STRUCTURAL NOTES
AMENITY II

GENERAL NOTES:

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO INSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.

DESIGN LOADS:

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE, 2017 (FBC 2017, 6TH EDITION). THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:

ROOF:

LIVE LOAD - 20 psf.
DEAD LOAD - 35 psf.

WIND:

ASCE 7-10
150 MPH REGION (Vult), 116.19 MPH (Vasd)
EXPOSURE D
ENCLOSED STRUCTURE
IMPORTANCE FACTOR = 1.00
INTERNAL PRESSURE COEFF. GCpi = +/- 0.18

SHOP DRAWING REVIEW:

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW NOTATION WILL BE RETURNED UNCHECKED.

ONE SET OF PRINTS WILL BE RETAINED BY THE ENGINEER AND ONE BY THE ARCHITECT, THE CONTRACTOR SHALL RECEIVE THE REMAINING PRINTS FOR SUBMITTAL TO THE BUILDING DEPARTMENT AND AS REQUIRED FOR DISTRIBUTION.

IN ALL INSTANCES THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN A REQUEST FOR INFORMATION (RFI) OR SIMILAR DOCUMENTATION BY THE ENGINEER.

SHOP DRAWINGS SHOULD BE SUBMITTED FOR ALL COMPONENTS OF THE STRUCTURAL FRAMING SYSTEM, AS REQUIRED BY THE ARCHITECT, AND AS NOTED ELSEWHERE IN THESE NOTES, INCLUDING, BUT NOT LIMITED TO:
CONCRETE MIX DESIGNS
MASONRY BLOCK
MASONRY BLOCK ACCESSORIES
MASONRY REINFORCING
CONCRETE REINFORCEMENT
PRE-ENGINEERED WOOD TRUSSES
PRE-ENGINEERED ALUMINUM FRAMING
ANY ALTERNATE MATERIAL/PRODUCT SUBSTITUTIONS

FOUNDATIONS:

SEE THE FOLLOWING REPORT FOR COMPLETE GEOTECHNICAL RECOMMENDATIONS AND INSTALLATION PROCEDURES. SITE PREPARATION AND FOUNDATION INSTALLATION SHALL COMPLY WITH
REPORT No.: DES 147479
DATED: SEPTEMBER 26, 2014
PREPARED BY: DRIGGERS ENGINEERING SERVICES INCORPORATED
TITLED: REPORT OF GEOTECHNICAL INVESTIGATION, CONDOMINIUM UNIT 1, SUNSET POINTE AT COLLANY KEY, TIERRA VERDE, FLORIDA

FOUNDATION DESIGN IS BASED ON A SOIL BEARING PRESSURE OF 2,000 psf.

FORMWORK AND SHORING (CONCRETE SLABS AND BEAMS):

NO STRUCTURAL CONCRETE SHALL BE STRIPPED UNTIL IT HAS REACHED AT LEAST TWO-THIRDS OF THE 28 DAY DESIGN STRENGTH. A MINIMUM OF 3 STORIES OF SHORING AND (OR) RESHORING SHALL BE USED WHICH SHALL CONSIST OF ONE COMPLETE SET OF VERTICAL SHORES AND TWO SETS OF VERTICAL SHORES THAT COMPRISE AT LEAST 50% OF A COMPLETE SET. DESIGN, ERECTION AND REMOVAL OF ALL FORMWORK, SHORES AND RESHORES SHALL MEET REQUIREMENTS SET FORTH IN ACI STANDARDS 347 AND 301. DRAWINGS FOR SHORING AND RESHORING SHALL BE PREPARED BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

PLUMBING SLEEVES:

MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE ENGINEER.

REINFORCING STEEL:

SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.

WELDED WIRE FABRIC:

TO CONFORM TO ASTM A-185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES.

CONCRETE:

SHALL BE PER AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

3000 PSI FOR FOUNDATIONS AND SLABS ON GRADE.
4000 PSI FOR ALL OTHER STRUCTURAL CONCRETE.

CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ALL STANDARDS AND SPECIFICATIONS.

SUBMIT PROPOSED MIX DESIGN WITH RECENT FIELD CYLINDER OR LAB TESTS FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE. FOR ALL FLATWORK, AT LEAST 75% OF LARGE AGGREGATE SHALL CONSIST OF #57 STONE. CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED.

THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. ALL SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

ALL CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE.

ALL CONCRETE DESIGN MIX SUBMITTALS SHALL INCLUDE TESTED, STATISTICAL BACK-UP DATA AS PER CHAPTER 5 OF ACI 301.

WATER/CEMENT RATIO FOR CONCRETE AT EXTERIOR BALCONIES SHALL NOT EXCEED 0.40 BY WEIGHT AND HAVE 5000 PSI MINIMUM COMPRESSIVE CAPACITY.

UNLESS NOTED OTHERWISE ON PLANS, THE FOLLOWING CONCRETE CLEAR COVER SHALL BE PROVIDED FOR ALL NON-PRESTRESSED CONCRETE REINFORCEMENT PER ACI 318:

CONCRETE CAST AGAINST EARTH:	ALL BARS	-	3"
CONCRETE EXPOSED TO EARTH (FORMED FACE):	ALL BARS	-	2"
CONCRETE EXPOSED TO WEATHER:	#6 BARS AND GREATER	-	2"
	#5 BARS AND SMALLER	-	1 1/2"
WHERE NOT EXPOSED TO EARTH OR WEATHER:			
SLABS, WALLS, AND JOISTS:	#14 & #18 BARS	-	1 1/2"
	#11 BARS AND SMALLER	-	3/4"
BEAMS AND COLUMNS:	ALL BARS		

SEE ACI 318 FOR ADDITIONAL REQUIREMENTS AND OTHER INFORMATION.

CONCRETE TESTING: AN INDEPENDENT TESTING LABORATORY SHALL PERFORM THE FOLLOWING TESTS ON CAST IN PLACE CONCRETE:

ASTM C143 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." MAXIMUM SLUMP SHALL BE 4-6 INCHES, PRIOR TO ADDING A SUPER PLASTISIZER.
ASTM C39 - "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. REQUIRED CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS:
1 @ 7 DAYS
1 @ 28 DAYS

ONE ADDITIONAL READY CYLINDER MAY BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER MAY BE DISCARDED.

NON-SHRINK GROUT:

NON-SHRINK GROUT SHALL BE A HIGH-STRENGTH MORTAR OR GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS. THE GROUT IS TO BE NON-METALLIC, NON-CORROSIVE, CEMENT-BASED AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM C1107. IT SHALL BOND PERMANENTLY TO A CLEAN METAL BASEPLATE AND CONCRETE SUBSTRATE AND WILL NOT SHRINK IN ITS PLASTIC STATE, AS TESTED IN ACCORDANCE WITH ASTM C827.

CHEMICAL ANCHORS:

SHALL BE AN EQUAL TWO PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS SIMPSON SET-XP "STRUCTURAL ANCHORING ADHESIVE"; HILTI HIT-HY 150 MAX-SD OR ENGINEER APPROVED SUBSTITUTION, INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE. BRUSH AND BLOW OUT ALL HOLES.

MASONRY WALLS:

MASONRY UNITS SHALL MEET ASTM C-90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 1900 PSI ON THE NET AREA (F'M = 1500 PSI). MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C-270. GROUT SHALL BE 3000 PSI MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C-476. PROVIDE HOOKED DOWELS IN FOOTINGS FOR ALL VERTICAL REINFORCING ABOVE. LAP SPLICES 48 BAR DIAMETERS.

BLOCK CELLS AS SHOWN ON PLANS SHALL BE GROUT FILLED WITH VERTICAL REINFORCING BARS. SEE PLAN NOTES FOR BAR SIZE AND SPACING. DOWELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW, UNLESS NOTED OTHERWISE. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE AND GROUT TO AREA REQUIRED.

PROVIDE 9 GAGE GALVANIZED HORIZONTAL JOINT REINFORCING (DUR-O-WALL OR ENGINEER APPROVED SUBSTITUTION) AT ALTERNATE BLOCK COURSES, BEGINNING 8" ABOVE FOOTINGS AND FLOOR LEVELS.

GROUT LIFT: AN INCREMENT OF GROUT HEIGHT WITHIN A TOTAL GROUT POUR.
GROUT POUR: THE TOTAL HEIGHT OF MASONRY TO BE GROUTED PRIOR TO ERECTION OF ADDITIONAL MASONRY. A GROUT POUR CONSISTS OF ONE OR MORE GROUT LIFTS. GROUT POURS SHALL SET FOR A MINIMUM OF 4 HOURS BEFORE ANY ADDITIONAL GROUT PLACEMENT.

GROUT SHALL HAVE A SLUMP BETWEEN 8 AND 11 INCHES, EXCEPT SELF-CONSOLIDATING GROUT. JOB-SITE PROPORTIONING OF SELF-CONSOLIDATING GROUT IS NOT PERMITTED.

MASONRY GROUTING REQUIREMENTS:
FIELD-MIXED GROUT SHALL BE PLACED WITHIN 1-1/2 HOURS FROM INTRODUCING WATER INTO THE MIXTURE AND BEFORE INITIAL SET.
GROUT SLUMP REQUIREMENTS:
FOR GROUT SLUMP BETWEEN 8 AND 10 INCHES, THE MAXIMUM GROUT LIFT HEIGHT IS 5 FEET.
FOR GROUT SLUMP BETWEEN 10 AND 11 INCHES, THE MAXIMUM GROUT LIFT HEIGHT IS 12.67 FEET.
FOR SELF-CONSOLIDATING GROUT, THE GROUT LIFT HEIGHT SHALL NOT EXCEED THE GROUT POUR HEIGHT (24 FEET MAX.).
GROUT LIFT HEIGHTS EXCEEDING 5 FEET SHALL MEET THE FOLLOWING REQUIREMENTS:
MASONRY MORTAR HAS CURED FOR AT LEAST 4 HOURS.
GROUT SLUMP IS BETWEEN 10 AND 11 INCHES.
NO INTERMEDIATE BOND BEAMS ARE PLACED BETWEEN THE TOP AND BOTTOM OF THE GROUT LIFT HEIGHT.
EACH GROUT LIFT SHALL BE CONSOLIDATED BY MECHANICAL VIBRATION AT THE TIME OF PLACEMENT. CONSOLIDATION IS NOT REQUIRED FOR SELF-CONSOLIDATING GROUT.
EACH GROUT LIFT SHALL BE RECONSOLIDATED BY MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED, AND BEFORE ADDING THE SUBSEQUENT GROUT LIFT. RECONSOLIDATION IS NOT REQUIRED FOR SELF-CONSOLIDATING GROUT.
THE TIME BETWEEN PLACING GROUT LIFTS SHALL NOT EXCEED 1 HOUR.
THE MAXIMUM POUR HEIGHT IS 24 FEET.
A GROUT KEY SHALL BE PROVIDED AT THE TOP OF EACH GROUT LIFT AND GROUT POUR. GROUT KEYS SHOULD BE FORMED BY TERMINATING THE GROUT 1-1/2 INCHES BELOW A MORTAR JOINT.

TIE BEAMS:

BEAMS WITH THE PREFIX "TB" SHALL BE OF CONCRETE POURED AFTER THE BLOCK WALLS BELOW ARE IN PLACE. REINFORCING SHALL BE CONTINUOUS THROUGH TIE BEAMS WITH MINIMUM LAP SPLICES OF 48 BAR DIAMETERS AND BENT BARS AT CORNERS. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE TO AREA REQUIRED, IN ACCORDANCE WITH ACI 530.1, SECTION 4.3.3.3 (SOLID METAL OR FELT CAVITY CAPS ARE PROHIBITED).

LINTELS:

MASONRY OPENINGS LESS THAN 6 FEET SHALL BE SPANNED WITH AN 8" SPAN RATED PRECAST/PRESTRESSED CONCRETE LINTEL. ALL PRECAST LINTELS SHALL BEAR A MINIMUM OF 8" AT EACH END ON A GROUT FILLED CELL.

MASONRY OPENINGS 6 FEET OR GREATER SHALL BE SPANNED WITH AN 8" SPAN RATED PRECAST/PRESTRESSED CONCRETE LINTEL WITH 1#5 BAR CONTINUOUS. PRECAST LINTEL AND ALL CELLS ABOVE, TO THE BOTTOM OF THE TIE BEAM OR BOND BEAM, SHALL BE GROUTED SOLID. ALL PRECAST LINTELS SHALL BEAR A MINIMUM OF 8" AT EACH END ON A GROUT FILLED CELL.

WHERE A CONCRETE COLUMN OR CONCRETE TIE COLUMN IS WITHIN 8" OF A MASONRY OPENING, THE LINTEL SHALL BE AN 8"X16" CONCRETE CAST-IN-PLACE BEAM WITH (2) #5 BARS TOP AND BOTTOM, AND #3 STIRRUPS AT 18" ON CENTER.

ALUMINUM:

SHALL CONFORM TO ASTM B308, ALUMINUM ALLOY 6061-T6, UNLESS NOTED OTHERWISE ON DRAWINGS. OTHER ALLOYS SHALL CONFORM TO ASTM B221 AND ASTM B429. ALL ALUMINUM SECTIONS SHALL BE FABRICATED AND DESIGNED PER THE ALUMINUM DESIGN MANUAL, 2005, SPECIFICATIONS AND GUIDELINES FOR ALUMINUM STRUCTURES, BY THE ALUMINUM ASSOCIATION. ALLOWABLE STRESSES SHALL BE REDUCED WITHIN ALL WELD-AFFECTED ZONES; THE ALUMINUM DESIGN MANUAL SECTION 7 DEFINES THE WELD-AFFECTED ZONE AS 1" TO EACH SIDE OF THE CENTERLINE OF A WELD.

ALL WELDING OF MEMBERS SHALL CONFORM TO THE AMERICAN WELDING SOCIETY D1.2 - STRUCTURAL WELDING CODE, ALUMINUM. UNLESS NOTED OTHERWISE, ALL SHOP CONNECTIONS SHALL BE WELDED WITH 5183 FILLER STRENGTH AND ALL FIELD CONNECTIONS SHALL BE BOLTED. NO WELDING OF MEMBERS IS PERMITTED EXCEPT THOSE SHOWN ON DRAWINGS. ALL PAINT SHALL CONFORM TO U.S. FEDERAL SPECIFICATION TT-P-645 - PAINT, FOR ALUMINUM AND HEAT-RESISTING.

ALL BOLTED CONNECTIONS SHALL CONSIST OF MINIMUM ½" DIAMETER BOLTS, ALLOY 6061-T6. CONNECTIONS SHALL NOT HAVE LESS THAN 2 ROWS OF BOLTS. ALL FLAT WASHERS SHALL BE ALCLAD 2024-T4. ALL ALUMINUM MEMBERS SHALL BE SHOP POWDER-COATED. ALL DISSIMILAR METALS IN CONTACT WITH ALUMINUM SHALL BE STAINLESS-STEEL. ALUMINUM SHALL NOT BE EMBEDDED IN CONCRETE EXCEPT IN EPOXY-GROUT, UNLESS SPECIFIED ON DRAWINGS.

WOOD:

STRUCTURAL WOOD COMPONENTS (BEAMS, JOISTS, RAFTERS, ETC.) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE LUMBER STRESSES FOR SOUTHERN PINE CONFORMING TO 2005 NDS, WITH 2013 SUPPLEMENT, AS FOLLOWS:

SHEAR		FV	= 175 PSI.
BENDING	2X6	FB	= 1,000 PSI.
BENDING	2X8	FB	= 925 PSI.
BENDING	2X10	FB	= 800 PSI.
BENDING	2X12	FB	= 750 PSI.

WOOD IN CONTACT WITH CONCRETE OR MASONRY, AND AT OTHER LOCATIONS AS SHOWN ON STRUCTURAL DRAWINGS, SHALL BE PROTECTED OR PRESSURE TREATED IN ACCORDANCE WITH AITC-109. MEMBER SIZES SHOWN ARE NOMINAL UNLESS NOTED OTHERWISE.

ALL NAILS SHOWN ON PLANS ASSUME COMMON WIRE NAILS UNLESS SPECIFICALLY NOTED OTHERWISE.

ENGINEERED WOOD TRUSS SYSTEMS SHALL BE DESIGNED BY SUPPLIER'S SPECIFIC ENGINEERING CONFIGURATION AND LOAD-CARRYING CAPACITY SHOWN ON DRAWINGS AND SPECIFICATIONS. ALL INDIVIDUAL TRUSS MEMBERS, STEEL PLATE CONNECTIONS, TRUSS-TO-TRUSS CONNECTIONS, COMMON TRUSSES AND GIRDER TRUSSES SHALL BE DESIGNED FOR COMPONENT AND CLADDING LOADINGS EXCEPT THOSE TRUSSES EXCEEDING 700 SQUARE FEET IN TRIBUTARY AREA. ALTERNATE TRUSS LAYOUTS, IF ACCEPTABLE TO ARCHITECT/ENGINEER ONLY AS A CHANGE ORDER WHICH WILL INCLUDE ENGINEERING CHARGES FOR REDESIGN OF THE STRUCTURE BY THE ENGINEER OF RECORD. SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTION TYPES PROVIDED WITHIN TRUSSES, AS WELL AS CONNECTORS UTILIZED IN ALL OTHER CONNECTIONS AND ATTACHMENTS BETWEEN TRUSSES OR COMPONENTS SUPPLIED AS PART OF THE ENGINEERED TRUSS SYSTEM. AN ERECTION DRAWING SHALL BE INCLUDED, IDENTIFYING ALL TRUSS SYSTEM COMPONENTS, AS WELL AS ALL PERMANENT BRACING REQUIRED FOR TRUSS DESIGN.

ENGINEERED SHOP DRAWINGS SHALL BE SIGNED AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER AS THE SPECIALTY ENGINEER FOR THE FOLLOWING LOAD DURATION FACTORS SHALL BE USED:

DEAD LOAD	0.9
DEAD LOAD + FLOOR LIVE LOAD	1.00
DEAD LOAD + ROOF LIVE LOAD	1.25
DEAD LOAD + WIND LOAD	1.60

THE SUPERIMPOSED DEAD LOADS SPECIFIED IN THE DESIGN LOADS SECTION ABOVE, INCLUDES THE OVERALL WEIGHT OF THE FIRE SPRINKLER SYSTEM PIPING. THE GENERAL CONTRACTOR SHALL PROVIDE THE TRUSS MANUFACTURER WITH THE LOCATIONS OF THE PIPE SUPPORTS AND THE LOADS FROM ANY HANGER LINES GREATER THAN 2' DIAMETER.

PLYWOOD ROOF, FLOOR, AND WALL SHEATHING ARE DESIGNED AS DIAPHRAGMS AND SHALL COMPLY WITH APPLICABLE PROVISIONS OF CHAPTER 23 OF THE FLORIDA BUILDING CODE AND SHALL BE FASTENED IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF TABLES 2306.2.1 OR 2306.3, UNLESS NOTED OTHERWISE. PLYWOOD SHALL BE INSTALLED WITH THE STRENGTH AXIS OF EACH PANEL PERPENDICULAR TO THE SUPPORTS IN ALL CASES. PLYWOOD ROOF PANELS SHALL BE INSTALLED AS SHOWN IN CASES 1 THROUGH 4 IN TABLE 2306.2.1 (CONT.).

WOOD FRAMING CONNECTORS:

ALL CONNECTORS SHALL BE GALVANIZED. CONNECTOR MODEL NUMBERS SHOWN ARE STRONG-TIE CONNECTORS AS MANUFACTURED BY SIMPSON STRONG-TIE CO., 5956 W. LAS POSITAS BLVD., P.O. BOX 10789, PLEASANTON, CA 94588, 800-999-5099, WWW.STRONGTIE.COM. SUBSTITUTIONS ARE ACCEPTABLE WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. UNLESS SHOWN OTHERWISE, INSTALL SIZE AND "MAXIMUM" NUMBER OF FASTENERS SHOWN IN LATEST SIMPSON CATALOG.

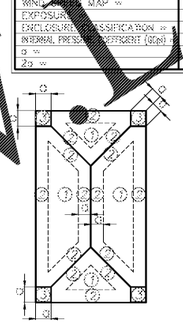
WIND LOAD SCHEDULE

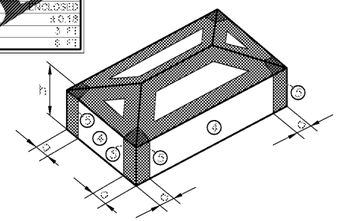
SCHEDULE OF COMPONENTS AND CLADDING LOADS

ZONE	ZONE DESCRIPTION	TRIBUTARY AREA (SF)	IN (PRESSURE) (+ PSF)	OUT (PRESSURE) (- PSF)
1	ROOF INTERIOR ZONE	LESS THAN 20	20.6	32.7
		20 - 100	19.1	31.2
		MORE THAN 100	14.5	26.7
2	ROOF, EDGE ZONE	LESS THAN 20	20.6	56.8
		20 - 100	19.1	52.3
		MORE THAN 100	14.5	41.8
	ROOF, OVERHANG, EDGE ZONE	LESS THAN 20		72.0
		20 - 100		72.0
		MORE THAN 100		72.0
3	ROOF, CORNER ZONE	LESS THAN 20	20.6	84.1
		20 - 100	19.1	78.0
		MORE THAN 100	14.5	62.1
	ROOF, OVERHANG CORNER ZONE	LESS THAN 20		105.1
		20 - 100		105.1
		MORE THAN 100		81.1
4	WALL, INTERIOR ZONE	LESS THAN 20	35.7	38.7
		20 - 100	34.2	37.2
		MORE THAN 100	30.3	33.2
5	WALL, EDGE ZONE	LESS THAN 20	35.7	46.8
		20 - 100	34.2	44.8
		MORE THAN 100	30.3	37.2

NOTE: WIND PRESSURES SHOWN ARE BASED ON Vasd

CODE =	ASCE 7-10
ULTIMATE WIND SPEED Vult =	150
ALLOWABLE WIND SPEED Vasd =	116
RISK CATEGORY =	II
WIND SPEED MAP =	FIGURE 26.5-1A
EXPOSURE =	D
ENCLOSURE CLASSIFICATION =	ENCLOSED
INTERNAL PRESSURE COEFFICIENT (GCpi) =	+/- 0.18
a =	5.4 ft
2a =	8.1 ft



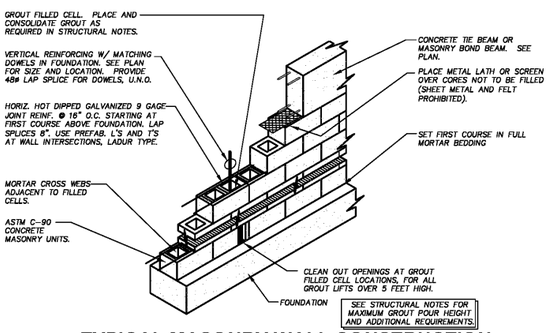


HIP ROOF (7 ° < Θ ≤ 27 °)
SCALE: N.T.S.

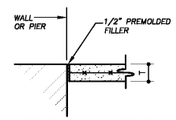
Interior Zones
ZONE 1 – ROOF
ZONE 4 – WALL

End Zones
ZONE 2 – ROOF
ZONE 5 – WALL

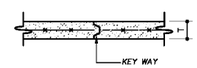
Corner Zones
ZONE 3 – ROOF



TYPICAL MASONRY WALL CONSTRUCTION
SCALE: NTS



ISOLATION JOINT (IJ)
SCALE: NTS



CONSTRUCTION JOINT (KJ)
SCALE: NTS

DEVELOPED BY:
COLONY ISLAND, LLC
333 3RD AVENUE NORTH
SUITE 400
ST. PETERSBURG, FL 33701

SUNSET POINTE AMENITY BUILDING
AT COLLANY KEY

To the best of the Architect's knowledge, the plans and specifications comply with the applicable minimum building code requirements and with the fire-safety standards as determined by the local authority in accordance with Fbc 110.3.7.4.4 and Chapter 633, Florida Statutes

CURTIS GAINES HALL JONES ARCHITECTS
1213 E. 6TH AVE
YBOR CITY
TAMPA FLORIDA 33605
PHONE 813 228-8000
FAX 813 228-0770
FL CERT. #AAC001590

JOB NO. 1311B
DATE JULY. 16, 2018
DRAWN BY
REV. 1 12-12-2018
REV. 2
REV. 3
REV. 4
REV. 5
REV. 6
REV. 7
REV. 8
REV. 9
REV. 10
REV. 11
REV. 12

PERMIT DOCUMENTS

2/11/2019

ENGINEER OF RECORD
ANTHONY R. WILSON
FL P.E. # 79451

STRUCTURAL NOTES & TYPICAL DETAILS

S-1

Wilson Structural Consultants, Inc.
8131 PROFESSIONAL PKWY WEST
SARASOTA, FLORIDA 34240
Cert. of Authorization #9099