Statement of Special Inspections

This Statement of Special Inspections / Quality Assurance Plan includes the following building systems:

- ☑ Soils and Foundations
- ☑ Cast-in-Place Concrete
- ☐ Masonry
- ☑ Structural Steel
- ☑ Wood Construction
- ☑ Special Cases

General Notes

The inspectors and testing agencies shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official, prior to commencing work.

The qualifications of all personnel performing Special Inspections and testing activities are subject to the approval of the Building Official and E.O.R. The credentials of all inspectors and testing technicians shall be provided if requested.

The special inspectors shall keep records of inspections and shall furnish inspection reports to the owner, Engineer of Record (E.O.R.) and Architect of Record (A.O.R.). Field and testing result reports shall be submitted to all designated parties as they are completed. The reports shall indicate that the work performed was done in accordance to the construction drawings. Discrepancies shall be brought to the attention of the general contractor for correction. If the discrepancies are not corrected, the discrepancies shall be brought to the attention of the E.O.R. prior to completion of that phase of work. A final report that documents required special inspections and corrections of discrepancies shall be submitted by the General Contractor to the Owner, E.O.R. and A.O.R.

Soils and Foundations

Item	Scope	Monitoring: Periodic (P) Continuous (C)
1. Shallow Foundations	*Inspect soils below footings for adequate bearing capacity and consistency with geotechnical report.* *Inspect removal of unsuitable material and preparation of subgrade prior to placement of controlled fill.*	P C
2. Controlled Structural Fill	*Perform sieve tests (ASTM D422 & D1140) and modified Proctor tests (ASTM D1557) of each source of fill material.* *Inspect placement, lift thickness and compaction of controlled fill.* *Test density of each lift of fill by nuclear methods (ASTM D2922)* *Verify extent and slope of fill placement.*	C

Note:
1. Special Inspection is not required during placement of controlled fill having a total depth of 12 inches or less.

Cast-in-Place Concrete

Item	Scope	Monitoring: Periodic (P) Continuous (C)
1. Mix Design	*Review concrete batch tickets and verify compliance with approved mix design. Verify that water added at the site does not exceed that allowed by the mix design. Submit proposed mix design of each class of concrete to Structural Engineer of Record and to inspection and testing firm for review prior to commencement of work.*	P
2. Material Certification	*Review for conformance to contract documents. Submit to Structural Engineer of Record for review.*	P
3. Anchor Rods	*Inspect size, positioning and embedment of anchor rods. Inspect concrete placement and consolidation around anchors.*	C
4. Concrete Placement	*Inspect placement of concrete. Verify that concrete conveyance and depositing avoids segregation or contamination. Verify that concrete is properly consolidated.*	C
5. Sampling and Testing of Concrete	*Test concrete compressive strength (ASTM C31 & C39), slump (ASTM C143), air-content (ASTM C231 or C173) and temperature (ASTM C1064). Three concrete test cylinders will be taken for every 75 or less cubic yards of each class of concrete placed, or concrete placed on any given day. One additional test cylinder will be taken during cold weather concreting, cured on job site under same conditions as concrete represents.*	C
6. Curing and Protection	*Inspect curing, cold weather protection and hot weather protection procedures.*	P

Note: Special Inspection is not required for flatwork patios, driveways and sidewalks, on grade not shown on structural drawings.

Structural Steel

Item	Scope	Monitoring: Periodic (P) Continuous (C)
1. Fabricator Certification/ Quality Control Procedures ☐ Fabricator Exempt	*Review shop fabrication and quality control procedures. (Note: Fabricator may be exempt if certified by AISC.)*	P
2. Material Certification	*Review certified mill test reports and identification markings on wide-flange shapes, high-strength bolts, nuts and welding electrodes.*	P
3. Welding	*Visually inspect all welds. Inspect pre-heat, post-heat and surface preparation between passes. Verify size and length of fillet welds.* *Ultrasonic testing of all full-penetration welds.* *Complete and partial penetration groove welds shall be inspected continuously* *Multi-pass fillet welds shall be inspected continuously.* *Single-pass fillet welds (> 5/16") shall be inspected continuously.* *Single-pass fillet welds (< 5/16") shall be inspected periodically.* *Floor and deck welds shall be inspected periodically.*	.
4. Structural Details	*Inspect steel frame for compliance with structural drawings, including bracing, member configuration and connection details.*	P

Wood Construction

Item	Scope	Monitoring: Periodic (P) Continuous (C)
1. Fabricator Certification/ Quality Control Procedures ☐ Fabricator Exempt	*Inspect shop fabrication and quality control procedures for wood truss plant.* *Confirm certification of supplier.*	P
2. Material Grading	*Inspect grade stamps on structural lumber for compliance with the contract documents.*	P
3. Connections	*Verify connection hardware and its installation. Inspect bearing, nails, bolts, hangers or clips, or other devices are tight and otherwise properly installed per the contract documents.*	C
4. Framing and Details	*Inspect members for size and placement for conformance to the SER approval submittals and contract document. Review engineered joist shop drawings. Submit to SER for review.*	P
5. Diaphragms & Shearwalls	*Inspect thickness and grade of plywood (or OSB), blocking, placement, embedment, size of hold down anchors and the edge and field nailing of the plywood (or OSB) to the framing for conformance to the contract documents.*	C
6. Prefabricated Wood Trusses	*Inspect the fabrication of wood trusses. Bottom chord splices are prohibited in the middle third point of the truss.*	P
7. Permanent Truss Bracing	*Bridging and bracing installed per the approved truss shop drawings.*	P

Special Cases

Item	Scope	Monitoring: Periodic (P) Continuous (C)
Epoxy Anchors in Concrete or CMU	*Review anchors and product being used for conformance to contract documents. Observe installation for compliance to manufacturers specifications. Perform pull test to 125% of allowable design load per manufacturer specifications. (Minimum of 10% of total anchors, to include a minimum of one of each type, size or embedment.)*	C

STRUCTURAL NOTES

GENERAL

1. THIS PROJECT SHALL MEET ALL REQUIREMENTS OF THE 2017 FLORIDA BUILDING.
2. THE GENERAL CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF ALL ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL OPENINGS (COORDINATE WITH APPLICABLE TRADES). THE CONTRACTOR SHALL PROVIDE FOR ALL OPENINGS, WHETHER SHOWN ON THE STRUCTURAL DRAWINGS OR NOT. ANY DEVIATION FROM OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ENGINEER'S ATTENTION FOR APPROVAL PRIOR TO CONSTRUCTION.
3. THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS BEFORE CONSTRUCTION AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES OR INCONSISTENCIES BEFORE PROCEEDING WITH THE WORK.
4. COMPLETE SHOP DRAWINGS AS REQUIRED FOR THE STRUCTURAL WORK SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO COMMENCEMENT OF CONSTRUCTION IN ACCORDANCE WITH THE SPECIFICATIONS. SUCH REVIEW BY THE ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR CORRECT FABRICATION AND CONSTRUCTION OF THE WORK. ALLOW TEN (10) BUSINESS DAYS FOR REVIEW FROM THE TIME SUBMITTALS ARE RECEIVED IN OUR OFFICE.
5. STRUCTURAL CONTRACT DRAWINGS SHALL NOT BE REPRODUCED, IN PART OR WHOLE, FOR USE AS SHOP DRAWINGS. SHOP DRAWINGS SUBMITTED AS SUCH WILL BE REJECTED WITHOUT REVIEW.
6. ANY DEVIATION FROM, ADDITION TO, SUBSTITUTION FOR, OR MODIFICATION TO THE STRUCTURE OR ANY PART OF THE STRUCTURE DETAILED ON THESE DRAWINGS SHALL BE SUBMITTED IN WRITING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS THAT ARE SUBMITTED FOR REVIEW DO NOT CONSTITUTE "IN-WRITING" UNLESS IT IS CLEARLY NOTED THAT SPECIFIC CHANGES ARE BEING SUGGESTED.
7. THE STRUCTURAL DRAWINGS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.
8. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN AND OTHER PERSONS DURING CONSTRUCTION.

SPECIAL INSPECTION

THE FOLLOWING ITEMS REQUIRE SPECIAL INSPECTION:
(REFERENCE SHEET S-000 FOR ADDITIONAL INFORMATION.)

1. SOILS AND FOUNDATIONS
2. CAST-IN-PLACE CONCRETE
3. STRUCTURAL STEEL
4. WOOD CONSTRUCTION
5. SPECIAL CASES (EPOXY ANCHORS)

DESIGN LOADS

1. ROOF DEAD LOADS: 20 PSF
   - 1.1. REFER TO ROOF PLAN FOR ADDITIONAL MECHANICAL EQUIP LOADS
2. ROOF LIVE LOADS: 20 PSF
3. ROOF SNOW DATA:
   - 3.1. GROUND SNOW LOAD, PG : 0 PSF
   - 3.2. EXPOSURE FACTOR, C : 1.0
   - 3.3. IMPORTANCE FACTOR, I : 1.0
   - 3.4. THERMAL FACTOR : 1.0
   - 3.5. FLAT ROOF SNOW LOAD, PSF : 0 PSF
4. WIND DESIGN DATA (ASCE 7-10):
   - 4.1. BASIC WIND SPEEDS (3 SEC. GUST): Vult = 150 MPH, Vasd = 116 MPH
   - 4.2. IMPORTANCE FACTOR, I : 1.0
   - 4.3. OCCUPANCY CATEGORY : II
   - 4.4. EXPOSURE CATEGORY : B
   - 4.5. INTERNAL PRESSURE COEFFICIENT: 0.18
5. COMPONENT & CLADDING LOAD : SEE 2/S-100
6. SEISMIC DESIGN DATA:
   - 6.1. IMPORTANCE FACTOR, I : 1.0
   - 6.2. OCCUPANCY CATEGORY : II
   - 6.3. MAPPED SPECTRAL RESPONSE ACCELERATIONS:
     - 6.3.1. Ss = 0.097
     - 6.3.2. S1 = 0.056
   - 6.4. SITE CLASS : D (ASSUMED)
   - 6.5. SPECTRAL RESPONSE COEFFICIENTS:
     - 6.5.1. Sds = 0.103
     - 6.5.2. Sd1 = 0.089
   - 6.6. DESIGN CATEGORY : B
   - 6.7. BASIC SEISMIC FORCE RESISTING SYSTEM: PLYWOOD SHEAR WALLS
   - 6.8. RESPONSE MODIFICATION FACTOR, R = 6.5
   - 6.9. RESPONSE COEFFICIENT, Cs = 0.016
   - 6.10. ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE
   - 6.11. DESIGN BASE SHEAR : 1.53 KIPS

FOUNDATION DESIGN AND SITEWORK FOR BUILDING (NOTE: UPDATE PER SITE SPECIFIC GEOTECHNICAL REPORT)

1. THE ACTUAL BEARING GRADE SHALL BE CHECKED IN THE FIELD DURING CONSTRUCTION BY A SOILS CONSULTANT TO ENSURE THAT THE FOUNDATION SYSTEM IS SUPPORTED BY SUITABLE BEARING SOILS AND WILL PERFORM IN ACCORDANCE WITH THE ASSUMED CONDITIONS.
2. FOUNDATION DESIGN AND SITEWORK SHALL BE VERIFIED PRIOR TO PLACEMENT OF CONCRETE. A WRITTEN VERIFICATION SIGNED AND SEALED BY A GEOTECHNICAL ENGINEER SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD STATING THAT THE BEARING ELEVATIONS EXPOSED AFTER SITE STRIPPING HAVE BEEN INSPECTED AND ARE ADEQUATE TO SUPPORT A MINIMUM OF 2000 PSF (ALLOWABLE SOIL BEARING CAPACITY) AND THE SOILS SUPPORTING THE SLAB-ON-GRADE ARE ADEQUATE TO MINIMIZE DIFFERENTIAL MOVEMENT TO LESS THAN ½" AND TOTAL MOVEMENT TO LESS THAN 1".
3. REMOVE ALL VEGETATION AND DEBRIS, INCLUDING PAVEMENTS, SIDEWALKS, BUILDING FOUNDATIONS, AND ABANDONED UTILITIES.
4. REMOVE ORGANIC SOILS PER GEOTECHNICAL RECOMMENDATIONS.
5. PROOFROLL THE EXPOSED SUBGRADE TO DETECT SOFT OR YIELDING SOILS. REMOVE ANY SOFT OR YIELDING SOILS, SCARIFY, MOISTURE CONDITION AND RECOMPACT IN ACCORDANCE WITH ASTM D-698.
6. SOILS EXCAVATED FROM THE SITE THAT ARE FREE OF DELETERIOUS MATERIALS MAY BE USED AS FILL.
7. RAISE EXCAVATIONS AND LOW AREAS WITH COMPACTED FILL (ASTM D-698).
8. STRUCTURAL FILL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-698. THE IN-PLACE MOISTURE CONTENT FOR COHESIVE SOILS SHALL NOT VARY BY MORE THAN -1% TO +3% AND GRANULAR SOIL ±3% OF THE OPTIMUM MOISTURE CONTENT.
9. FOUNDATION DESIGN IS BASED ON AN ALLOWABLE BEARING PRESSURE OF 2,000 PSF AT MINIMUM 36" INTO SUITABLE EXISTING SOILS. MINIMUM CONTINUOUS FOOTING WIDTH IS 16 INCHES.
10. PROVIDE POSITIVE DRAINAGE AWAY FROM EXCAVATIONS SO AS NOT TO ALLOW STANDING WATER FOR LONG PERIODS OF TIME.
11. PROVIDE A 10 MIL THICK VAPOR BARRIER BETWEEN THE COMPACTED BASE AND CONCRETE SLAB.
12. DO NOT PUNCTURE THE VAPOR BARRIER. LAP AND TAPE ENDS.
13. BACKFILL AND COMPACT UTILITY TRENCHES AS DESCRIBED ABOVE.
14. PERFORM ALL SITEWORK UNDER THE DIRECT SUPERVISION OF A GEOTECHNICAL ENGINEER.

CONCRETE

1. ALL CONCRETE SHALL BE NORMAL WEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS, (U.N.O.).
2. MINIMUM CEMENT CONTENT SHALL BE 5 SACKS PER CUBIC YARD.
3. TYPE C OR F FLY ASH MAY BE USED UP TO 20% OF TOTAL CEMENT CONTENT BY VOLUME. THIS IS ONLY FOR CONCRETE SPECIFIED IN THESE STRUCTURAL DRAWINGS. REFER TO SPECIFICATIONS BY OTHER DISCIPLINES.
4. MAXIMUM SLUMP SHALL BE 5 IN., U.N.O.
5. MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE AMERICAN CONCRETE INSTITUTE'S "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS", ACI 301.
6. DO NOT USE CALCIUM CHLORIDE.
7. CONCRETE TEST REPORTS SHALL BE MADE AVAILABLE AT THE JOB SITE.

REINFORCING STEEL

1. BARS SHALL BE ASTM A615, GRADE 60.
2. DETAIL, FABRICATE, AND PLACE IN CONFORMANCE WITH ACI 315 AND 318.
3. LAP ALL REINFORCING STEEL 40 BAR DIAMETERS (U.N.O.).
4. LAP CONTINUOUS BARS IN GRADE BEAMS 40 BAR DIAMETERS (U.N.O. ON DRAWINGS). TOP BARS TO BE SPLICED BETWEEN SUPPORTS AND BOTTOM BARS TO BE SPLICED AT SUPPORTS, AS APPLICABLE.
5. PROVIDE ACCESSORIES FOR SUPPORT OF ALL REINFORCING.
6. SUBMIT SHOP DRAWINGS SHOWING ALL REINFORCING FOR APPROVAL BY THE STRUCTURAL ENGINEER OF RECORD.
7. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:
   - 7.1. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH – 3"
   - 7.2. CONCRETE EXPOSED TO EARTH OR WEATHER – 2"
   - 7.3. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND – ¾"

CONCRETE MASONRY UNITS

1. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, TYPE 1, WITH COMPRESSIVE STRENGTH (f'm) OF 1,500 PSI
2. GROUT FOR CONCRETE MASONRY SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI
3. MORTAR SHALL BE TYPE N, U.N.O.
4. REINFORCING IN MASONRY WALLS SHALL BE GRADE 60, ASTM A615.
5. TYPICAL REINFORCING:
   - 5.1. VERTICAL – REF. PLAN FOR SIZE AND SPACING
   - 5.2. HORIZONTAL – #9 WIRE IN DUR-O-WALL (LADUR TRUSS TYPE) OR APPROVED EQUAL AT 16" O.C. IN MASONRY WALLS, EXCEPT AT 8" O.C. FOR WALL BELOW GRADE OR STACK BOND WALLS (U.N.O. ON DRAWINGS).
   - 5.3. MATERIALS AND WORKMANSHIP SHALL CONFORM TO DESIGN REQUIREMENTS BASED UPON THE AMERICAN CONCRETE INSTITUTES "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES", ACI 530.

POST-INSTALLED ANCHORS

1. EXCEPT WHERE NOTED ON DRAWINGS, USE WWW.STRONGTIE.COM FOR ADDITIONAL PRODUCT DATA. IT IS ACCEPTABLE TO USE THE SIMPSON SET-XP ADHESIVE SYSTEM OR APPROVED EQUAL (TYP., U.N.O.) ICC ESR-2508.
2. EXCEPT WHERE INDICATED ON THE DRAWINGS, HILTI PRODUCTS MAY BE USED. CONTACT HILTI AT (800) 879-8000 FOR PRODUCT RELATED QUESTIONS.
3. ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED OR SUCH OTHER METHOD AS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE.
4. INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING.
5. INSTALL ACCORDING TO MANUFACTURER'S SPECIFICATIONS. THREADED ROD AND REBAR DIAMETERS AND EMBEDMENT LENGTHS SHALL BE AS NOTED ON DRAWINGS.
6. OVERHEAD ADHESIVE ANCHORS MUST BE INSTALLED USING PRODUCTS WHICH HAVE SPECIFIC APPLICATIONS THAT ARE INTENDED FOR OVERHEAD USE.
7. THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
8. ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
9. EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY FERROSCAN, GPR, X-RAY, CHIPPING, OR OTHER MEANS.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHAPES AND PLATES SHALL BE ASTM A36, U.N.O.
2. ANCHOR BOLTS SHALL BE ASTM F1554, TYPICAL, U.N.O.
3. TUBE STEEL SHALL BE ASTM A500, GRADE B.
4. BOLTS SHALL BE ASTM A325, ¾" DIA. (MIN.), U.N.O.
5. STEEL DESIGN IS BASED ON THE CURRENT AISC MANUAL OF STEEL CONSTRUCTION (ASD).
6. WELDING ELECTRODES SHALL BE SERIES E70XX.
7. PERFORM WELDING IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY'S AWS D1.1 "STRUCTURAL WELDING CODE".
8. STEEL FABRICATORS SHALL BE A COMPANY THAT IS CERTIFIED BY AISC. (NOTE: IF NOT CERTIFIED BY AISC, THEN SPECIAL INSPECTION FOR REVIEW OF SHOP FABRICATION AND QUALITY CONTROL PROCEDURES IS REQUIRED.)
9. SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF MEMBERS, BRIDGING, BRACING AND ERECTION DETAILS.
10. SHOP PAINT: REFER TO SPECIFICATIONS.
11. FASTENERS FOR PRESERVATIVE-TREATED AND FIRE-RETARDANT-TREATED WOOD SHALL BE HOT-DIPPED ZINC-COATED GALVANIZED STEEL.

LIGHT GAGE METAL FRAMING

1. STEEL STUD SIZES SHALL BE AS NOTED ON THE DRAWINGS, CONFORMING TO SSMA STANDARDS.
2. FABRICATE AND ERECT AS SPECIFIED. MANUFACTURER SHALL SUPPLY ALL CLIPS, FASTENERS, TEMPORARY BRACING, ACCESSORIES, STANDARD BRIDGING, ETC.
3. ALL STEEL STUDS SHALL HAVE A MINIMUM 1⅝ " FLANGE WITH A ½" RETURN (U.N.O.).
4. ALL STUDS 18 GAGE AND THINNER SHALL HAVE A MINIMUM YIELD STRENGTH, Fy, OF 33 KSI AND TENSILE STRENGTH, Fu, OF 45 KSI. ALL STUDS 16 GAGE AND THICKER SHALL HAVE A MINIMUM STRENGTH OF 50 KSI AND TENSILE STRENGTH OF 65 KSI (U.N.O.).
5. TRACK SECTIONS SHALL BE EQUAL GRADE AND GAGE THICKNESS OF STUDS BEING USED. TYPICAL, U.N.O.
6. ALL FASTENERS SHALL BE SELF-TAPPING NO. 12-14 GAGE SCREWS, OR WELD IN ACCORDANCE WITH SECTION 6.0 OF THE AMERICAN WELDING SOCIETY'S "STRUCTURAL WELDING CODE – SHEET METAL" (AWS D1.3) AS SHOWN ON DRAWINGS.

STRUCTURAL WOOD

1. WOOD FRAMING SHALL COMPLY WITH THE SOUTHERN PINE INSPECTION BUREAU, OR SHALL CONFORM TO SPECIFICATIONS AS PUBLISHED BY THE WESTERN WOODS PRODUCTS ASSOCIATION.
2. WOOD FRAMING EXCEPT EXTERIOR WALL STUDS 2x (NOMINAL) TO 4x (NOMINAL) SHALL BE NO. 1 SOUTHERN YELLOW PINE, NO. 1 DOUGLAS FIR LARCH, OR EQUIVALENT (U.N.O.).
3. EXTERIOR WALL STUDS SHALL BE SELECT STRUCTURAL SOUTHERN YELLOW PINE, SELECT STRUCTURAL DOUGLAS FIR LARCH, OR EQUIVALNET
4. WOOD COLUMNS 6 INCHES x 6 INCHES AND LARGER SHALL BE SELECT STRUCTURAL SOUTHERN YELLOW PINE, SELECT STRUCTURAL DOUGLAS FIR LARCH, OR EQUIVALENT.
5. ALL EXPOSED WOOD RAFTERS AND COLUMNS SHALL BE "SELECT" GRADE AS DESCRIBED IN AITC.
6. ALL PLATES IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED (USE CATEGORY 2 AS SPECIFIED BY AWPA) FOR MOISTURE PROTECTION. ALL WOOD EXPOSED TO WEATHER SHALL BE PRESSURE TREATED (USE CATEGORY 3B AS SPECIFIED BY AWPA) OR WESTERN RED CEDAR.
7. GLUED-LAMINATED MEMBERS SHALL BE DOUGLAS FIR INDUSTRIAL GRADE (24F-V8) OR APPROVED EQUAL U.N.O. MEMBER SIZES ARE BASED ON CURRENT NDS WESTERN SPECIES. GRADE 24F-V4 MAY BE SUBSTITUTED FOR SIMPLY SUPPORTED SPANS ONLY.
8. PRE-FABRICATED METAL PLATED WOOD TRUSSES
   - 8.1. TRUSS FABRICATION AND INSTALLATION SHALL COMPLY WITH THE FOLLOWING STANDARDS:
9. ANSI/TPI 1 "NATIONAL DESIGN STANDARD FOR METAL-PLATE-CONNECTED WOOD TRUSS CONSTRUCTION".
10. TPI HIB "COMMENTARY AND RECOMMENDATIONS FOR HANDLING INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES".
11. TPI DSB "RECOMMENDED DESIGN SPECIFICATION FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES".
12. TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS:
    - 12.1. TOP CHORD:
      - 12.1.1. DEAD LOAD = 10 PSF
      - 12.1.2. LIVE LOAD = 20 PSF
    - 12.2. BOTTOM CHORD:
      - 12.2.1. DEAD LOAD = 10 PSF
      - 12.2.2. LIVE LOAD = 5 PSF (NON-CONCURENT)
13. REFERENCE ROOF PLAN FOR ADDITIONAL MECHANICAL EQUIPMENT LOADS.
14. REQUIRED UPLIFT
    - 14.1. ALL TRUSS-TO-TRUSS CONNECTORS SHALL BE SPECIFIED BY THE TRUSS MANUFACTURER. TRUSS-TO-FRAMING CONNECTORS SHALL BE SPECIFIED BY THE ENGINEER OF RECORD, UNLESS SPECIFICALLY NOTED.
    - 14.2. BOTTOM CHORD TENSION SPLICES ARE PROHIBITED WITHIN THE MIDDLE ⅓ OF THE SPAN.
    - 14.3. FABRICATOR SHALL SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF MEMBERS, BRIDGING, BRACING, ERECTION DETAILS, TRUSS PENETRATIONS, AND DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER (STATE).
15. ROOF DECK
    - 15.1. ALL ROOF DECK SHALL BE APA RATED CDX EXTERIOR PLYWOOD OR ORIENTED STRAND BOARD.
    - 15.2. ROOF SHEATHING SHALL BE 23/32 INCH THICK MINIMUM (48/24) U.N.O.
    - 15.3. STAGGER ENDS OF SHEETS.
    - 15.4. PROVIDE BLOCKING AT EDGES OF ALL ROOF SHEETS.
    - 15.5. NAIL EDGES OF ROOF SHEETS AT 6 IN. O.C. MAXIMUM (U.N.O.).
    - 15.6. NAIL FACES OF ROOF SHEETS AT 12 IN. O.C. MAXIMUM.
    - 15.7. USE MINIMUM 10d COMMON NAILS (U.N.O.).
16. WALL SHEATHING
    - 16.1. ALL WALL SHEATHING SHALL BE APA RATED CDX EXTERIOR PLYWOOD OR ORIENTED STRAND BOARD.
    - 16.2. WALL SHEATHING SHALL BE 15/32 INCH THICK MINIMUM (32/16) U.N.O.
    - 16.3. STAGGER ENDS OF SHEETS.
    - 16.4. PROVIDE BLOCKING AT EDGES OF ALL SHEARWALL SHEETS.
    - 16.5. NAIL EDGES OF SHEARWALL SHEETS PER SCHEDULE ON PLAN (OTHER WALLS AT 6 IN. O.C. MAXIMUM).
    - 16.6. NAIL FACES OF WALL SHEETS AT 12 IN. O.C. MAXIMUM.
    - 16.7. USE MINIMUM 10d COMMON NAILS (U.N.O.).
17. CONNECTORS SHALL BE AS MANUFACTURED BY THE SIMPSON CO. OR APPROVED EQUAL. CONNECTORS USED WITH PRESSURE TREATED LUMBER OR IN UNCONDITIONED SPACE, SHALL HAVE THE ZMAX (G185) COATING.
18. NAILING, UNLESS NOTED OTHERWISE, SHALL BE PER THE 2009 INTERNATIONAL BUILDING CODE.

PANDA EXPRESS, INC.
1683 Walnut Grove Ave.
Rosemead, California
91770
Telephone: 626.799.9898
Facsimile: 626.372.8288

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REVISIONS:

ISSUE DATE:

1st	ISSUE FOR BUILDING PERMIT	08-16-18
2nd	ISSUE FOR BID	11-29-18

DRAWN BY: EMC

PANDA PROJECT #: S8-19-D6558
ARCH PROJECT #: 1804

EMC
STRUCTURAL ENGINEERS, P.C.
4525 Trousdale Drive
Nashville, Tennessee 37204
(615) 781-8199
(615) 781-4088 (fax)

EMC Project No. 18254

MARK W. SAVAGE
LICENSE
No. 81067
STATE OF FLORIDA
PROFESSIONAL ENGINEER
8-19-2018

PANDA EXPRESS
US 98 and PORTSIDE DRIVE
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