls

Area	GCp +/- GCpi 10 sf	100 sf	200 sf	500 sf	Surface Pressure (psf) 10 sf	100 sf	200 sf	500 sf
Negative Zone 4	-1.17	-1.01	-0.96	-0.90	-31.2	-26.9	-25.7	-24.0
Negative Zone 5	-1.44	-1.12	-1.03	-0.90	-38.4	-29.9	-27.4	-24.0
Positive Zone 4 & 5	1.08	0.92	0.87	0.81	28.8	24.5	23.3	21.6

Note: GCp reduced by 10% due to roof angle <= 10 deg.

SCHEDULE OF COMPONENTS AND CLADDING WIND PRESSURES (1 / S0.2)
SCALE: N.A.



NOTE:
FOR WIND PRESSURE, SEE SCHEDULE 1 / S0.2. NUMBERS INDICATE ZONES.

DIAGRAM OF COMPONENTS AND CLADDING WIND PRESSURES (1A / S0.2)
SCALE: N.A.

STEEL COLUMN SCHEDULE

MARK	C1	C2	C3
ROOF / SECOND FLOOR	HSS 4 x 4 x 1/4 (note 2)	—	HSS 4 x 4 x 3/8 (note 2)
FIRST FLOOR/FOUND.	HSS 4 x 4 x 1/4 (note 1)	HSS 4 x 4 x 1/4 (note 1)	HSS 4 x 4 x 3/8 (note 1)
BASE PLATE	3/4 x 10 x 0'-10	3/4 x 10 x 0'-10	3/4 x 10 x 0'-10
ANCHOR BOLTS	SEE 8 / S2.1	SEE 8 / S2.1	SEE 8 / S2.1
REMARKS			

NOTES:
1. SEE DETAIL 8 / S2.1 FOR TYPICAL COLUMN BASE PLATE LAYOUT.
2. CONTINUE COLUMN TO THE TOP OF THE PARAPET WHERE REQUIRED.

SCHEDULE OF STEEL COLUMNS (2 / S0.2)
SCALE: NONE

COLUMN FOOTING SCHEDULE

MARK	F4.0	F4.5	F5.5
SIZE	4'-0" x 4'-0"	4'-6" x 4'-6"	5'-6" x 5'-6"
DEPTH	1'-2"	1'-2"	1'-2"
REINFORCEMENT	(7) #4 BOTTOM EACH WAY	(7) #4 BOTTOM EACH WAY	(6) #5 BOTTOM EACH WAY
REMARKS	-	-	-

SCHEDULE OF COLUMN FOOTINGS (3 / S0.2)
SCALE: NONE

SHEAR WALL SCHEDULE

SHEAR WALL DESIGNATION	DESCRIPTION	END POST
⚠ 1	TYPICAL INTERIOR STUDS AT 2'-6" ON CENTER SHEATHED WITH 26 GAGE U-PANELS ON BOTH SIDES. SEE 1 / S5.1 AND 3B / S5.1 FOR WALL CONSTRUCTION.	(2) TYPICAL INTERIOR STUDS

NOTES:
29 GAGE U-PANEL PROPERTIES: Ix (TOP) = 0.0151 IN^4 / FOOT, Ix (BOTTOM) = 0.106 IN^4 / FOOT
26 GAGE U-PANEL PROPERTIES: Ix (TOP) = 0.0223 IN^4 / FOOT, Ix (BOTTOM) = 0.0152 IN^4 / FOOT

SCHEDULE OF SHEAR WALLS (4 / S0.2)
SCALE: NONE

HEADER SCHEDULE

HEADER DESIGNATION	DESCRIPTION	JAMB	SILL TRACK SEE NOTE 1	NO. OF SCREWS EACH END
DH4E	(2) 400 S 250-54 BOXED WITH 600 T 200-54 TRACKS TOP AND BOTTOM. SEE DETAIL 9 / S5.1	(2) TYPICAL EXTERIOR STUDS	-	6
DH6	(2) 600 S 250-68 BACK-TO-BACK. SEE DETAIL 6 / S5.1	(2) TYPICAL INTERIOR STUDS	-	8
DH8	(2) 800 S 250-68 BACK-TO-BACK. SEE DETAIL 6 / S5.1	(2) TYPICAL INTERIOR STUDS	-	10
DH10	(2) 1000 S 250-97 BACK-TO-BACK. SEE DETAIL 6 / S5.1	(2) TYPICAL INTERIOR STUDS	[illegible]	20
H8	800 S 250-68. ATTACH TO EACH LOAD BEARING STUD WITH (6) #12 SCREWS. SIMILAR TO DETAIL 7 / S5.1	TYPICAL INTERIOR STUD	-	6

NOTES:
1. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS WHERE SILL IS REQUIRED.

SCHEDULE OF HEADERS (5 / S0.2)
SCALE: NONE

COLD-FORMED STEEL MEMBER NAMING SYSTEM AND DEFINITIONS

DEPTH	MEMBER	FLANGE	[RETURN LIP]	THICKNESS (MILS)
250 (2 1/2")	S (STUD)	137 (1 3/8")	[3/8"]	33 (20 GAUGE)
350 (3 1/2")	T (TRACK)	162 (1 5/8")	[1/2"]	43 (18 GAUGE)
362 (3 5/8")	Z (Z-PURLIN)	200 (2")	[5/8"]	54 (16 GAUGE)
400 (4")		250 (2 1/2")	[5/8"]	68 (14 GAUGE)
550 (5 1/2")		300 (3")	[5/8"]	97 (12 GAUGE)
600 (6")				
800 (8")				
925 (9 1/4")				
1000 (10")				
1200 (12")				
1400 (14")				

EXAMPLES:

STUD	600 S 200 - 54
TRACK	400 T 250 - 33
Z - PURLIN	800 Z 200 - 68

SCHEDULE OF COLD FORMED MEMBERS (6 / S0.2)
SCALE: NONE

COLD-FORMED STEEL CONNECTORS AND FASTENING SYSTEM DESCRIPTIONS AND DEFINITIONS

SELF DRILLING SCREWS

DRAWING DEFINITION	SIZING INFORMATION	PROVIDE SCREWS COMPLYING WITH ASTM C1513
#8	#8 - 18 x 3/4" HHW	REFERENCE PUBLICATIONS
#10	#10 - 16 x 3/4" HHW	ICC ESR - 3294 ELCO CONSTRUCTION PRODUCTS
#12	#12 - 14 x 1" HHW	ICC ESR - 1976 ITW BUILDEX
#14	1/4 - 14 x 7/8" HHW	ICC ESR - 2169 HILTI, INC.

POWDER ACTUATED FASTENERS (PAF)

DRAWING DEFINITION	SHANK DIAMETER	MANUFACTURER'S MODEL: SIMPSON STRONG-TIE	MANUFACTURER'S MODEL: HILTI	MINIMUM PENETRATION	REFERENCE PUBLICATIONS
0.157" Ø PAF	0.157"	PDP	X-U 15	1 1/4"	ICC ESR - 2269 HILTI (X-U); ICC ESR - 1663 HILTI (DS)
0.177" Ø PAF	0.177"	PDPH	DS	1 1/4"	ICC ESR - 2138 SIMPSON STRONG-TIE

SCHEDULE OF COLD FORMED STEEL CONNECTORS (7 / S0.2)
SCALE: NONE

TENSION LAP SPLICE LENGTHS *(IN INCHES)*

UNCOATED BARS - NORMAL WEIGHT CONCRETE - GRADE 60 BARS

BAR SIZE	LAP CLASS	3,000 psi CONCRETE TOP BARS CASE 1	CASE 2	OTHER BARS CASE 1	CASE 2	4,000 psi CONCRETE TOP BARS CASE 1	CASE 2	OTHER BARS CASE 1	CASE 2
#3	A	22	32	17	25	19	28	15	22
	B	28	42	22	32	24	36	19	28
#4	A	29	43	22	33	25	37	19	29
	B	37	56	29	43	32	48	25	[illegible]
#5	A	36	54	28	41	31	47	24	36
	B	47	70	36	54	40	60	31	47
#6	A	43	64	33	50	37	56	[illegible]	43
	B	56	84	43	64	48	72	37	56
#7	A	63	94	48	72	54	[illegible]	[illegible]	63
	B	81	122	63	94	70	[illegible]	[illegible]	81
#8	A	72	107	55	82	62	93	48	72
	B	93	139	72	[illegible]	[illegible]	[illegible]	62	93
#9	A	81	121	62	[illegible]	[illegible]	105	54	81
	B	[illegible]	[illegible]	[illegible]	[illegible]	[illegible]	136	70	105
#10	A	91	136	70	105	79	118	61	91
	B	118	177	91	136	102	153	79	118
#11	A	[illegible]	151	78	116	87	131	67	101
	B	[illegible]	196	101	151	113	170	87	131

NOTES:
1. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS. CASES 1 AND 2, WHICH DEPEND ON THE TYPE OF STRUCTURAL ELEMENT, CONCRETE COVER, AND THE CENTER TO CENTER SPACING OF THE BARS ARE DEFINED AS FOLLOWS:
2. BEAMS OR COLUMNS:
   CASE 1: COVER AT LEAST 1 BAR DIAMETER AND CTR. - CTR. SPACING AT LEAST 2 BAR DIAMETERS.
   CASE 2: COVER LESS THAN 1 BAR DIAMETER OR CTR. - CTR. SPACING LESS THAN 2 BAR DIAMETERS.
   ALL OTHERS:
   CASE 1: COVER AT LEAST 1 BAR DIAMETER AND CTR. - CTR. SPACING AT LEAST 3 BAR DIAMETERS.
   CASE 2: COVER LESS THAN 1 BAR DIAMETER OR CTR. - CTR. SPACING LESS THAN 3 BAR DIAMETERS.

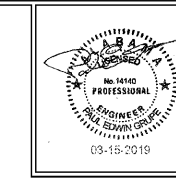
TABLE OF CONCRETE REINFORCEMENT SPLICE LENGTHS (8 / S0.2)
SCALE: NONE

TENSION LAP SPLICE IN MASONRY (IN INCHES)

f'm	#3	#4	#5	#6	#7	#8	#9
1500 psi	18	24	30	36	42	48	54

NOTES:
1. IN ACCORDANCE WITH 2107.2 (2015 IBC).
2. FOR f_y = 60,000 psi UNCOATED.
3. MINIMUM MASONRY COVER = 3 1/2".

TABLE OF MASONRY REINFORCEMENT LAP SPLICE (9 / S0.2)
SCALE: NONE



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S0.2