

GENERAL NOTES

COLD-FORMED STEEL NOTES

- ALL EXTERIOR METAL STUDS ARE TO BE C.S.J. (U.N.O.)
- ALL EXTERIOR METAL STUDS SHALL BE DESIGNED FOR LATERAL WIND PRESSURE BASED UPON ULTIMATE DESIGN WIND SPEED AS SHOWN ON DESIGN NOTES AND SHOULD BE LIMITED TO LATERAL DEFLECTION OF L/600 WHEN BACKING MASONRY, AND L/360 WHEN BACKING E.I.F.S., AND METAL PANELS
- ALL EXTERIOR FLANGES OF STUD WALLS SHALL BE BRACED BY SHEATHING PROPERLY ATTACHED TO THEM.
- CONTRACTOR SHALL FURNISH DETAILED DESIGN CALCULATIONS INCLUDING CONNECTION AND GAUGE OF ALL EXTERIOR METAL STUDS, METAL STUD TRUSSES AND METAL STUD TRUSS GIRDERS SEALED BY A REGISTERED ENGINEER IN THE STATE IN WHICH THE PROJECT IS BUILT.
- THE TOP COMPRESSION FLANGES OF JOISTS & TRUSSES ARE TO BE LATERALLY BRACED BY PROPER ATTACHMENT OF THE DECKING.
- THE TOP FLANGES OF BOTTOM CHORDS OF TRUSSES, SHALL BE BRACED LATERALLY AT INTERVALS NOT EXCEEDING 3'-0" o.c. BY CONTINUOUS C STUDS WELDED TO TOP FLANGES OR BY SHEATHING ATTACHED DIRECTLY TO BOTTOM FLANGE.
- TOP FLANGE OF CEILING STUD SHALL BE BRACED LATERALLY BY CONTINUOUS C STUDS WELDED TO EACH JOIST. EACH BRACE SHALL BE CAPABLE OF TRANSMITTING A MINIMUM OF 200 POUNDS IN TENSION OR COMPRESSION. BOTTOM FLANGE OF CEILING SHALL BE BRACED BY SHEATHING PROPERLY ATTACHED TO BOTTOM FLANGE.
- ALL INTERIOR FLANGES OF STUD WALLS SHALL BE BRACED EITHER BY SHEATHING OR BY CONTINUOUS CHANNELS WELDED TO FLANGE AT INTERVALS NOT EXCEEDING 2'-0" o.c. OR AS DETERMINED BY DETAIL DESIGN DOCUMENTS FURNISHED BY A REGISTERED ENGINEER IN THE STATE IN WHICH THE PROJECT IS BUILT.
- ALL NON-LOAD-BEARING METAL STUD WALLS (INTERIOR AND EXTERIOR) SHALL BE CONNECTED TO ALLOW FOR 3/4" VERTICAL MOVEMENT BETWEEN STUD WALL AND STRUCTURE.
- ALL OTHER INTERIOR METAL STUD PARTITIONS, BULKHEADS, CEILING STUDS AND ECT. SHALL BE FURNISHED UNDER THE DIRECTION OF THE ARCHITECTURAL DOCUMENTS. ANY ADDITIONAL STUD DESIGN REQUIREMENTS SHALL BE PROVIDED BY THE GENERAL CONTRACTOR'S METAL STUD DESIGN ENGINEER.
- METAL STUD WALLS SHALL NOT BE ERECTED UNTIL AFTER DEAD LOADS AND ALL CONCRETE FLOORS ON FRAMING ABOVE ARE IN PLACE.

POST-INSTALLED ANCHOR NOTES

- THE BELOW PRODUCTS ARE THE DESIGN BASIS FOR THIS PROJECT. PRODUCT DIAMETER, EMBEDMENT SHALL BE AS SHOWN IN THE DETAILS. INSTALL PRODUCTS IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII). SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED BELOW MAY BE SUBMITTED TO THE CONTRACTOR TO THE ENGINEER-OF-RECORD (EOR) FOR REVIEW. SUBSTITUTIONS WILL ONLY BE CONSIDERED FOR PRODUCTS HAVING A CODE REPORT RECOGNIZING THE PRODUCT FOR THE APPROPRIATE APPLICATION. SUBSTITUTION REQUESTS SHALL INCLUDE CALCULATION THAT DEMONSTRATE THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE EQUIVALENT PERFORMANCE VALUES OF THE DESIGN BASIS PRODUCT. CONTRACTOR SHALL CONTACT MANUFACTURER'S REPRESENTATIVE (800-537-5098) FOR PRODUCT INSTALLATION TRAINING AND A LETTER SHALL BE SUBMITTED TO THE ENGINEERING FIRM THAT HAS TAKEN PLACE. SPECIAL INSPECTIONS ARE REQUIRED PER THE IBC AND ICC-ES REPORTS.
- FOR ANCHORING INTO CONCRETE:
 - MECHANICAL ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 308.4 AND ICC-ES AC109 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS. PRE-APPROVED ANCHORS INCLUDE:
 - SIMPSON STRONG TIE "STRONG-BOLT 2" (APMO-UES ESR-2138)
 - ADHESIVE FOR REBAR AND ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 308.4 AND ICC-ES AC308 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS. ADHESIVE ANCHORS SHALL BE INSTALLED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER PER ACI 308.11 D 9.2.2 WHERE INDICATED ON THE CONTRACT DOCUMENTS. INSTALLATIONS REQUIRING CERTIFIED INSTALLERS SHALL BE INDICATED PER ACI 308.11 D 9.2.4. PRE-APPROVED ANCHORS INCLUDE:
 - SIMPSON STRONG TIE "SET-XP" (ICC-ES ESR-2508)
 - SIMPSON STRONG TIE "AT-XP" (APMO-UES ESR-2653)
 - POWER-ACTUATED FASTENERS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC70. PRE-APPROVED ANCHORS INCLUDE:
 - SIMPSON STRONG TIE "GAS ACTUATED PINS" (ICC-ES ESR-2811)
 - SIMPSON STRONG TIE "POWDER ACTUATED PINS" (ICC-ES ESR-2138)
 - FOR ANCHORING INTO MASONRY:
 - ANCHORAGE TO SOLID-GROUTED CONCRETE MASONRY
 - MECHANICAL ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC01 OR ICC-ES AC108. PRE-APPROVED ANCHORS INCLUDE:
 - SIMPSON STRONG TIE "TITEN-HD" (ICC-ES ESR-1056)
 - SIMPSON STRONG TIE "STRONG-BOLT 2" (APMO-UES ER-240)
 - ADHESIVE FOR REBAR AND ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC08. PRE-APPROVED ANCHORS INCLUDE:
 - SIMPSON STRONG TIE "AT-XP" (APMO-UES ER-2811)
 - SIMPSON STRONG TIE "SET-XP" (APMO-UES ER-2655)
 - POWER-ACTUATED FASTENERS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC70. PRE-APPROVED ANCHORS INCLUDE:
 - SIMPSON STRONG TIE "GAS ACTUATED PINS" (ICC-ES ESR-2811)
 - SIMPSON STRONG TIE "POWDER ACTUATED PINS" (ICC-ES ESR-2138)
 - FOR ANCHORING INTO STEEL: POWER-ACTUATED FASTENERS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC70. PRE-APPROVED ANCHORS INCLUDE:
 - SIMPSON STRONG TIE "GAS ACTUATED PINS" (ICC-ES ESR-2811)
 - SIMPSON STRONG TIE "POWDER ACTUATED PINS" (ICC-ES ESR-2138)

SLAB-ON-GRADE NOTES

- WHERE SLABS REST ON FILL, FILL SHALL BE COMPACTED IN ACCORDANCE WITH THE SPECIFICATIONS.
- SLABS ON GROUND MAY BE PLACED IN LANE FASHION USING GALVANIZED STEEL, PREFORMED KEVED FORMS AT FLOOR JOINT LOCATION INDICATED. REINFORCING SHALL NOT CROSS CONSTRUCTION OR KEVED JOINTS.
- SEE CIVIL AND ARCH. DRAWINGS AND SPECIFICATIONS FOR EXTERIOR SLAB WORK AND JOINING HORIZONTAL RUNS OF CONDUIT AND PIPE SHALL NOT BE PLACED IN SLABS ON GROUND, PLACE IN SUB-GRADE.
- AT ALL TRUCK DOCK SLABS OR SERVICE COURT SLABS, PROVIDE AN 8" CONCRETE SLAB REINFORCED WITH #4 @ 12" o.c. EACH WAY, TOP AND BOTTOM. PROVIDE KEVED JOINTS AT 15'-0" x 15'-0" PATTERN. TYPICAL, UNLESS NOTED OTHERWISE.

FOUNDATION NOTES

- BUILDING FOOTINGS ARE DESIGNED TO BEAR ON ORIGINAL EARTH OR LABORATORY CONTROLLED SOILS WITH AN ALLOWABLE BEARING CAPACITY OF 3000 PSF. ALLOWABLE BEARING CAPACITY AT CONTINUOUS WALL FOOTINGS SHALL BE 3000 PSF. SOIL BEARING CAPACITY SHALL BE VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER AT THE TIME OF EXCAVATION. ELEVATIONS GIVEN ARE FOR PURPOSES OF CONTRACT AND SHALL BE ADJUSTED AT THE TIME OF EXCAVATION TO MEET SOILS CONDITIONS. SITE SHALL BE PREPARED IN ACCORDANCE TO THE GEOTECHNICAL REPORT PREPARED BY.
- WALL FOOTINGS, UNLESS SHOWN OTHERWISE, SHALL BE 12" THICK AND 6" WIDE EACH SIDE OF MASONRY WALL, REINFORCED WITH 2 #6 CONTINUOUS. (UNLESS NOTED OTHERWISE)
- THE MAXIMUM WALL FOOTING STEP-OFF SHALL BE 2'-0" VERTICAL. SPACED NOT LESS THAN 4'-0" o.c. FOOTING STEP-OFFS ARE INDICATED BY THE SYMBOL.
- WHERE PIPES OR CONDUITS RUN PERPENDICULAR TO A FOOTING, STEP TOP OF FOOTING DOWN TO ALLOW PIPES OR CONDUIT TO RUN OVER TOP OF FOOTINGS.
- WHERE PIPES OR CONDUIT RUN PARALLEL TO FOOTING, STEP DOWN BOTTOM OF FOOTING SO THAT A LINE DRAWN BETWEEN INVERT OF PIPE AND BOTTOM OF FOOTING SHALL NOT EXCEED 30 DEGREES. SEE "TRENCHES NEAR FOOTINGS" DETAIL.
- NO PIPES OR CONDUIT SHALL BE PLACED BETWEEN THE FOOTINGS, OR SLAB ON GRADE.

REPAIR, PROTECT, AND STRENGTHENING NOTES

- THE BELOW PRODUCTS ARE THE DESIGN BASIS FOR THIS PROJECT. CONTRACTOR SHALL FOLLOW MANUFACTURER'S INSTALLATION INSTRUCTIONS AND CONTACT MANUFACTURERS REPRESENTATIVE (800-537-5098) WITH PRODUCT RELATED QUESTIONS. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED BELOW MAY BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER-OF-RECORD (EOR) FOR REVIEW AND APPROVAL. SUBSTITUTIONS WILL ONLY BE CONSIDERED FOR PRODUCTS HAVING INDEPENDENT TEST REPORTS OR OTHER DOCUMENTATION INDICATING THE PRODUCT IS APPROPRIATE FOR THE INTENDED APPLICATION.
- REPAIR PRIMER: REBAR PRIMER SHALL BE USED TO PROTECT EXISTING STEEL REINFORCING AND ESTABLISH POSITIVE BOND FROM EXISTING STEEL REINFORCING TO NEW REPAIR MATERIAL. PRE-APPROVED PRODUCTS INCLUDE:
 - SIMPSON STRONG TIE "FX-406"
 - BONDING AGENTS: BONDING AGENTS SHALL BE USED TO ENCOURAGE POSITIVE BOND OF NEW REPAIR MATERIAL TO EXISTING CONCRETE. PRE-APPROVED PRODUCTS INCLUDE:
 - SIMPSON STRONG TIE "FX-762"
 - REPAIR MORTARS: REPAIR MATERIAL SHALL BE USED TO REPAIR AREAS OF DAMAGED CONCRETE. PRE-APPROVED PRODUCTS INCLUDE:
 - SIMPSON STRONG TIE "FX-263" (FOR USE IN OVERHEAD AND VERTICAL APPLICATIONS)
 - SIMPSON STRONG TIE "FX-261" (FOR USE IN HORIZONTAL AND FORM & POUR APPLICATIONS)
 - CRACK REPAIR SYSTEM: CRACK REPAIR SYSTEM SHALL CONSIST OF CRACK INJECTION MATERIAL AND PASTE OVER ADHESIVE AND SHALL BE USED TO PRESSURE INJECT CRACKS. PRE-APPROVED SYSTEMS INCLUDE:
 - SIMPSON STRONG TIE "FX-751 LV" INJECTION MATERIAL WITH "FX-763" PASTE OVER ADHESIVE
 - NON-SHRINK GROUT MATERIAL: NON-SHRINK GROUT MATERIAL SHALL BE USED TO GROUT BENEATH BASEPLATES, BEARING PLATES AND EQUIPMENT BASES. PRE-APPROVED PRODUCTS INCLUDE:
 - SIMPSON STRONG TIE "FX-228"

STEEL STAIR AND ALL RAILING NOTES

- SUBMIT COMPLETE ERECTION, FABRICATION DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE IN WHICH THIS PROJECT IS BUILT WHICH ADDRESSES THE DESIGN FOR ALL STEEL STAIRS, GUARD RAILS AND HANDRAILS. DETAILS SHALL BE FURNISHED OF ALL STEEL STAIRS. STAIRS SHALL MEET THE SPECIFICATIONS FOR ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS) OF THE AISC CODE OF STANDARD PRACTICE. ALLOW TEN (10) BUSINESS DAYS FOR REVIEW OF SUBMITTED STRUCTURAL DETAILS.
- TREADS AND RISERS SHALL BE PERFORMED OF 14 GAUGE METAL. LANDING PAN SHALL BE 12 GAUGE MINIMUM.
- STRINGERS, LANDING BEAMS, CLOSURE PLATES, STIFFENERS AND CONNECTION ANGLES SHALL BE OF SIZES DESIGNED BY THE STAIR PROVIDER AND SHALL RESIST A MINIMUM OF SELF WEIGHT OF STEEL, CONCRETE, ARCHITECTURAL FINISHES AND 100 PSF LIVE LOAD.
- AT LOWEST LEVEL, STAIRS MAY BE SUPPORTED FROM POST BEARING ON A FOOTING. THICKENED SLAB OR CONCRETE WALL PROVIDED SUPPORT BASE PLATE CAN REMAIN CONCEALED WITHIN ARCHITECTURAL ELEMENTS WITHOUT BEING EXPOSED.
- WHERE STAIR LANDINGS ARE SUSPENDED FROM FLOORS ABOVE, USE A MINIMUM OF DOUBLE ANGLE 3x3x1/4" HANGERS. HANG CENTRICALLY FROM FLOOR BEAM. CONTRACTOR SHALL COORDINATE WITH THE ARCHITECTURAL PARTITION SCHEDULE TO ENSURE HANGERS ARE CONCEALED WITHIN PARTITIONS. DO NOT USE ROD HANGERS.
- INTERMEDIATE LANDINGS SHALL BE COMPRISED OF CHANNELS SPANNING BETWEEN SUPPORT STRINGERS SPACED AT A MAXIMUM OF 2'-0" o.c. MAX.
- AT ORNAMENTAL STAIRS, STRINGERS SHALL BE COMPRISED OF TUBE STEEL OR CHANNELS WITH CONTINUOUS COVER PLATES TO FORM A TUBE. IN THIS CASE WELDS SHALL BE BLENDED AND GROUND SMOOTH.
- SEE ARCHITECTURAL DRAWINGS FOR ALL STAIR LAYOUTS, DIMENSIONS AND SECTIONS.

SUBMITTAL NOTES

- THE GENERAL CONTRACTOR SHALL SUBMIT A SCHEDULE OF SUBMITTALS PRIOR TO CONSTRUCTION BEGINNING ON THE PROJECT. THE SCHEDULE SHOULD DESCRIBE WHAT EACH SUBMITTAL IS, WHETHER IT IS THE ENTIRE PACKAGE, OR BROKEN INTO PHASES FOR REVIEW.
- THE GENERAL CONTRACTOR SHALL ALLOW FOR 10 BUSINESS DAYS OF ALL STRUCTURAL SUBMITTALS. IF THE CONTRACTOR WISHES TO EXPEDITE OR REDUCE THE REVIEW TIME, IT SHALL BE DONE FOR AN ADDITIONAL CHARGE AND MUST BE NEGOTIATED WITH THE ARCHITECT AND ENGINEER PRIOR TO THE SUBMISSION OF THE PACKAGE.
- IN THE EVENT THE CONTRACTOR ATTEMPTS TO SUBMIT THE ENTIRE PROJECT AT THE SAME TIME FOR SUBMISSION, ADDITIONAL TIME WILL BE REQUIRED BEYOND THE STANDARD 10 DAY REVIEW TIME.
- WHERE GLASS REQUIRES ADDITIONAL DESIGN OF MISCELLANEOUS STEEL AND MULLIONS, CONTRACTOR SHALL FURNISH DETAILED DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER IN THE STATE IN WHICH THE PROJECT IS BUILT.

CONCRETE NOTES

- REINFORCING STEEL FOR ALL MEMBERS SHALL HAVE A YIELD STRENGTH OF 60,000 PSI MINIMUM, EXCEPT FOR STIRRUPS, TIES AND HOOPS, WHICH SHALL HAVE A YIELD STRENGTH OF 40,000 PSI MINIMUM.
- UNLESS SHOWN OTHERWISE, FRAMED CONCRETE SLABS SHALL BE 4" THICK REINFORCED WITH #4 @ 8" o.c. EACH WAY.
- WHERE MAIN REINFORCING IN A SOLID SLAB IS ONE DIRECTION, PROVIDE #3 BARS AT 12" o.c. IN THE OTHER DIRECTION, UNLESS SHOWN OTHERWISE.
- BACKFILL SHALL NOT BE PLACED AGAINST BASEMENT WALLS UNTIL THE BASEMENT FLOOR SLAB AND THAT PORTION OF THE FIRST SLAB OVER THE BASEMENT ARE IN PLACE.
- UNLESS SHOWN OTHERWISE, CONCRETE WALLS 8" THICK OR LESS SHALL BE REINFORCED WITH #4 @ 8" o.c. EACH WAY. WALLS OVER 8" THICK SHALL BE REINFORCED WITH #4 @ 12" o.c. EACH WAY, EACH FACE.
- WHERE THE LENGTH OF A BAR IS GIVEN, AND IT IS TO BE HOOKED, THE HOOK SHALL BE IN ADDITION TO THE LENGTH GIVEN.
- WHERE OPENINGS OCCUR IN SLABS, PLACE THE REINFORCING THAT WOULD OCCUR IN LINE WITH THE OPENING EQUALLY TO EITHER SIDE OF THE OPENING. CUT NO STEEL IN THE FIELD.
- REINFORCING BARS THAT ARE TO BE WELDED SHALL BE OF A WELDABLE GRADE AND WELDED IN PROTECTIVE COVERING OF REINFORCEMENT (SEE DETAILS) SHALL BE AS FOLLOWS: FOOTINGS AND GRADE BEAMS 3" CLEAR BOTTOM AND SIDES, 1 1/2" CLEAR SIDES, BEAMS 1 1/2" CLEAR TO STIRRUPS, CONCRETE COLUMNS AND PIERS 1 1/2" CLEAR TO TIES, 3/4" CLEAR FOR CONCRETE JOIST TOP BARS.
- ALL CONCRETE FOR TOPPING SLAB SHALL BE REGULAR WEIGHT CONCRETE.

CONTRACTOR'S NOTES

- SEE ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWING, AND SPECIFICATIONS FOR SIZE AND LOCATION OF DRIPS, CHAMFERS, SLEEVES, ANCHORS, INSERTS AND OPENINGS REQUIRED. THE LOCATION AND SIZE OF SLEEVES OR OPENINGS NOT SHOWN ON THE DRAWINGS IN STRUCTURAL MEMBERS SHALL BE SUBJECT TO THE APPROVAL OF THE ARCHITECT. ANY CONFLICT SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT BEFORE PROCEEDING WITH THE WORK AFFECTED. PRINCIPAL OPENINGS IN THE STRUCTURE ARE INDICATED ON THE CONTRACT DRAWINGS. ANY SUBSTITUTIONS RESULTING IN REVISIONS TO THE STRUCTURE SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE WITH THE ARCHITECT.
- THE CONTRACTOR SHALL NOT SCALE THE CONTRACT DRAWINGS FOR THE PURPOSE OF ESTABLISHING CORRECT DIMENSIONS.
- U.N.O. DENOTES "UNLESS NOTED OTHERWISE"
- GENERAL CONTRACTOR SHALL FIELD VERIFY EXISTING SIZES, DIMENSIONS, NOTES OR CONDITIONS PRIOR TO ANY DETAILING, OR FABRICATION OF MATERIALS.

DECKING NOTES

- NO PIPING, DUCTWORK, OR CONDUIT LARGER THAN 3/4" DIAMETER OR STUD WALLS SHALL BE SUPPORTED DIRECTLY FROM METAL DECK OR PERMANENT METAL FORM.
- ALL METAL ROOF DECKS SHALL BE WIDE RIB AND GALVANIZED TYPE "B" DECKS UNLESS NOTED OTHERWISE.

STRUCTURAL STEEL NOTES

- FABRICATION AND ERECTION OF ALL STEEL SHALL BE IN ACCORDANCE WITH THE AISC SPECIFICATIONS.
- ALL BOLTS SHALL BE ASTM A-325 HIGH STRENGTH BOLTS U.N.O. (T.C. BOLTS)
- WHERE BEAMS REST ON MASONRY OR CONCRETE WALLS THEY SHALL BEAR A MINIMUM OF 4" AND SHALL HAVE BEARING PLATES AND ANCHORS.
- WHERE NOT SHOWN OTHERWISE, ALL COLUMNS SHALL HAVE BASE PLATES AND (4) 3/4" DIAMETER X 1'-2" LONG-HOOK ANCHOR BOLTS.
- UNLESS SHOWN OTHERWISE, PROVIDE 5/8" X 7'-1/2" BEARING PLATES ON 1" DIA. TUB WITH (4) 3/4" DIAMETER ANCHOR BOLTS UNDER ALL STEEL BEAMS THAT BEAR ON MASONRY WALLS. COLUMN BASES ARE DESIGNED AS UNRESTRAINED, THEREFORE COLUMNS MUST BE BRACED DURING ERECTION.
- AS ERECTION PROGRESSES, FRAMING SHALL BE ADEQUATELY BRACED AND BRACE CONNECTIONS SHALL BE SECURELY BOLTED OR WELDED.
- NO SHOP SPlice OR OTHER CONNECTION WILL BE PERMITTED UNLESS THE SPlice OR DETAIL SHOWN ON SHOP DRAWINGS AND REVIEWED BY THE ENGINEERS.
- ALL STEEL SHALL CONFORM TO ASTM A992, 50 KSI STEEL UNLESS SHOWN OTHERWISE. TUBE STEEL SHALL CONFORM TO ASTM A513, 48 KSI MINIMUM. ALL MISCELLANEOUS CONNECTION PLATES AND ANGLES SHALL BE 36 KSI STEEL.
- ALL FASTENERS SHALL CONSIST OF ONE NUT, ONE BOLT AND ONE WASHER.
- ALL BEAM SHOP CONNECTIONS SHALL BE WELDED. BEAM SHOP CONNECTIONS MAY BE BOLTED IN LIEU OF WELDING PROVIDED THAT THE CONTRACTOR ASSUMES AND RECEIVES AN AFFIRMATIVE ANSWER BY THE ENGINEER OF RECORD IN WRITING. FURTHERMORE, ACCEPTANCE OF A BOLTED CONNECTION WILL ONLY OCCUR IF THE FABRICATOR FOLLOWS THE ECCENTRIC LOAD TABLES PROVIDED ON PAGES 7-30 THROUGH 7-33 OF THE AISC MANUAL.
- FACE OF FRAMING MEMBER TO BE ECCENTRICITY OF CONNECTION SEE 7-30 AND 7-33, AISC CONNECTION EDITION
- ALL WELDING SHALL CONFORM TO THE STRUCTURAL WELDING CODE - AMERICAN WELDING SOCIETY* FOR WELD TESTING REQUIREMENTS. SEE SPECIFICATIONS.
- ALL ANCHOR BOLTS SHALL BE ASTM F-1554.
- NO SLOTTING IS PERMITTED UNLESS SPECIFICALLY DETAILED BY THE ENGINEER OF RECORD.
- IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO SECURE STEEL AGAINST DISPLACEMENT DURING ERECTION AND TO MAINTAIN IT AGAINST DISPLACEMENT UNTIL THE ERECTION OF ALL STEEL IS COMPLETED, ALL FLOOR AND ROOF DECKS ARE IN PLACE, AND ALL EXTERIOR MASONRY IS COMPLETED. ALL STRUCTURAL METAL WORK SHALL HAVE TEMPORARY JOISTS, BRACES AND STAYS TO HOLD IT IN POSITION UNTIL IT IS PERMANENTLY SECURE.
- GROUT UNDER BASE PLATES AND BEAM BEARING PLATES SHALL BE NON SHRINK GROUT. (10,000 PSI)
- WHERE ROOF DECKS FRAME ONTO A ROOF MEMBER, AND DECK IS IN A DIFFERENT PLANE THAN THE ROOF MEMBER, PROVIDE A 3/16" CONTINUOUS BENT PLATE WELDED ACROSS THE TOP OF THE ROOF MEMBER SLOPED TO MATCH PLANE OF ROOF DECK.
- FILL ALL CMU CELLS WITH GROUT WHERE REINFORCING AND/OR ANCHORS OCCUR.
- HEADED CONCRETE ANCHORS SHALL BE NELSON OR K.S.M. HEADED CONCRETE ANCHORS (OR APPROVED EQUAL) AND SHALL CONFORM TO ASTM A-108.
- ANCHORS SHALL BE AUTOMATICALLY END WELDED WITH SUITABLE STUD WELDING EQUIPMENT IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER.
- WHERE A ROOF FRAMING MEMBER DOES NOT FRAME OVER A COLUMN, PROVIDE A 1/2" THICK CAP PLATE (I.e. TUBES AND PIPES)
- FABRICATOR SHALL SUBMIT COMPLETE CALCULATIONS OF ALL CONNECTIONS, STRUCTURAL STEEL, AND JOIST SEALED BY THE REGISTERED DESIGN ENGINEER
- WHERE HSS OUTLOOKERS OCCUR PROVIDE 1/4" CAP PLATE AT END

STEEL BAR JOIST NOTES

- JOIST BRIDGING SHALL NOT BE USED TO SUPPORT CONDUIT, PIPING, DUCTWORK, ETC.
- JOISTS SHALL NOT BE FIELD MODIFIED EXCEPT AS SHOWN.
- WHERE JOISTS CANNOT ACHIEVE THE REQUIRED BEARING ON STEEL BEAMS, STAGGER JOISTS TO PROVIDE ADEQUATE BEARING.
- STEEL JOISTS SHALL BE OPEN WEB STEEL JOISTS OF THE SIZES AND SERIES SHOWN ON THE DRAWINGS. JOISTS, BRIDGING AND SPACING OF BRIDGING SHALL MEET THE REQUIREMENTS OF THE LATEST EDITION OF THE "STANDARD SPECIFICATIONS" OF THE STEEL JOIST INSTITUTE, EXCEPT WHERE OTHERWISE INDICATED BY THE DRAWINGS OR SPECIFICATIONS.
- WHERE ANGLE BRACES ARE SHOWN ON STRUCTURAL SECTIONS, JOIST MANUFACTURER SHALL RESOLVE AN AXIAL LOAD OF 2000 POUNDS FROM THE BRACE INTO THE JOIST - TYPICAL UNLESS NOTED OTHERWISE.
- IN ADDITION TO WHAT IS CALLED FOR ON PLAN, BAR JOISTS SHALL BE DESIGNED TO SUPPORT AN ADDITIONAL CONCENTRATED LOAD OF 300 POUNDS AT TOP OR BOTTOM CHORD AT ANY ONE LOCATION ALONG THE SPAN.
- AT THE END OF EACH ROOF JOIST, PROVIDE A CONTINUOUS ROW OF BRIDGING AT THE LAST BOTTOM CHORD PANEL POINT FOR UPLIFT. TYPICAL AT EACH END OF JOIST.

REVISIONS	NO.	DATE	COMMENTS
	01	15/19	Permit Set

RETAIL DEVELOPMENT
POP'S WINE & SPIRITS
 2-1764 TR A-B MCFARLAND 400 IND PARK
 MCFARLAND PKWY ALPHARETTA GA 30004

General Notes
 DATE: 01-15-19
 PROJ. NO.: 201812
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Order Plans

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