GENERAL NOTES:

1. THESE NOTES SHALL APPLY UNLESS OTHERWISE INDICATED BY THE DRAWINGS OR SPECIFICATIONS.
2. REFER TO ARCH DRAWINGS FOR DIMENSIONS NOT SHOWN.
3. STRUCTURAL DRAWINGS INDICATE TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY. SHOP DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH SPECIFIED STANDARDS AND THE SPECIFIC REQUIREMENTS OF THIS PROJECT AS INDICATED ON THE DRAWINGS.
4. CONTRACTOR SHALL PROVIDE ADEQUATE BRACING OR SHORING FOR ALL WORK DURING THE CONSTRUCTION PERIOD.
5. THESE STRUCTURAL DRAWINGS HAVE BEEN PREPARED IN ACCORDANCE WITH THE 2012 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC)

WIND DESIGN CRITERIA.
WIND SPEED - 115 MPH (ULTIMATE LOADING)
IMPORTANCE FACTOR Iw = 1.0
WIND ANALYSIS BASED ON SIMPLIFIED METHOD
BASIC WIND PRESSURE 17.1 PSF

SEISMIC DESIGN CRITERIA:
SEISMIC USE GROUP: SUG = I
SEISMIC SITE CLASS: C (PER GEOTECHNICAL REPORT)
SEISMIC DESIGN CATEGORY: SDC = C
SEISMIC RESPONSE COEFFICIENTS Sd1 = 0.138 Sds = 0.138
SEISMIC IMPORTANCE FACTOR Ie = 1.0
SEISMIC RESPONSE MODIFICATION FACTOR R = 3
SEISMIC ANALYSIS BASED ON ELF METHOD.

GRAVITY DESIGN LOADS
ROOF DESIGN LIVE LOAD - ROOF 20 PSF (300# CONCENTRATED)

ROOF DESIGN DEAD LOAD - 20 PSF

6. LATERIAL LOAD RESISTING SYSTEM - ORDINARY REINFORCE MASONRY SHEAR WALLS (SEE PLANS FOR LOCATIONS)
7. DESIGN REFERENCE STANDARDS: AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC), THE AMERICAN CONCRETE INSTITUTE (ACI 318-99), NATIONAL CONCRETE MASONRY ASSOCIATION (NCMA), AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM), THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI), THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC), AND THE AMERICAN PLYWOOD ASSOCIATION (APA).

FOUNDATION NOTES:

1. THE FOUNDATION DESIGN IS BASED ON STANDARD 2000 PSF ALLOWABLE LOADING. CONTRACTOR SHALL VERIFY BEFORE CONSTRUCTION
2. DROP FOOTINGS WHERE REQUIRED TO AVOID INTERFERENCE WITH PLUMBING AND/OR OTHER UTILITIES.
3. BOTTOM OF ALL FOOTING SHALL BE 12" BELOW GRADE.

CONCRETE NOTES:

1. THE DESIGN OF CONCRETE STRUCTURAL ELEMENTS INCLUDING WALLS, SLABS, AND FOOTINGS IS IN ACCORDANCE WITH ACI 318.
2. ALL CONCRETE SHALL BE NORMAL WEIGHT (150 PCF) 3000 PSI COMPRESSIVE STRENGTH AT 28 DAYS. THE RESULTS FOR ALL CONCRETE TESTS SHALL BE AVAILABLE ON THE JOB SITE FOR REVIEW BY THE INSPECTOR.
3. CONSTRUCTION OF CONTROL JOINTS SHALL BE PROVIDED IN SLABS ON GRADE SO THAT THE MAXIMUM AREA OF SLAB BETWEEN JOINTS SHALL BE 900 SQUARE FEET. THE LENGTH BETWEEN C.J. SHALL NOT BE MORE THAN 2 TIMES THE WIDTH BETWEEN C.J.
4. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-82 AND A-185. PLACE WWF 1" BELOW TOP OF SLAB.
5. REINFORCING BARS SHALL CONFORM WITH ASTM A 615. ALL BARS SHALL BE GRADE 60. MINIMUM CONCRETE COVER SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE ON THE DRAWINGS.

CONCRETE CAST AGAINST EARTH - 3"
FORM CAST CONCRETE WITH EARTH CONTACT - 2"
MASONRY - CENTER BARS IN CELLS UNLESS NOTED OR SHOWN OTHERWISE

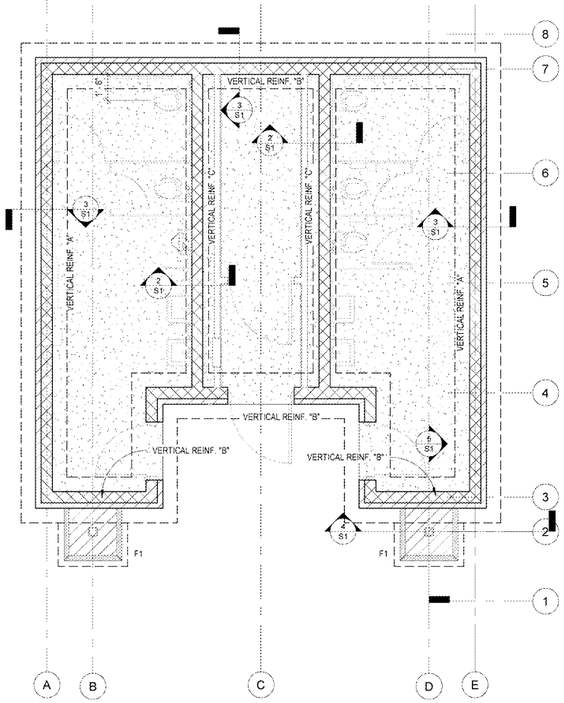
6. FOR ALL CONCRETE - REINFORCING LAP SPLICES SHALL BE 22" FOR #4, 34" FOR #5, 46" FOR #6, AND 74" FOR #7 UNLESS NOTED OTHERWISE ON THE PLANS. PROVIDE CORNER BARS AT ALL CORNERS AND INTERSECTIONS.

LOAD BEARING MASONRY NOTES:

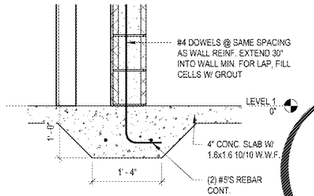
1. HOLLOW LOAD BEARING MASONRY UNITS SHALL CONFORM TO ASTM C90-01.a, LIGHTWEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH f'm= 1500 PSI ON THE NET BLOCK AREA. ALL CMU UNITS SHALL BE OF THE SIZE INDICATED ON THE DRAWINGS AND SHALL BE LAID IN RUNNING BOND.
2. MORTAR SHALL CONFORM TO ASTM C270 CEMENT-LIME, TYPE M OR S. ALL MASONRY GROUT SHALL CONFORM TO ASTM C 478 (3000 PSI COMPRESSIVE STRENGTH).
3. HORIZONTAL WALL REINFORCEMENT SHALL BE #9 (W1.7) TRUSS TYPE WIRE REINFORCING AT 16"o.c. LAP 16" MINIMUM.
4. VERTICAL WALL REINFORCING SHALL BE AS INDICATED ON THE DRAWINGS BUT NOT LESS THEN 1 #4 AT EACH CORNER, WALL END, EA SIDE OF EACH OPENING, AND AT 48"o.c. LAP VERTICAL BARS AS INDICATED ON THE DETAILS. FILL ALL REINFORCED CELLS WITH GROUT.

STRUCTURAL STEEL NOTES:

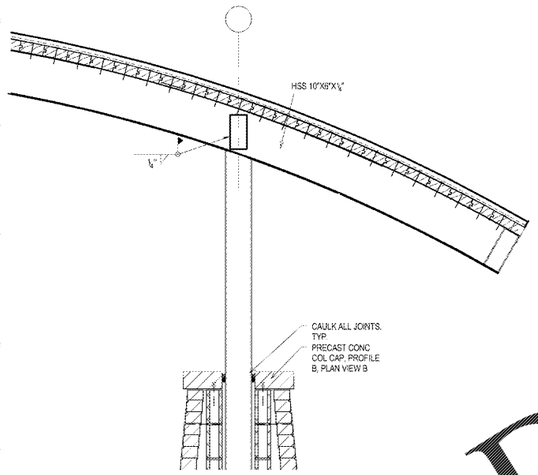
1. STRUCTURAL STEEL SHALL CONFORM TO ASTM A992. UNLESS OTHERWISE NOTED ON DRAWINGS. STRUCTURAL TUBES SHALL CONFORM TO ASTM A500 GRADE B. MISCELLANEOUS STEEL SHAPES, PLATES, ETC. SHALL HAVE A MINIMUM YIELD STRENGTH OF 36 KSI.
2. STRUCTURAL STEEL DETAILING, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE "MANUAL OF STEEL CONSTRUCTION" OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION. SHOP DRAWINGS SHALL SHOW COMPLETE WELDING INFORMATION, BOTH SHOP AND FIELD, USING AWS SYMBOLS UNLESS OTHERWISE INDICATED OR SHOWN. BOLTED CONNECTIONS SHALL BE MADE USING 3/4" DIAMETER BOLTS CONFORMING TO ASTM A325 UNLESS OTHERWISE NOTED.
3. ALL BOLTS SHALL BE INSTALLED PER THE SPECIFICATIONS FOR STRUCTURAL JOINTS. SNUG TIGHT, AND INSPECTED IN ACCORDANCE WITH RCSC-2000. COPIES OF INSPECTION REPORTS SHALL BE AVAILABLE ON THE JOB SITE FOR REVIEW BY THE INSPECTOR.
4. ALL WELDING SHALL BE PREFORMED IN ACCORDANCE WITH AWS D1.1 STRUCTURAL WELDING CODE. A COPY OF EACH WELDERS CERTIFICATION SHALL BE AVAILABLE ON THE JOB SITE AT ALL TIMES.

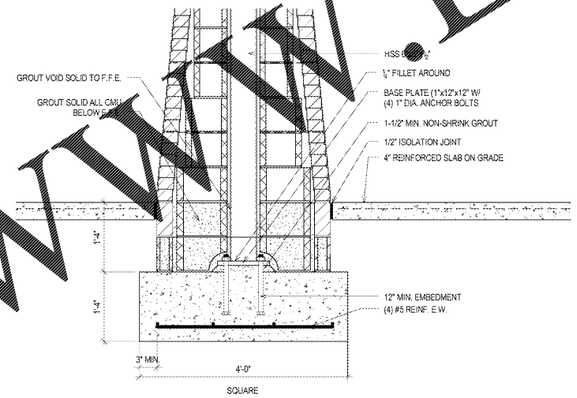


1/S1 FOUNDATION PLAN
1/4" = 1'-0"

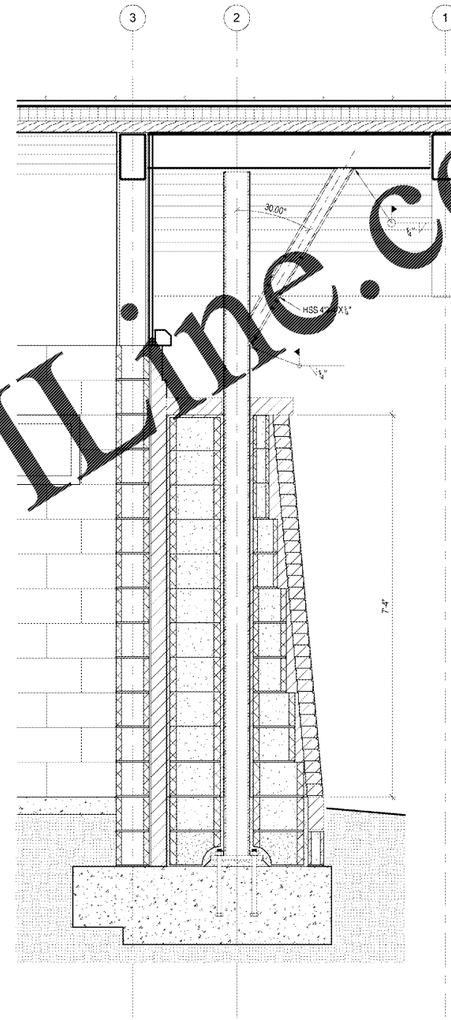


2/S1 TYPICAL GRADE BEAM
3/4" = 1'-0"

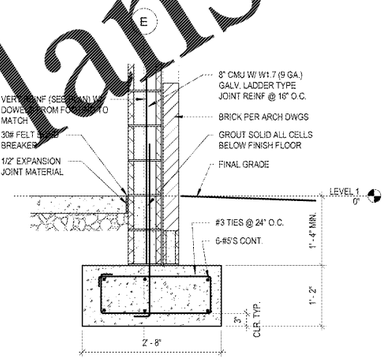




4/S1 TYP COLUMN CROSS SECTION
3/4" = 1'-0"



5/S1 TYP COLUMN TRAVERSE SECTION
3/4" = 1'-0"



3/S1 STRUCTURAL FOOTING SECTION
3/4" = 1'-0"

FOOTING SCHEDULE		
MARK	SIZE	REINFORCING
F1	48"x48"x16"	(4) #5'S EACH WAY

VERTICAL WALL REINFORCING SCHEDULE	
MARK	REINFORCING SIZE , NOTES
A	#7 @ 16" MAX - TYPICAL AT STUB COLUMN WALLS
B	#6 @ 16" MAX - TYPICAL AT EXTERIOR WALLS
C	#4 @ 48" MAX - TYPICAL INTERIOR WALL REINFORCING

1. ALL VERTICAL BARS SHALL BE HELD IN PLACE WITH WIRE POSITIONERS.
2. REINFORCING BARS SHALL NOT BE "WET SET".
3. POSITION OF VERTICAL BARS TAKES PRIORITY OVER HORIZONTAL REINFORCING.
4. SPACING ALONG LENGTH OF WALL IS NOMINAL AND MAY BE SLIGHTLY ADJUSTED TO POSITION THE BARS IN CENTER OF CELLS.
5. VERTICAL REINFORCING SHALL BE PLACED IN CELLS IMMEDIATELY ADJACENT TO CELLS WHICH STUB COLUMN BASE PLATE ANCHORAGE IS PLACED. NO VERTICAL REINFORCING SHALL BE PLACED IN CELL DIRECTLY BELOW BASE PLATE.

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NO.	DATE	DESCRIPTIONS	BY

ISSUES / REVISIONS

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S1