GALLOWAY & ASSOCIATES, INC.
STRUCTURAL ENGINEERING
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STRUCTURAL NOTES

A. GENERAL

1. THE DRAWINGS ARE INTENDED TO SHOW THE GENERAL ARRANGEMENT, DESIGN AND EXTENT OF THE WORK AND ARE PARTIALLY DIAGRAMMATIC. THEY ARE NOT INTENDED TO BE SCALED FOR ROUGH-IN MEASUREMENTS, OR TO SERVE AS SHOP DRAWINGS OR PORTIONS THEREOF.
2. ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT, EXCEPT WHERE A DIFFERENT DETAIL OR SECTION IS SHOWN.
3. PRIOR TO START OF CONSTRUCTION, THE CONTRACTOR AND ALL THE SUB-CONTRACTORS SHALL VERIFY ALL GRADES, UNITS, LEVELS, DIMENSIONS AND COORDINATE EXISTING CONDITIONS AT THE JOB SITE WITH THE PLANS AND SPECIFICATIONS. THEY SHALL REPORT ANY INCONSISTENCIES OR ERRORS IN THE ABOVE TO THE ARCHITECT/ ENGINEER BEFORE COMMENCING WORK. THE CONTRACTOR AND HIS SUBCONTRACTORS SHALL LAY OUT THEIR WORK FROM ESTABLISHED REFERENCE POINTS AND BE RESPONSIBLE FOR ALL LINES, ELEVATIONS AND MEASUREMENTS IN CONNECTION WITH THEIR WORK.
4. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES ANY REQUIRED SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIE DOWNS WHICH MIGHT BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER THE COMPLETION OF THE PROJECT.
5. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.
6. THE CONTRACTOR SHALL USE THE STRUCTURAL DRAWINGS AND SPECIFICATIONS TOGETHER WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND OTHER TRADE DRAWINGS AND SHOP DRAWINGS, TO COORDINATE ALL DETAILS, DIMENSIONS, ELEVATIONS, ETC. NOTIFY ARCHITECT/ENGINEER, IN WRITING, OF ANY POTENTIAL CONFLICTS BEFORE PROCEEDING WITH THE WORK.
7. SHOULD ANY OF THE DETAILED INSTRUCTIONS SHOWN ON THE PLANS CONFLICT WITH THESE STRUCTURAL NOTES, THE SPECIFICATIONS, OR WITH EACH OTHER, THE STRICTEST PROVISION SHALL GOVERN.

B. GOVERNING CODE: 2012 - INTERNATIONAL BUILDING CODE

1. ROOF SNOW LOADS
   A. GROUND SNOW LOAD: Pg = 5 PSF. (FIGURE 1608.2)
   B. FLAT-ROOF SNOW LOAD: Pf = 5 PSF. (ASCE 7, SECT 7.3)
   C. SNOW EXPOSURE FACTOR: Ce = 1.0 (TABLE 1608.3.1)
   D. SNOW LOAD IMPORTANCE FACTOR: Is = 1.0 (TABLE 1604.5)
   E. SNOW THERMAL FACTOR: Ct = 1.0 (TABLE 1608.3.2)
   F. ALL APPLICABLE EFFECTS DUE TO SNOW DRIFTING (SECTION 1608)
2. ROOF LIVE LOADS
   A. MINIMUM ROOF LIVE LOAD = 20 PSF. (SECTION 1607.11.2)
   B. SEE "PREFABRICATED WOOD TRUSSES" DESIGN CRITERIA FOR ADDITIONAL LOADING INFORMATION.
3. WIND LOADS
   A. BASIC WIND SPEED = 115 MPH (FIGURE 1609)
   B. WIND LOAD IMPORTANCE FACTOR: Iw = 1.0 (TABLE 1604.5)
   C. WIND EXPOSURE CATEGORY "B" (SECTION 1609.4)
4. SEISMIC DESIGN DATA:
   A. SEISMIC SITE CLASS – BASED ON SECTION 1615.1
   B. SEISMIC IMPORTANCE FACTOR: Ie = 1.0 (TABLE 1604.5)
   C. SITE (SOIL) CLASS – SECTION 1615.1.2
   D. STRUCTURAL FRAMING AND SEISMIC RESISTING SYSTEM: LIGHT-FRAMED WALLS WITH SHEAR PANELS (TABLE 1617.6.2, ITEM 1 K)

C. FOUNDATION

1. FOUNDATIONS ARE DESIGNED TO BEAR ON NATURAL GRADE OR FILL WELL COMPACTED OF AN ALLOWABLE BEARING CAPACITY, INDICATED ON THE FOUNDATION PLAN.
2. A CERTIFIED TESTING LABORATORY SHALL BE ENGAGED BY THE OWNER TO PERFORM SOIL BORINGS, PROVIDE A FOUNDATION REPORT AND VERIFY THAT THE REQUIRED MINIMUM BEARING CAPACITY WAS OBTAINED.
3. SAID SOIL CAPACITY SHALL BE CERTIFIED AND TESTED BY A REGISTERED FOUNDATION ENGINEER, PRIOR TO CASTING OF CONCRETE IN THE FOOTINGS.
4. BOTTOM OF FOOTING ELEVATION TO BE DETERMINED BY THE SOIL CONDITIONS AND FROST-LINE DEPTH.
5. ALL LONGITUDINAL REBARS IN THE WALL FOOTINGS, SHALL BE CONTINUOS AND SPLICED AS SPECIFIED. CONTINUE ALL HORIZONTAL REBARS AT BENTS AND CORNERS BY BENDING THE REBARS 48 BAR DIAMETERS AROUND THE CORNERS OR ADDING MATCHING CORNER BARS, EXTENDING 48 BAR-DIAMETERS INTO FOOTING EACH SIDE OF CORNER OR BENT.

D. REINFORCED CONCRETE

1. MATERIALS:
   A. SPECIFICATIONS: IN GENERAL, COMPLY WITH ACI 301-(LATEST EDITION) "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS."
   B. STRUCTURAL CONCRETE:

CLASS	LOCATION	F'_c
I.	FOOTINGS, CAISSONS, & GRADE BEAMS	3,000
II.	INTERIOR SLABS ON GRADE, AND ALL INTERIOR CONCRETE NOT OTHERWISE IDENTIFIED	3,500
III.	PIERS PLACED INTEGRALLY WITH WALLS, EXTERIOR SLABS ON GRADE, AND ALL EXTERIOR CONCRETE (WITH AIR) NOT OTHERWISE IDENTIFIED	4,000
IV.	BACKFILL BELOW FOOTINGS AND GRADE BEAMS	1,500

   C. ALL DEFORMED REINFORCING BARS: fY = 60,000.
   D. GALVANIZED WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 (LATEST EDITION). USE SHEET FORM, NOT ROLLED.
2. FIELD MANUAL: PROVIDE AT LEAST ONE COPY OF THE ACI FIELD REFERENCE MANUAL SP-15, IN THE FIELD OFFICE AT ALL TIMES.
3. CONTINGENCIES:
   A. PROVIDE SUPPORTS AS REQUIRED TO MAINTAIN ALIGNMENT AND CONCRETE COVER OVER THE REINFORCING.
4. FOOTINGS:
   A. VERTICAL DOWELS IN FOOTINGS TO MATCH VERTICAL WALL REINFORCING.
   B. PROVIDE LEAN CONCRETE (CLASS IV) UNDER FOUNDATIONS FOR ACCIDENTAL OVER-EXCAVATION, SOFT SPOTS AND TRENCHES.
5. SPLICES: UNLESS NOTED OTHERWISE, MINIMUM LAP SPLICE LENGTHS TO BE AS FOLLOWS:

A. VERTICAL BARS IN WALLS, PIERS, OR COLUMNS (INCLUDING DOWELS)	30 DIAMETER
B. HORIZONTAL BARS IN SLABS & FOOTING	35 DIAMETER
C. HORIZONTAL BARS IN WALL	45 DIAMETER

6. SAW-CUT & CONSTRUCTION JOINTS:
   PROVIDE JOINTS IN ALL SLABS-ON-GRADE, AS INDICATED ON THE FOUNDATION PLAN.
7. CONCRETE COVER: UNLESS NOTED OTHERWISE, DETAIL REINFORCING TO PROVIDE CONCRETE COVER AS FOLLOWS:

A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3 INCHES
B. CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 BARS AND SMALLER	1-1/2 INCHES
OTHERS	2 INCHES
C. CONCRETE NOT EXPOSED TO EARTH OR WEATHER:	
BEAM AND COLUMN BARS INCLUDING TIES, STIRRUPS AND SPIRALS	1-1/2 INCHES
SLABS, WALLS, JOISTS:	
#11 BARS AND SMALLER	1 INCH
OTHERS	1-1/2 INCHES

E. STRUCTURAL STEEL

1. MATERIALS:
   A. STRUCTURAL STEEL: ASTM A36, FY = 36,000 PSI, U.O.N.
   ANCHOR BOLTS: ASTM F1554 GR A36; ELECTRODES: SEE NOTE 2.
   EXPANSION BOLTS: HILTI "KWIK BOLTS" OR APPROVED EQUAL.
2. SPECIFICATIONS: WELDING PERSONNEL AND PROCEDURE SHALL BE QUALIFIED PER AWS D1.1. UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION AND ERECTION TO BE GOVERNED BY THE FOLLOWING CODES (LATEST EDITION)
   A. AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS.
   B. AISC CODE OF STANDARD PRACTICE.
   C. STRUCTURAL WELDING CODE, AWS D1.1 OF THE AMERICAN WELDING SOCIETY.
3. CONNECTIONS:
   CONNECTIONS TO BE DESIGNED BY THE FABRICATOR TO DEVELOP FULL STRENGTH OF MEMBER OR NODE SHOWN ON THE PLANS, WHICHEVER GOVERNS. FOLLOW INSTRUCTIONS AND DETAILS FOR GENERAL ARRANGEMENT OF PARTICULAR DETAILS.
4. GALVANIZING: ALL SHELF ANGLES, LINTELS IN EXTERIOR WALLS, AND ALL ANCHOR STEEL EXPOSED TO THE ELEMENTS SHALL BE HOT DIP GALVANIZED.
5. MISCELLANEOUS:
   A. PROVIDE HOLES FOR OTHERS. IF OPENING IS NOT SHOWN ON THE STRUCTURAL DRAWINGS, OBTAIN PRIOR APPROVAL.
   B. STEEL SUPPORTING OR CONNECTED TO HVAC AND OTHER EQUIPMENT AND ROOF OPENINGS AS SHOWN ON THE DRAWINGS IS SHOWN FOR BIDDING PURPOSES ONLY. CONTRACTOR SHALL RECONCILE EXACT SIZE AND LOCATION BEFORE PROCEEDING WITH HIS WORK.
   C. GROUT UNDER BEARING PLATES, BASE PLATES, AND SETTING PLATES TO BE NON-SHRINKING TYPE.
   D. STEEL BELOW GRADE TO BE PROTECTED BY A MIN. OF 3 INCHES OF CONCRETE.
   E. PROVIDE HEAVY WASHER AT ALL ANCHOR BOLTS.
   F. EMBEDMENT LENGTH OF EXPANSION BOLTS INTO SOLID MASONRY OR CONCRETE SHALL BE AS FOLLOWS:
   1/2 INCH DIAMETER BOLTS —— 3 1/2 INCHES EMBEDMENT
   3/4 INCH DIAMETER BOLTS —— 5 INCHES EMBEDMENT

F. MASONRY

1. MATERIALS:
   A. CONCRETE BLOCK: ASTM C90 (HOLLOW) ASTM C145 (SOLID).
   B. MORTAR: ASTM C270 TYPE "S", AVERAGE COMPRESSIVE STRENGTH: 1800 PSI MINIMUM (AT 28 DAYS).
   C. BOND BEAM AND CORE FILL: ASTM C476, COARSE TYPE.
   D. JOINT REINFORCING: MILL GALVANIZED FINISH, 9 GAGE MINIMUM SIDE WIRES AND CROSS WIRES (LADDER OR TRUSS TYPE).
   E. BAR REINFORCING: ASTM A615, GRADE 60.
2. REINFORCED MASONRY: WHERE VERTICAL BARS ARE TO BE GROUTED INTO CORES, THE FOLLOWING REQUIREMENTS APPLY:
   A. PROVIDE DOWELS FROM FOOTING, SAME SIZE AND SPACING AS WALL BARS. LAP 12 INCHES MINIMUM WITH WALL BAR. EMBED INTO FOOTING 9 INCHES.
   B. PROVIDE A CONTINUOUS VERTICAL CAVITY, AT LEAST 2"x3" IN SIZE, FREE OF MORTAR DROPPINGS.
   C. AT SPLICES IN VERTICAL BARS, PROVIDE MECHANICAL COUPLERS OR 48 DIAMETER LAP.
   D. ALL REINFORCEMENT MUST BE INSTALLED AND SECURELY ANCHORED IN PLACE PRIOR TO PLACEMENT OF GROUT.
3. MISCELLANEOUS:
   A. FILL CORE SOLID AROUND ANCHOR BOLTS.
   B. PROVIDE 100% SOLID BLOCKS OR SOLIDLY FILLED HOLLOW BLOCKS AT ALL EXPANSION BOLT LOCATIONS.
   C. HOLLOW MASONRY UNITS TO BE LAID WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS. WEBS SHALL ALSO BE BEDDED IN ALL COURSES IN THE STARTING COURSE ON FOOTINGS, AND WHEN ADJACENT TO CELLS OR CAVITIES TO BE REINFORCED OR FILLED WITH CONCRETE OR GROUT. SOLID UNITS TO BE LAID WITH FULL HEAD AND BED JOINTS.
   E. PROVIDE JOINT REINFORCING AT 16 INCHES, EXCEPT AS NOTED.
   F. LAP JOINT REINFORCING 6 INCHES FOR STANDARD, 15 INCHES FOR HEAVY WEIGHT.
   G. WHERE MASONRY UNITS ARE USED ABOVE HOLLOW UNITS OF A DIFFERENT THICKNESS, PROVIDE A CONTINUOUS COURSE OF 100% SOLID MASONRY (OR SOLID GROUTED BLOCK) AT LEAST 8 INCHES HIGH BELOW TRANSITION.

H. STRUCTURAL LUMBER

1. A. STUDS: STRUCTURAL LUMBER: DOUGLAS FIR-LARCH #2, OR APPROVED EQUAL:

SIZE	Fb	Ft	Fv	Fc⊥	Fc	E
2x4	1,350	825	90	565	1,650	1,600,000
2x6	1,250	725	90	565	1,600	1,600,000
2x8	1,200	600	90	565	1,550	1,600,000
2x10	1,050	600	90	565	1,500	1,600,000
2x12	975	550	90	565	1,450	1,600,000

   B. ROOF/WALL: ORIENTED STRAND BOARD: STRUCTURAL 1, EXPOSURE 1, EXTERIOR GLUE. FOR ROOF AND WALLS PANEL IDENTIFICATION INDEX 24/16 – 5/8 INCH OR 24/0 – 1/2 INCH (WITH PLYWOOD CLIPS AT ROOF).
   C. ROOF/WALL: PLYWOOD: C-DPLUGGED, STRUCTURAL 1, EXPOSURE 1, EXTERIOR GLUE FOR ROOF AND WALL PANEL IDENTIFICATION INDEX 24/16–5/8 INCH OR 24/0–1/2 INCH (WITH PLYWOOD CLIPS AT ROOF).
   D. SILL PLATES: NO. 2 SPRUCE-PINE-FIR, OR EQUAL FC=675 PSI, FV=70 PSI, E= 1,200,000 PSI
2. SPECIFICATIONS: UNLESS SPECIFICALLY SHOWN OTHERWISE, DESIGN, FABRICATION AND ERECTION SHALL BE GOVERNED BY THE LATEST REVISIONS OF:
   A. NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENINGS.
   B. U.S. PRODUCT STANDARD PS-1 FOR SOFTWOOD PLYWOOD – CONSTRUCTION AND INDUSTRIAL.
3. CONNECTIONS:
   A. JOISTS TO BEAMS – 16 GA. GALVANIZED STD. JOIST HANGERS, UNLESS SHOWN OTHERWISE.
   B. PLYWOOD TO ROOF TRUSSES OR RAFTERS – NAILED – USE 8d RING SHANK NAILS AT 6 INCHES O/C AT PANEL EDGES AND 12 INCHES C/C AT INTERMEDIATE SUPPORTS. PROVIDE PLYWOOD CLIPS AT MID-SPAN OF PLYWOOD BETWEEN SUPPORTS.
4. ALL STRUCTURAL WOOD TO BE SURFACED FOUR (4) SIDES (S-4-S) A AND MAXIMUM MOISTURE CONTENT OF 19 PERCENT.
5. ALL LUMBER AND PLYWOOD IN CONTACT WITH CONCRETE, STUCCO, MASONRY OR OTHER CEMENTITIOUS MATERIALS SHALL BE TREATED WITH AN E.P.A. ACCEPTABLE WOOD PRESERVATIVE (SUCH AS "ACQ" – ALKALINE-COPPER-QUATERNARY OR "CBA-A" COPPER AZOLE TYPE A & B).
6. ALL WOOD CONNECTORS SHALL BE GALVANIZED STEEL OR RUST-PROOF PAINTED STEEL (U.O.N.). ALL GALVANIZED METAL CONNECTORS IN CONTACT WITH TREATED WOOD (ITEM #5) SHALL BE "TRIPLE-ZINC G-185" GALVANIZED. ANY FIELD WELDS (INTERIOR OR EXTERIOR) OF SUCH CONNECTORS SHALL BE WIRE BRUSH CLEANED AND RUST PROOF PAINTED.
7. MISCELLANEOUS:
   A. USE ONE LINE OF SOLID BLOCKING OR CROSS BRIDGING AT 8'– 0" O/C MAX. FOR ALL JOISTS AND RAFTERS, USE SOLID BLOCKING AT JOIST AND RAFTER BEARING.
   B. USE SOLID BLOCKING AT MID-HEIGHT FOR ALL EXTERIOR STUD WALLS AND INTERIOR BEARING PARTITIONS.
   C. USE DOUBLE STUDS UNDER BEAM AND LINTEL BEARING, UNLESS SHOWN OTHERWISE.

I. PREFABRICATED WOOD TRUSSES

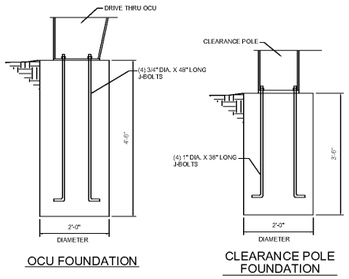
1. MATERIALS:
   A. LUMBER: SEE "STRUCTURAL LUMBER" SECTION FOR WOOD INFORMATION.
   B. METAL CONNECTOR PLATES: GALVANIZED SHEET STEEL ASTM A446 (LATEST EDITION) GRADE "A". COATING CLASS G60 PER ASTM A525 (LATEST EDITION). MANUFACTURED WITH HOLES, PLUGS, TEETH, OR PRONGS UNIFORMLY SPACED AND FORMED.
   SEE "STRUCTURAL LUMBER" SECTION FOR GALVANIZED CONNECTIONS FOR TREATED WOOD.
2. DESIGN CRITERIA:
   A. LOADING:

TOP CHORD LIVE LOAD:	20 PSF
TOP CHORD DEAD LOAD:	20 PSF + MECH EQUIP.
BOTTOM CHORD DEAD LOAD:	10 PSF
NET WIND UPLIFT:	15 PSF

   B. DESIGN OF MEMBERS AND CONNECTIONS IS TO BE BY A PROFESSIONAL ENGINEER, REGISTERED IN THE STATE OF THIS PROJECT, EXPERIENCED IN SIMILAR DESIGN, RETAINED BY THE TRUSS MANUFACTURER.
   C. SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE ENGINEER RESPONSIBLE FOR THE TRUSS DESIGN. IN ADDITION, SIGNED AND SEALED DESIGN CALCULATIONS FOR THESE TRUSSES SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.
   D. MEMBER SIZES SHOWN ARE MINIMUM SIZES.
   E. MAXIMUM LIVE LOAD DEFLECTION IS TO BE L/360.
   F. MAXIMUM TOTAL LOAD DEFLECTION IS TO BE L/240.
3. MISCELLANEOUS:
   A. BOLT TOP CHORDS OF ALL MULTIPLE MEMBER TRUSSES TOGETHER WITH 1/2"ø BOLTS AT 4'–0" O.C. BOLT WEB MEMBERS TOGETHER WITH 1/2"ø BOLTS AT 2'–0" O.C. AT CONCENTRATED LOADS, UNLESS OTHERWISE SPECIFIED BY THE TRUSS DESIGN ENGINEER.
   B. VERIFY ALL DIMENSIONS, ELEVATIONS AND SLOPES PRIOR TO MANUFACTURING. REPORT ANY DISCREPANCIES IMMEDIATELY TO THE ARCHITECT.
   C. WOOD TRUSSES SHALL BE DESIGNED AND FABRICATED TO CONFORM TO THE GEOMETRY SHOWN ON THE DRAWINGS. WEB CONFIGURATIONS ARE TO BE DETAILED AS REQUIRED BY THE DESIGNER/FABRICATOR.
   D. PROVIDE 2x4 BOTTOM CHORD BRIDGING AT A MAXIMUM SPACING OF [illegible] O.C.

J. ABBREVIATIONS:

T = TOP
B = BOTTOM
C.M.U. = CONCRETE MASONRY UNIT
E.F. = EACH FACE
E.W. = EACH WAY
U.O.N. = UNLESS OTHERWISE NOTED
CLR = CLEAR
T.O.F. = TOP OF FOOTING ELEVATION
T.O.S. = TOP OF SLAB ELEVATION
T.O.W. = TOP OF WALL ELEVATIONS
S.O.G. = SLAB ON GRADE
W.W.F. = WELDED WIRE FABRIC
U.N.O. = UNLESS NOTED OTHERWISE
OTHERWISE
TYP. = TYPICAL
B.E. = BRUSSED END ELEVATION
J.E. = JOIST BEARING ELEVATION
L.E. = LOAD LED VERTICAL
L.E.H. = LOAD LED HORIZONTAL

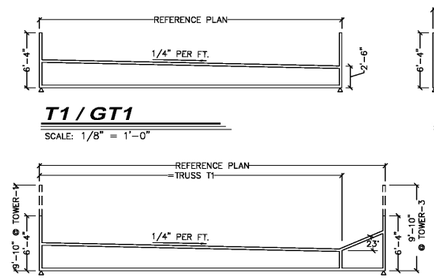


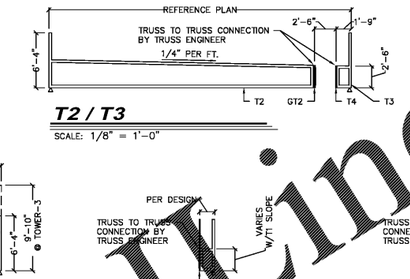
OCU FOUNDATION

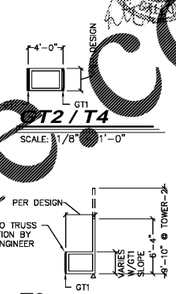
CLEARANCE POLE FOUNDATION

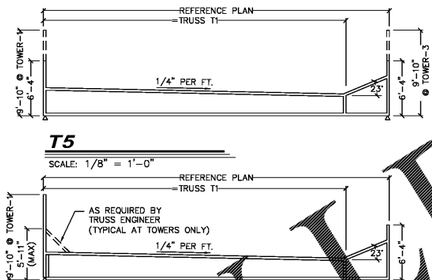
SCHEDULE OF SPECIAL INSPECTION SERVICES		
MATERIAL/ACTIVITY	SERVICE	EXTENT
1704.4 CONCRETE CONSTRUCTION		
INSPECTION OF REINFORCING STEEL INSTALLATION	FIELD INSPECTION	PERIODIC
INSPECTION OF CAST-IN-PLACE BOLTS	FIELD INSPECTION	CONTINUOUS
VERIFICATION OF REQUIRED DESIGN MIX	REVIEW SUBMITTALS	PERIODIC
FRESH CONCRETE SAMPLING	FIELD TESTING	CONTINUOUS
CONCRETE PLACEMENT	FIELD REVIEW	CONTINUOUS
CONCRETE CURING OPERATIONS	FIELD REVIEW	PERIODIC
EVALUATION OF CONCRETE STRENGTH	FIELD TESTING AND REVIEW	
1704.5 STRUCTURAL WOOD		
VERIFY FABRICATION/QUALITY CONTROL PROCEDURES	IN-PLANT REVIEW	PERIODIC
INSPECTION OF NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS	SHOP AND FIELD INSPECTION	PERIODIC
ROOF DIAPHRAGM & SHEAR WALLS	FIELD INSPECTION	PERIODIC
1704.7 SOILS		
VERIFY SITE PREPARATION COMPLIES WITH APPROVED SOILS REPORT.	FIELD INSPECTIONS	CONTINUOUS
VERIFY PLACEMENT AND COMPACTION OF FILL MATERIALS COMPLIES WITH APPROVED SOILS REPORT.	FIELD INSPECTION	CONTINUOUS
VERIFY DRY DENSITY OF COMPACTED FILL COMPLIES WITH APPROVED SOILS REPORT.	REVIEW FIELD TESTING	CONTINUOUS

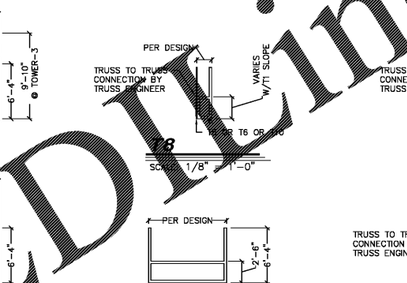
NOTE: THE INSPECTION AND TESTING AGENTS SHALL BE ENGAGED BY THE OWNER OR THE OWNERS AGENT, AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL PRIOR TO COMMENCING WORK. THE QUALIFICATIONS OF THE INSPECTION AGENT MAY BE SUBJECT TO THE APPROVAL OF THE BUILDING OFFICIAL. SPECIAL INSPECTIONS REPORTS AND A FINAL REPORT IN ACCORDANCE WITH SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF THE WORK IS APPROVED FOR OCCUPANCY.

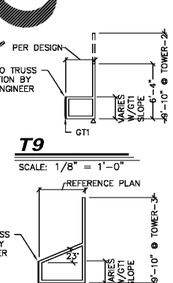


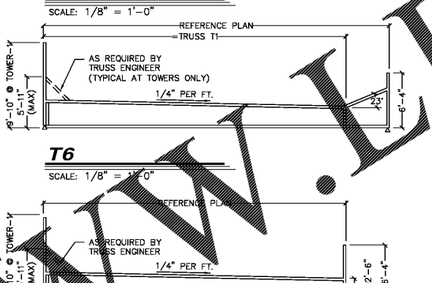


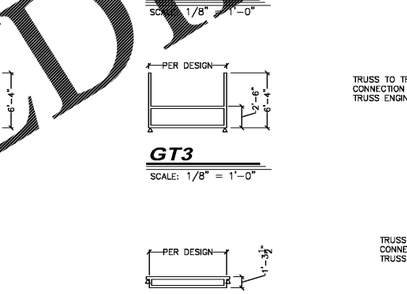


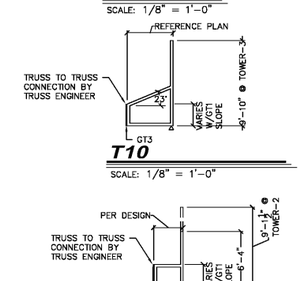


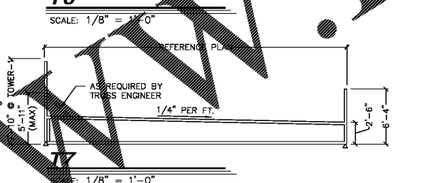


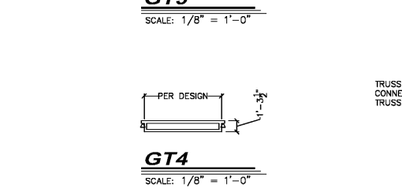


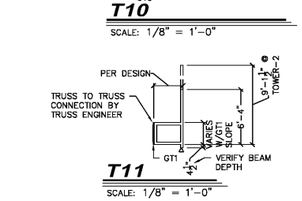








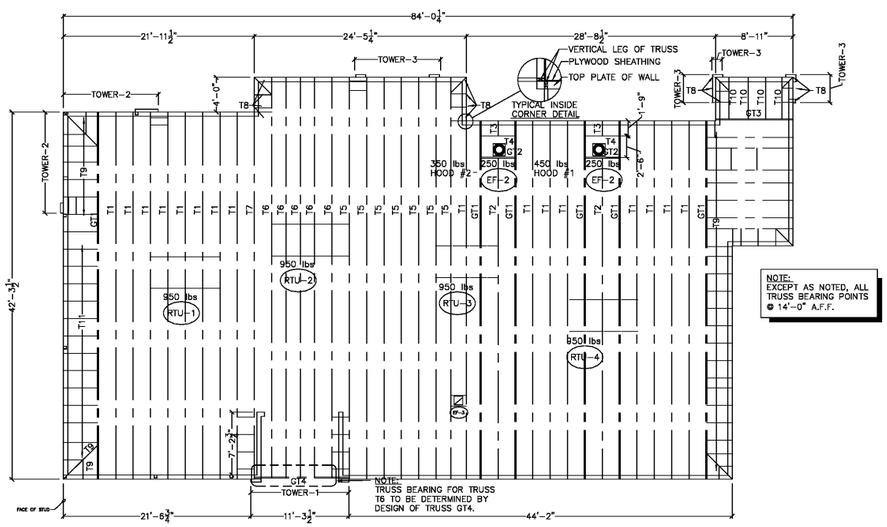




- TRUSS PROFILE PROVIDE FOR DESIGN INTENT ONLY. FIELD MEASUREMENTS REQUIRED FOR SITE SPECIFIC PROJECT.
- TRUSS DESIGN/MEMBER SIZES TO BE DETERMINED BY THE TRUSS ENGINEER BASED ON LOCAL DESIGN LOAD REQUIREMENTS.
- REFER TO GENERAL NOTES FOR ADDITIONAL INFORMATION

TRUSS PROFILES

SCALE: 1/8" = 1'-0"



TRUSS LAYOUT

SCALE: 1/8" = 1'-0"

Other Plans @ WWW.LDLine.com

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DRAWN BY:
CHECKED BY:
NO. DATE:

PROJECT #:18026.02
ROC-60 TALL 20/20 IMAGE: NOVEMBER 2018 DESIGN RELEASE
BURGER KING RESTAURANT
STREET ADDRESS
HARPERSVILLE, AL
STRUCTURAL NOTES AND DETAILS

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